

Standard precast concrete bridge beams
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Research Report 364

Standard precast concrete bridge beams

Beca Carter Hollings & Ferner Ltd (Beca)

Opus International Consultants Ltd (Opus)

NZ Transport Agency Research Report 364

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We also acknowledge the contribution made by various members of the precast industry during several workshops held during the preparation of these standard designs.

Keywords: Precast concrete, bridge decks, standard designs for New Zealand, Super T, I beams, Hollow core

An important note for the reader

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This research report is the final stage of a project commissioned by Transfund New Zealand before 2004 and is published by the NZ Transport Agency.

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Additional note

The NZ Transport Agency (NZTA) was formally established on 1 August 2008, combining the functions and expertise of Land Transport NZ and Transit NZ. The new organisation will provide an integrated approach to transport planning, funding and delivery.

This research report was prepared prior to the establishment of the NZTA and may refer to Land Transport NZ and Transit NZ.

Abstract

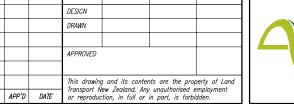
Hollow core units for bridge spans of various length.

The standardised designs for precast bridge beams presented in this publication are expected to result in significant economies for NZ Transport Agency bridge projects utilising these elements in New Zealand

- 1. REFER TO DRAWING SO.02 FOR GENERAL NOTES.
- 2. REFER TO DRAWING SO.03 FOR CONCRETE NOTES.

DRAWING INDEX

DRAWING NO.	TITLE	
\$0.01 \$0.02 \$0.03	GENERAL GENERAL GENERAL	DRAWING INDEX USER NOTES CONCRETE NOTES
\$1.01 \$1.02 \$1.03 \$1.04 \$1.05 \$1.06 \$1.07	SUPER T BRIDGE 1025 DEEP SUPER T BRIDGE 1025 DEEP	20m & 22.5m SPAN - ARRANGEMENT AND DIMENSIONS 20m & 22.5m SPAN - PRESTRESSING DETAILS 20m & 22.5m SPAN - REINFORCEMENT SHEET 1 20m & 22.5m SPAN - REINFORCEMENT SHEET 2 20m & 22.5m SPAN - UNIT DATA 20m & 22.5m SPAN - IN-SITU END DIAPHRAGM DETAILS 20m & 22.5m SPAN - DECK DETAILS
\$1.11 \$1.12 \$1.13 \$1.14 \$1.15 \$1.16 \$1.17	SUPER T BRIDGE 1225 DEEP SUPER T BRIDGE 1225 DEEP	25m & 27.5m SPAN - ARRANGEMENT AND DIMENSIONS 25m & 27.5m SPAN - PRESTRESSING DETAILS 25m & 27.5m SPAN - REINFORCEMENT SHEET 1 25m & 27.5m SPAN - REINFORCEMENT SHEET 2 25m & 27.5m SPAN - UNIT DATA 25m & 27.5m SPAN - END DIAPHRAGM DETAILS 25m & 27.5m SPAN - DECK DETAILS
\$1.21 \$1.22 \$1.23 \$1.24 \$1.25 \$1.26 \$1.27	SUPER T BRIDGE 1225 DEEP SUPER T BRIDGE 1225 DEEP	30m SPAN - ARRANGEMENT AND DIMENSIONS 30m SPAN - PRESTRESSING DETAILS 30m SPAN - REINFORCEMENT SHEET 1 30m SPAN - REINFORCEMENT SHEET 2 30m SPAN - UNIT DATA 30m SPAN - END DIAPHRAGM DETAILS 30m SPAN - DECK DETAILS
\$2.01 \$2.02 \$2.03 \$2.04 \$2.05	SINGLE HOLLOW CORE BEAMS 650 DEEP SINGLE HOLLOW CORE BEAMS 650 DEEP	16m & 18m SPAN — ARRANGEMENT AND DIMENSIONS 16m SPAN — REINFORCEMENT & STRESSING DETAILS 18m SPAN — REINFORCEMENT & STRESSING DETAILS 16m & 18m — SPAN UNIT DATA 16m & 18m SPAN — LINKAGE BAR & TRANSVERSE CONNECTION DETAILS
\$2.10 \$2.11 \$2.12 \$2.13 \$2.14 \$2.15	SINGLE HOLLOW CORE BEAMS 900 DEEP SINGLE HOLLOW CORE BEAMS 900 DEEP	20m, 22.5m & 25m SPAN — ARRANGEMENT AND DIMENSIONS 20m SPAN — REINFORCEMENT & STRESSING DETAILS 22.5m SPAN — REINFORCEMENT & STRESSING DETAILS 25m SPAN — REINFORCEMENT & STRESSING DETAILS 20m, 22.5m & 25m SPAN — UNIT DATA 20m, 22.5m & 25m — LINKAGE BAR & TRANSVERSE CONNECTION DETAILS
\$3.01 \$3.02 \$3.03 \$3.04 \$3.05	DOUBLE HOLLOW CORE BEAMS 587 DEEP	12m & 14m SPAN — ARRANGEMENT AND DIMENSIONS 12m SPAN — REINFORCEMENT & STRESSING DETAILS 14m SPAN — REINFORCEMENT & STRESSING DETAILS 12m & 14m SPAN — UNIT DATA 12m & 14m SPAN — LINKAGE BAR & TRANSVERSE CONNECTION DETAILS
\$4.01 \$4.02 \$4.03 \$4.04 \$4.05 \$4.06	I-BEAMS 1500 DEEP	18m & 20m SPAN — ARRANGEMENT AND DIMENSIONS 18m SPAN — REINFORCEMENT & STRESSING DETAILS 20m SPAN — REINFORCEMENT & STRESSING DETAILS 18m & 20m SPAN — UNIT DATA 18m & 20m SPAN — DIAPHRAGM DETAILS 18m & 20m SPAN — DECK DETAILS
\$4.10 \$4.11 \$4.12 \$4.13 \$4.14 \$4.15 \$4.16	I-BEAMS 1600 DEEP	22m & 24m SPAN — ARRANGEMENT AND DIMENSIONS 22m SPAN — REINFORCEMENT & STRESSING DETAILS 24m SPAN — REINFORCEMENT & STRESSING DETAILS 22m & 24m SPAN — UNIT DATA 22m & 24m SPAN — END DIAPHRAGM DETAILS 22m & 24m SPAN — MIDSPAN DIAPHRAGM DETAILS 22m & 24m SPAN — DECK DETAILS
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DESIGN DRAWN

APPROVED

NZ **TRANSPORT** AGENCY WAKA KOTAHI



STANDARD PRECAST CONCRETE BRIDGE BEAMS

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GRAPHIC SCALES

INTRODUCTION

ENCLOSED ARE STANDARD DRAWINGS FOR HOLLOW CORE, SUPER 'T' AND 'I' GIRDER PRECAST BEAMS, AND BRIDGE DECK ARRANGEMENTS. SIMPLY SUPPORTED BRIDGE DECKS UP TO 30 M IN SPAN ARE DETAILED ON THE DRAWINGS.

THE DESIGNS ARE FOR A SINGLE CARRIAGEWAY CROSS SECTION CONSISTING OF 2 NO. 3.5 M LANES AND 2 NO. 1.2 M SHOULDERS. OTHER CARRIAGEWAY CONFIGURATIONS HAVE NOT BEEN CONSIDERED. THE DESIGNS ARE APPLICABLE FOR BRIDGE SKEWS UP TO 15°. THE DECKS ARE DESIGNED TO SUPPORT TL4 RIGID CONCRETE BARRIER EDGE PROTECTION.

THE FOLLOWING ITEMS ARE NOT COVERED IN THE STANDARD DESIGNS:

- SUBSTRUCTURES
- SEISMIC DESIGN (SOME STANDARD DETAILS ARE INCLUDED HOWEVER SPECIFIC DESIGNS OF LINKAGE BOLTS, SHEAR KEYS AND SUCH LIKE ARE REQUIRED)
- · BEARINGS (GRAVITY AND LIVE LOADINGS, AND ROTATIONS UNDER LIVE LOADINGS ARE LISTED)
- BRIDGE JOINTS

OUTCOMES OF THIS PROJECT.

BACKGROUND

IN THE MID 1970S THE MINISTRY OF WORKS (MWD) DESIGNED A RANGE OF TWIN HOLLOW CORE, 'I' AND 'U' PRECAST CONCRETE BRIDGE BEAMS WHICH WERE ADOPTED AS NEW ZEALAND INDUSTRY STANDARDS. USE OF THESE STANDARD DESIGNS LED TO MORE COST-EFFICIENT DESIGN AND CONSTRUCTION.

THE STANDARD MWD BRIDGE BEAM DESIGNS COMPLETED IN THE 1970S ARE NOW OUT OF DATE, BOTH WITH RESPECT TO DESIGN CODES AND HIGHER STRENGTH MATERIALS NOW COMMONLY USED. IN PARTICULAR, CHANGES TO CONCRETE DURABILITY, BRIDGE DECK WIDTH AND SIDE PROTECTION REQUIREMENTS HAVE LED TO THE EXISTING DESIGNS NO LONGER COMPLYING WITH CURRENT STANDARDS. DISCUSSIONS WITH A WIDE RANGE OF INDUSTRY PARTICIPANTS (CONSULTANTS, BRIDGE CONTRACTORS, CONCRETE PRECASTERS AND ROAD CONTROLLING AUTHORITIES [RCAS]) INDICATED STRONG SUPPORT FOR A NEW RANGE OF STANDARD BEAM DESIGNS. SUBSEQUENTLY, A LAND TRANSPORT NZ SPONSORED PROJECT WAS INSTIGATED TO BEGIN THE PROCESS OF UPDATING THE STANDARD BEAM DESIGNS. THE CEMENT & CONCRETE ASSOCIATION AND

PRECAST NEW ZEALAND ALSO PROVIDED FINANCIAL SUPPORT FOR

THE PROJECT. THE DESIGNS INCLUDED HEREIN ARE THE FINAL

THE PROJECT

GENERAL

BECA AND OPUS WERE COMMISSIONED TO DEVELOP THE NEW STANDARD BEAM DESIGNS. A STEERING GROUP WAS SET UP TO GUIDE THE PROJECT FROM THE OUTSET. THIS GROUP INCLUDED END USERS, PRECASTERS, CONSULTANTS, CONTRACTORS, PRECAST NEW ZEALAND AND CEMENT & CONCRETE ASSOCIATION.

THE INITIAL STAGE OF THE PROJECT INVOLVED IDENTIFYING THE MOST APPROPRIATE CONCRETE BRIDGE BEAM SHAPES AND SPANS THAT SHOULD BE ADOPTED AS INDUSTRY STANDARDS FOR THE FUTURE. AFTER EXTENSIVE CONSULTATION WITH THE INDUSTRY TWO EXISTING BEAM TYPES WERE RETAINED AS STANDARD BEAM SHAPES:

- HOLLOW CORE BEAMS CURRENTLY SUITABLE FOR SPANS UP TO 20 M (587 MM AND 650 MM DEEP), BUT TO BE EXTENDED FOR SPANS UP TO 25 M (900 MM DEEP)
- I-BEAMS FOR A SPAN RANGE OF 16-24 M.

IN ADDITION TO THE ABOVE, A NOW COMMONLY USED NEW SHAPE WAS SELECTED: THE SUPER T BEAM, WHICH IS WIDELY USED IN AUSTRALIA. BEAM DEPTHS FOR DESIGN ARE AS FOLLOWS:

- 1025 MM FOR SPANS UP TO 22.5 M
- 1225 MM FOR SPANS UP TO 30 M

STAGE 2

THIS STAGE INVOLVED CARRYING OUT DESIGN CALCULATIONS AND PRODUCING CONSTRUCTION DRAWINGS FOR THE ABOVE BEAM CONFIGURATIONS

DESIGN STANDARDS

THE MAIN STANDARDS AND CODES USED FOR THE DESIGN PHASE INCLUDED:

- TRANSIT NEW ZEALAND BRIDGE MANUAL, 2ND EDITION, (TNZBM), INCLUDING AMENDMENTS ISSUED IN SEPTEMBER 2004 AND
- CONCRETE STRUCTURES STANDARD, NZS 3101: 2006
- CONCRETE CONSTRUCTION NZS 3109: 1997.

DESIGN METHODOLOGY

CALCULATION OF SECTION DEMANDS

THE BRIDGE DECKS HAVE BEEN ANALYSED USING A TWO-DIMENSIONAL PLANE FRAME GRILLAGE MODEL TO TAKE ADVANTAGE OF THE TRANSVERSE LOAD SPREAD THAT OCCURS IN BRIDGE DECKS.

SERVICEABILITY LIMIT STATE DESIGN OF BEAMS

PARTIAL PRESTRESS (CRACKED SECTION) ANALYSIS OF THE BEAMS IN ACCORDANCE WITH NZS 3101 WAS UNDERTAKEN TO ENSURE MAXIMUM ECONOMIES IN DESIGN WERE ACHIEVED.

LILTIMATE LIMIT STATE

THE DESIGN PROCEDURES IN NZS 3101 WERE ADOPTED FOR CALCULATING SECTION CAPACITIES

GENERAL LIMITATIONS OF STANDARD DESIGNS

THE DESIGNS HAVE BEEN DEVELOPED FOR THE TWO-LANE CARRIAGEWAY ARRANGEMENT; OTHER CARRIAGEWAY CONFIGURATIONS WILL REQUIRE SPECIFIC DESIGNS TO BE CARRIED OUT.

THE DESIGNS ARE SUITABLE FOR SKEWS UP TO 15° MAXIMUM. BRIDGES WITH HIGHER SKEWS WOULD REQUIRE BRIDGE SPECIFIC DESIGNS TO BE

SEISMIC DESIGN IS NOT ADDRESSED IN THESE STANDARD DESIGNS. THE DESIGNS ARE SUITABLE FOR MULTIPLE SIMPLY SUPPORTED SPAN ARRANGEMENTS. SEISMIC DETAILING INCLUDING LINKAGE SYSTEMS WOULD NEED TO BE DESIGNED TO SHIT LOCALITY REQUIREMENTS

SUBSTRUCTURE DETAILS ARE NOT INCLUDED. THESE WILL NEED TO BE DESIGNED TAKING INTO ACCOUNT LOCALITY, GEOTECHNICAL CONDITIONS, SEISMICITY AND OTHER RELEVANT FACTORS

SPECIFICATION REQUIREMENTS

THE DESIGNS ARE BASED ON MATERIAL AND WORKMANSHIP BEING IN ACCORDANCE WITH THE SPECIFICATION DEVELOPED IN CONJUNCTION WITH THE STANDARD DRAWINGS.

SUBSEQUENT AMENDMENTS

THE CONTINUOUS DEVELOPMENT OF BRIDGE TECHNOLOGY AND OF NEW PRODUCTS AND MATERIALS IS EXPECTED TO RESULT IN FUTURE AMENDMENTS AND ADDITIONAL STANDARD BEAMS BEING REQUIRED.

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THIS RESEARCH REPORT IS THE FINAL STAGE OF A PROJECT COMMISSIONED BY TRANSFUND NEW ZEALAND BEFORE 2004 AND IS PUBLISHED BY THE NZ TRANSPORT AGENCY.

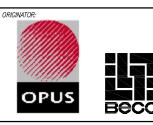
THE MATERIAL CONTAINED IN THIS RESEARCH REPORT, INCLUDING THE STANDARD BRIDGE BEAM DESIGNS, HAS BEEN PREPARED BY BECA CARTER HOLLINGS AND FERNER LIMITED (BECA) FOR THE NZ TRANSPORT AGENCY BECA'S APPROACH TO AND SCOPE OF ENQUIRIES ANALYSIS UNDERTAKEN, AND THE CONTENT, CONCLUSIONS AND RECOMMENDATIONS CONTAINED IN THE RESEARCH REPORT, HAVE BEEN SPECIFICALLY AGREED UPON WITH NZ TRANSPORT AGENCY. WHILE CARE HAS BEEN TAKEN IN COMPILING THE RESEARCH REPORT, NEITHER THE NZ TRANSPORT AGENCY (INCLUDING ITS EMPLOYEES, AGENTS AND CONSULTANTS), BECA, NOR BECA'S EMPLOYEES, AGENTS, SUBCONSULTANTS, JOINT VENTURES OR RELATED COMPANIES (TOGETHER RELATED PARTIES) UNDERTAKE ANY DUTY OF CARE TO THIRD PARTY USERS OF THE RESEARCH REPORT. BEFORE ANY DECISION IS MADE OR ACTION TAKEN ON THE RESEARCH REPORT, APPROPRIATE LEGAL OR OTHER PROFESSIONAL ADVICE SHOULD BE SOUGHT.

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STANDARD PRECAST CONCRETE BRIDGE BEAMS

GENERAL USER NOTES

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- b. ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE CURRENT CODES OF PRACTICE, EXCEPT WHERE VARIED BY THE SPECIFICATION AND/OR DRAWINGS
- c. ANY CODES OF PRACTICE AND OR STANDARDS REFERRED TO ON THE DRAWINGS AND/OR SPECIFICAITON REFER TO THE LATEST ISSUE AND AMENDMENTS CURRENT AT THE TIME OF PREPARING THESE DRAWINGS
- d. REQUIREMENTS FOR:
 - SHOP DRAWINGS
 - PROPPING DOCUMENTATION
 - INSPECTION AND TESTING DOCUMENTS
 - MATERIAL SPECIFICATION
 - TOLERANCES

ARE INCLUDED IN THE SPECIFICATION.

2. DIMENSIONS

a. ALL DIMENSIONS ARE IN MILLIMETRES, UNLESS NOTED OTHERWISE.

3. ABBREVIATIONS

LV

MAX

MIN

NF

NOM

RB

REF

REINF

RH

RL

TYP

UNO

ALT APPROX B	ALTERNATE APPROXIMATE BOTTOM
С	COVER
CJ	CONSTRUCTION JOINT
CL Q	CENTRELINE
COL	COLUMN
CONC	CONCRETE
CRS	CENTRES
CS	CRITICAL SECTION
D	DEFORMED BAR GRADE 300E
DH	DEFORMED BAR GRADE 500E
DIA Ø	DIAMETER
DWG	DRAWING
EF	EACH FACE
EQ	EQUAL
EW	EACH WAY
FF	FAR FACE
GL	GROUND LEVEL

LENGTH VARIES

PLAIN BAR GRADE 300E

PLAIN BAR GRADE 500E

UNLESS NOTED OTHERWISE

MAXIMUM

MINIMUM

NOMINAL

RFIDBAR

REINFORCEMENT

REDUCED LEVEL

REFER

TOP

TYPICAL

NEAR FACE

4. REINFORCED CONCRETE

4.1 CONCRETE STRENGTHS

CONCRETE STRENGTHS ARE 'SPECIFIED 28 DAY COMPRESSIVE STRENGTHS' AS DEFINED IN NZS3109.

CONCRETE STRENGTHS ARE GENERALLY SPECIFIED ON INDIVIDUAL DRAWINGS. WHERE NOT SPECIFIED CONCRETE STRENGTH SHALL BE 40

4.2 CONCRETE FINISHES

WHERE NOT SPECIFIED AND NOT SHOWN ON DRAWINGS, SURFACE FINISHES SHALL BE AS FOLLOWS: (REFER NZS 3114 FOR DEFINITIONS)

a.	CONCEALED FORMED SURFACES	F1	
b.	EXPOSED FORMED SURFACES	F4	
c.	EXPOSED UNFORMED SURFACES	U2	
d.	CONCEALED UNFORMED SURFACES	U1	
e.	CORNERS TO BE RADIUSED OR CHAMFERED	UNLESS NOTED	
	OTHERWISE.		

4.3 CONCRETE COVER TO REINFORCEMENT

MINIMUM CONCRETE COVERS ARE GENERALLY SPECIFIED ON INDIVIDUAL DRAWINGS.

WHERE NOT SPECIFIED, MINIMUM CONCRETE COVERS SHALL BE AS FOLLOWS:

	BEAMS		SLABS	
EXPOSURE	SITUATION	MAIN BARS	STIRRUPS & TIES	ALL BARS
EXPOSED TO	CAST-IN PLACE	40	40	50
WEATHER	PRECAST	40	40	40
NOT EXPOSED	CAST-IN PLACE	30	30	30
TO WEATHER	PRECAST	30	30	30

- (i) TOLERANCES ON COVERS SHALL BE IN ACCORDANCE WITH NZS3109. THE SPECIFICATION AND THE DRAWINGS AS APPROPRIATE.
- (ii) PRECAST IN THE CONTEXT OF THIS TABLE MEANS CONCRETE CAST UNDER PLANT CONTROL CONDITIONS, UTILISING RIGID FORMWORK AND INTENSE COMPACTION
- (iii) TIES MAY INTRUDE 10mm MAXIMUM INTO THE SPECIFIED CONCRETE COVER

4.4 PLACING AND SPACING OF REINFORCEMENT

- a. SPLICING OF REINFORCEMENT, WHETHER BY LAPPING OR MECHANICAL SPLICE, SHALL ONLY BE CARRIED OUT AS SHOWN ON THE DRAWINGS.
- b. ALL HOOKS ON STIRRUPS AND TIES MUST FIT CLOSELY AROUND MAIN BARS UNO; FIRST STIRRUP TO BE PLACED NOT FURTHER THAN THE LESSER OF HALF THE STIRRUP SPACING OR 50mm FROM SUPPORT FACE.

4.5 LAP SPLICES IN REINFORCEMENT

- a. LAP LENGTHS FOR DEFORMED BARS SHALL BE AS SHOWN IN THE FOLLOWING TABLES WHERE SPACING OF ADJACENT BARS IS EQUAL TO OR GREATER THAN 2.5 db.
- b. LAP LENGTHS FOR PLAIN ROUND BARS SHALL BE TWICE THOSE SHOWN IN THE FOLLOWING TABLES.
- c. ALL BEAM MAIN REINFORCEMENT LAP SPLICES SHALL HAVE CRANKED LAPS UNLESS NOTED OTHERWISE.
- d. LAP LENGTHS ARE IN ACCORDANCE WITH NZS 3101.

NOTE. RE: USE OF FOLLOWING TABLES

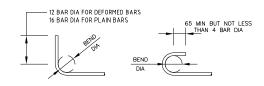
TOP BAR FACTOR IS 1.0 FOR ALL VERTICAL BARS AND FOR HORIZONTAL BARS WITH LESS THAN 300mm OF FRESH CONCRETE CAST BENEATH BAR (TYPICALLY BEAM BOTTOM BARS AND SLAB BARS)

TOP BAR FACTOR IS 1.3 FOR ALL HORIZONTAL BARS WITH MORE THAN 300mm OF FRESH CONCRETE CAST BENEATH THE BAR (TYPICALLY BEAM

				BA	R DIA	METER		
			10	12	16	20	25	32
CONCRETE	30 MPa	TOP BAR FACTOR = 1.3	360	430	575	715	895	1140
STEEL GRADE	300 MPa	BAR FACTOR = 1	300	330	440	550	690	880
CONCRETE	40 MPa	TOP BAR FACTOR = 1.3	310	375	495	620	775	990
STEEL GRADE	300 MPa	BAR FACTOR = 1	300	300	380	475	595	760
CONCRETE	50 MPa	TOP BAR FACTOR = 1.3	300	335	445	555	690	885
STEEL GRADE	300 MPa	BAR FACTOR = 1	300	300	340	425	535	680
CONCRETE	30 MPa	TOP BAR FACTOR = 1.3	595	715	950	1190	1485	1900
STEEL GRADE	500 MPa	BAR FACTOR = 1	460	550	735	915	1145	1485
CONCRETE	40 MPa	TOP BAR FACTOR = 1.3	515	620	825	1030	1290	1645
STEEL GRADE	500 MPa	BAR FACTOR = 1	400	475	635	795	990	1270
CONCRETE	50 MPa	TOP BAR FACTOR = 1.3	465	555	740	920	1150	1475
STEEL GRADE	500 MPa	BAR FACTOR = 1	355	425	570	710	885	1135

4.6 BENDING OF REINFORCEMENT

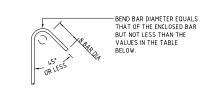
a. BENDS FOR ALL BARS EXCEPT STIRRUPS AND TIES.



STANDARD	HOOK	STANDARD	180*	HOOK

STEEL GRADE	BAR DIAMETER	MINIMUM BEND DIAMETER
GRADE 300	6 TO 20	5 BAR DIAMETERS
RADE 500 FOR CONCRETE STRENGTH QUAL TO OR MORE THAN 40 MPa	25 AND ABOVE	6 BAR DIAMETERS

b. BENDS FOR STIRRUPS AND TIES



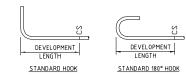
STANDARD STIRRUP & TIE HOOK

STEEL GRADE	BAR DIAMETER	MINIMUM BEND DIAMETER						
STEEL GRADE	DAR DIAMETER	PLAIN BARS	DEFORMED BARS					
GRADE 300/500	6 TO 20	2 BAR DIAMETERS	4 BAR DIAMETERS					
GRADE 300/500	25 TO 32	3 BAR DIAMETERS	6 BAR DIAMETERS					

c. BARS PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE SITE BENT UNLESS SHOWN ON THE DRAWINGS OR SPECIFICALLY APPROVED BY

4.7 REINFORCEMENT ANCHORAGE WITH STANDARD HOOKS

- a. DEVELOPMENT LENGTH PAST CRITICAL SECTION (SHOWN CS ON DRAWING) FOR DEFORMED BARS SHALL BE AS PER TABLE BELOW.
- b. DEVELOPMENT LENGTHS FOR PLAIN ROUND BARS SHALL BE TWICE THOSE SHOWN IN THE FOLLOWING TABLE:



NOTES:

SIDE COVER FACTOR = 0.7

FOR SIDE COVER EQUAL TO OR GREATER THAN 60mm, WITH HOOK COVER NOT LESS THAN 40mm

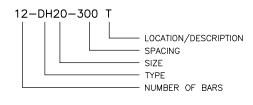
SIDE COVER FACTOR = 1.0

IN ALL OTHER SITUATIONS.

				BA	BAR DIAMETER				
			10	12	16	20	25	32	
CONCRETE	30 MPa	COVER FACTOR = 1	135	160	215	265	330	425	
STEEL GRADE	300 MPa	COVER FACTOR = 0.7	95	115	150	185	235	295	
CONCRETE	40 MPa	COVER FACTOR = 1	115	140	185	230	290	365	
STEEL GRADE	300 MPa	COVER FACTOR = 0.7	85	100	130	165	205	260	
CONCRETE	50 MPa	COVER FACTOR = 1	105	125	165	205	260	330	
STEEL GRADE	300 MPa	COVER FACTOR = 0.7	85	100	130	165	205	260	
CONCRETE	30 MPa	COVER FACTOR = 1	220	265	355	440	550	705	
STEEL GRADE	500 MPa	COVER FACTOR = 0.7	155	185	250	310	385	495	
CONCRETE	40 MPa	COVER FACTOR = 1	195	230	305	380	475	610	
STEEL GRADE	500 MPa	COVER FACTOR = 0.7	135	160	215	270	335	430	
CONCRETE	50 MPa	COVER FACTOR = 1	175	205	275	340	425	545	
STEEL GRADE	500 MPa	COVER FACTOR = 0.7	120	145	195	240	300	385	

NOTE: INTERPOLATE FOR CONCRETE STRENGTHS IN BETWEEN

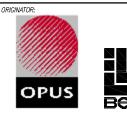
4.8 REINFORCEMENT NOTATION



CHECKED DATE DESIGN DRAWN **APPROVED** This drawing and its contents are the property of Land Transport New Zeoland. Any unauthorised employment or reproduction, in full or in part, is forbidden.

APP'D DATE

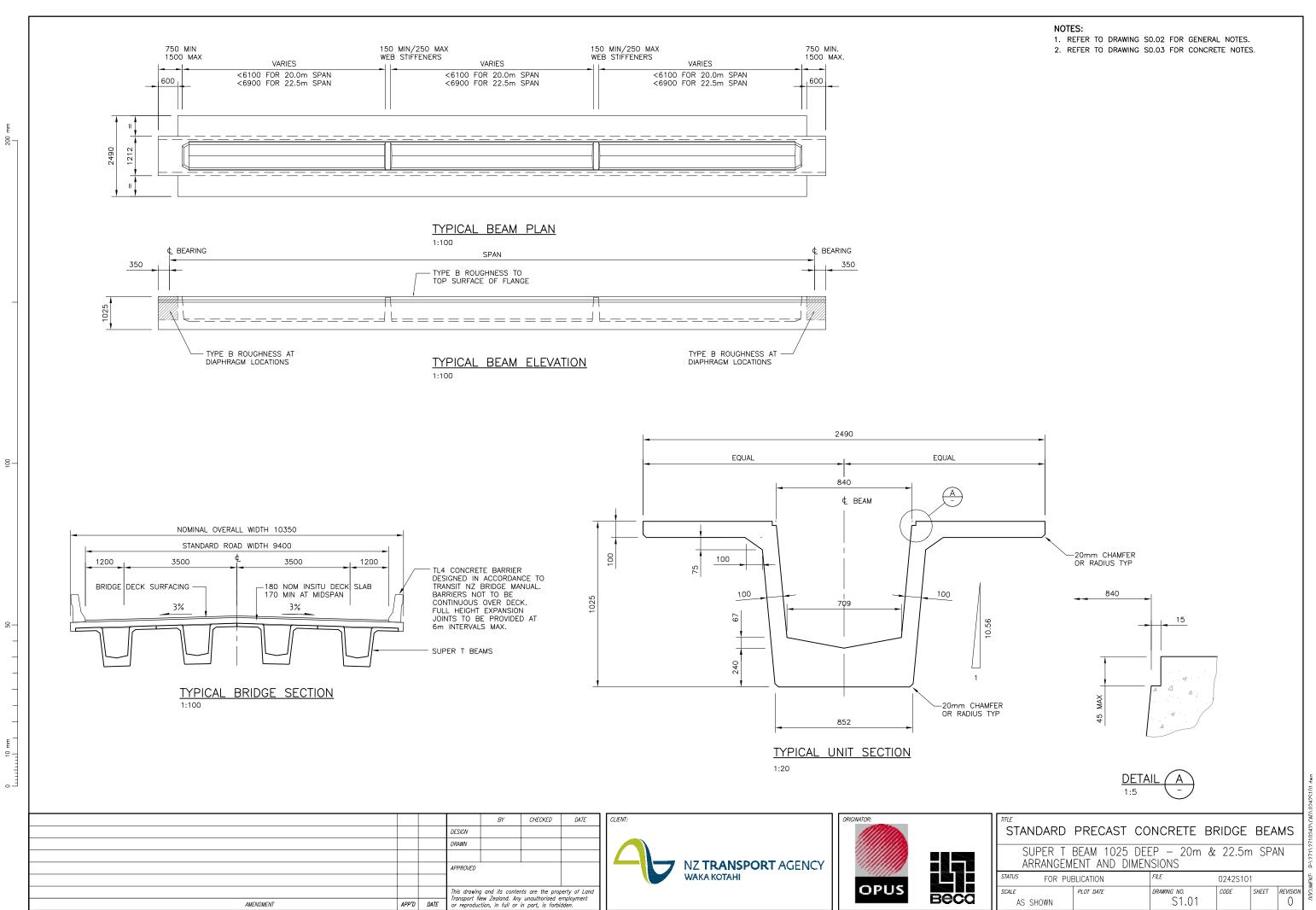




TITLE				
STANDARD	PRECAST	CONCRETE	BRIDGE	BEAMS

ENERAL	
ONCRETE	NOTES

STATUS	FOR PUE	BLICATION	FILE	FILE 0242S003						
SCALE		PLOT DATE	DRAWING NO	0.3	SHEET	REVISION				



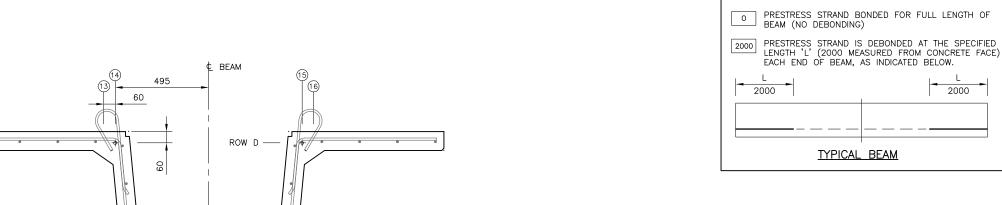
DEBOND LENGTH 'L' mm STRANDS NO. 1 2 4 6 9 10 | 11 | 12 | 16 13 | 14 3 5 8 15 ROW ROW D ROW C 0 0 0 8 0 0 0 2000 0 2000 0 0 0 ROW B 0 10 ROW A 0 0 0 0 0 0 8 TOTAL PER BEAM 28

LEGEND:

NOTE:
THE MANUFACTURERS CAN CHOOSE TO HAVE 2 STRANDS IN ROW D
AND STRESSED TO THE VALUE SPECIFIED IN CLAUSE 2.d ON
DRAWING \$1.05 OR 4 STRANDS IN ROW D AND STRESSED TO 50% OF THE VALUE SPECIFIED IN CLAUSE 2.d ON DRAWING S1.05

STRAND LAYOUT AND DEBONDING SCHEDULE

PRESTRESSING DETAILS - 20m SPAN



STRAND		DEBOND LENGTH 'L' mm												STRANDS			
NO.	13	14	1	2	3	4	5	6	7	8	9	10	11	12	15	16	PER ROW
ROW D		0													0		2
ROW C			0	2500	0	0	0			0	0	0	2500	0		•	10
ROW B			0	0	0	0	0			0	0	0	0	0			10
ROW A			0	0	2500	0	0			0	0	2500	0	0			10
					•					•				TOTAL	. PER	BEAM	32

NOTE:
THE MANUFACTURERS CAN CHOOSE TO HAVE 2 STRANDS IN ROW D
AND STRESSED TO THE VALUE SPECIFIED IN CLAUSE 2.d ON
DRAWING \$1.05 OR 4 STRANDS IN ROW D AND STRESSED TO 50%
OF THE VALUE SPECIFIED IN CLAUSE 2.d ON DRAWING \$1.05

STRAND LAYOUT AND DEBONDING SCHEDULE

PRESTRESSING DETAILS - 22.5m SPAN

				BY	CHECKED	DATE
			DESIGN			
			DRAWN			
			APPROVED			
A A	PG		This drawing and its contents are the property of L Transport New Zealand. Any unauthorised employmen or reproduction, in full or in part, is forbidden.			erty of Land
AMENDMENT	APP'D	DATE	or reproduc	ew zealana. Any tion, in full or	in part, is forbio	mpioyment dden.

7 9 1

275

110

TYPICAL STRAND ARRANGEMENT

1 3 5

275

1:20

— ROW R







STANDARD PRECAST CONCRETE BRIDGE BEAMS

1. REFER TO DRAWING SO.02 FOR GENERAL NOTES.

2. REFER TO DRAWING SO.03 FOR CONCRETE NOTES.

FINISHED

DECK PROFILE

Ф

A2

BEAM PRECAMBER

DESCRIPTION

ESTIMATE HOG OF BEAM

ESTIMATED HOG AT 100 DAYS AFTER TRANSFER

AMENDED DEFLECTION AT

CASTING OF TOP SLAB

PERMITTED TOP SLAB THICKNESS AT MIDSPAN

ESTIMATED INSTANT

AT TRANSFER

DECK SLAB

BEAM

¢ PIER

¢ BEARING

SPAN (m)

180mm ±10mm

22.5

+20mm

+40mm

+20mm

20

+15mm

+35mm

¢ PIER

¢ BEARING

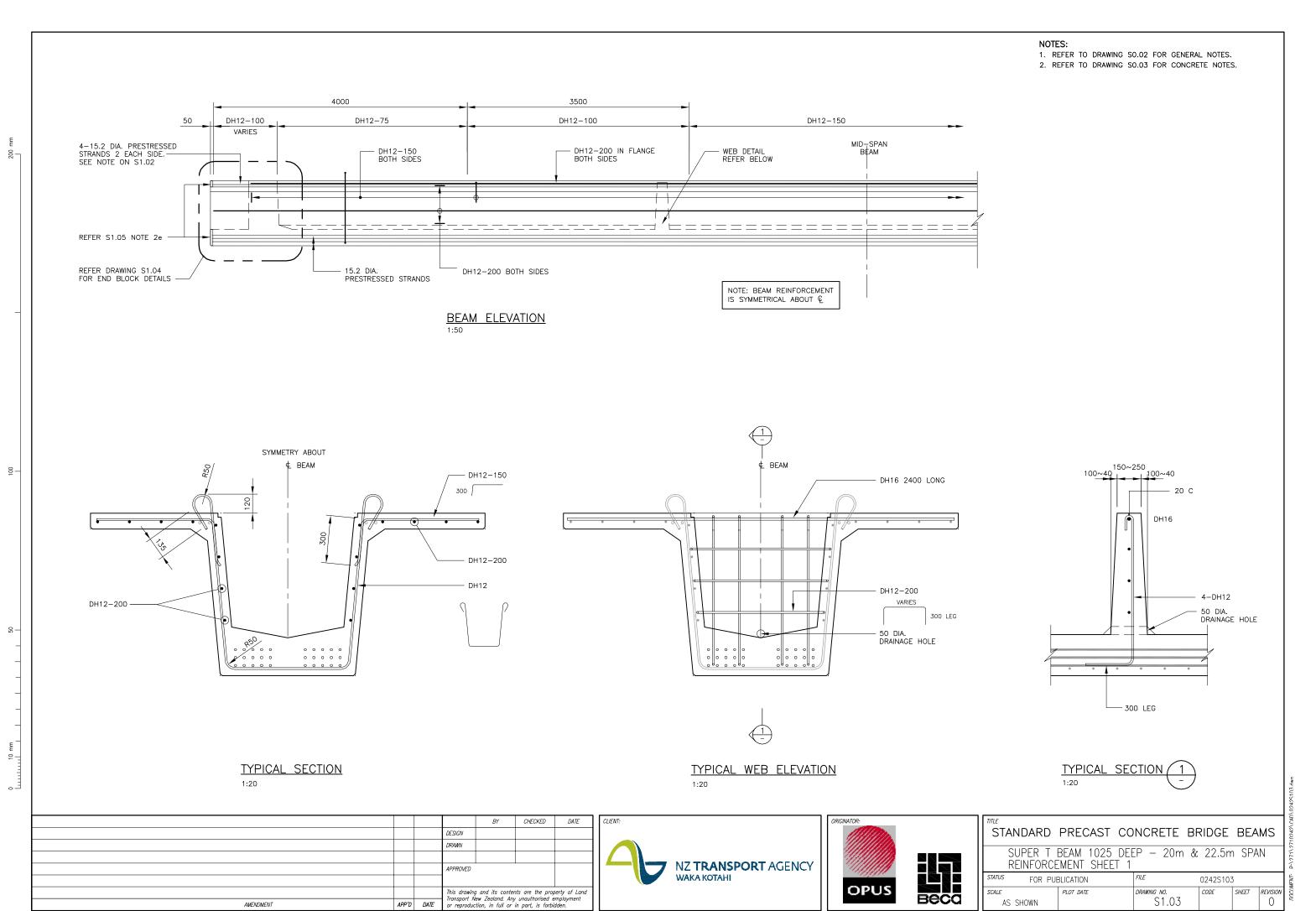
KEY

Α1

SUPER	Τ	BEAM	1025	DEEP	_	20m	&	22.5m	SPAN	
PRESTRESSING DETAILS										

11(2011(2011(0 021)(120											
TATUS	FOR PUE	BLICATION	FILE 0242S102								
CALE 1:20		PLOT DATE	DRAWING NO. \$1.02	CODE SHEET REVIS			moon				

GRAPHIC SCALES



APP'D DATE

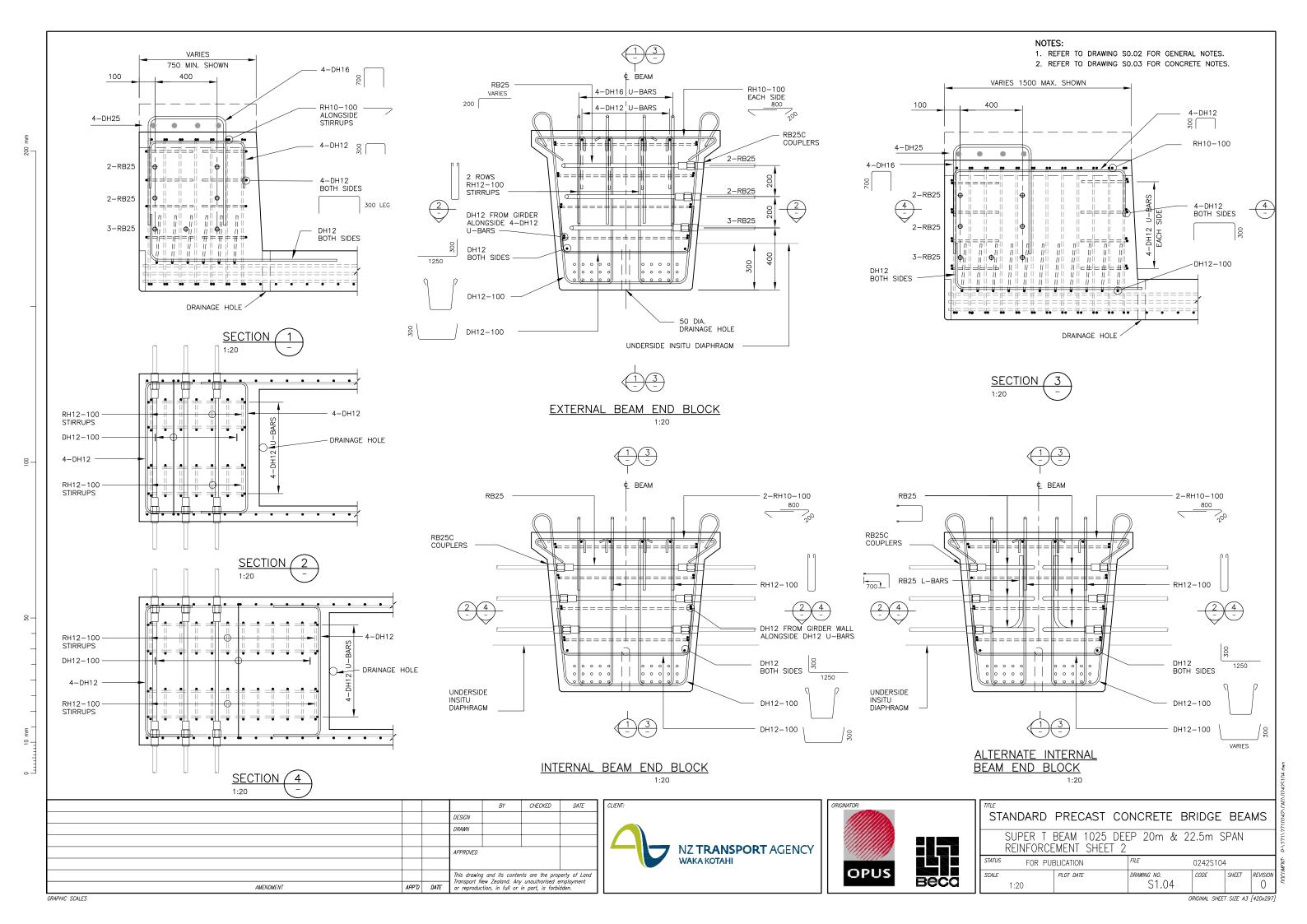
GRAPHIC SCALES

ORIGINAL SHEET SIZE A3 [420x297]

0

S1.03

AS SHOWN



2. REINFORCEMENT & PRESTRESSING a. ALL REINFORCEMENT SHALL BE GRADE 500E TO AS/NZS4671

- b. ALL PRESTRESSING STRAND SHALL BE 15.2mm DIAMETER LOW RELAXATION STRESS RELIEVED SUPER GRADE 7 WIRE STRAND COMPLYING WITH AS/NZS 4672 OR BS 5896
- c. MINIMUM BREAKING LOAD OF STRAND 250 kN
- d. FORCE IN STRANDS IMMEDIATELY PRIOR TO TRANSFER SHALL BE 185 kN. RELAXATION PRIOR TO TRANSFER SHALL BE ACCOUNTED FOR IN THE JACKING FORCE REQUIRED TO ACHIEVE THIS VALUE. TYPICALLY RELAXATION PRIOR TO TRANSFER IS IN THE ORDER OF 1%. WHERE CURING AT ELEVATED TEMPERATURES IS EMPLOYED, HIGHER RELAXATION RATES MAY RESULT AND DUE ALLOWANCE FOR THIS SHALL BE MADE BY THE PRECASTER IN DETERMINING THE JACKING FORCE REQUIRED TO ACHIEVE THE MINIMUM FORCE STATED ABOVE.
- e. ENDS OF STRAND SHALL BE CUT FLUSH AND PROTECTED WITH A MINIMUM OF 5mm
- f. UPWARD DEFLECTION OF GIRDERS DUE TO PRESTRESS IS GIVEN IN THE BEAM HOG TABLE. THESE ARE ESTIMATES ONLY. ESTIMATES ARE MADE FOR HOG AT TRANSFER AND AT 100 DAYS WITH DUE ALLOWANCE FOR INCREASE IN HOG DUE TO CREEP OF CONCRETE UNDER SUSTAINED LOAD.
- g. COMPONENTS PREFIXED RB ARE REIDBAR ITEMS. REIDBAR SHALL BE GRADE 500E TO

3. CONCRETE COVER (MINIMUM)

COVER TO ALL PRESTRESSING COMPONENTS - 40mm COVER TO ALL REINFORCEMENT EXPOSED SURFACE - 40mm COVER TO ALL REINFORCEMENT INTERNAL SURFACE - 30mm COVER ADJACENT TO CORED HOLES - 30mm COVER TO BRIDGE DECK & ALL CAST INSITU CONCRETE - 50mm COVER TO BARRIER FIXING STEEL (WITHIN BARRIER)

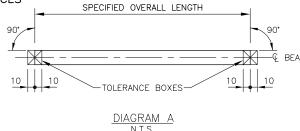
4. DESIGN LOADING

HN-H0-72 (INCLUDING SLS LIVE LOAD FACTOR OF 1.35)

5. SPECIFICATION

THIS DESIGN IS BASED ON LTNZ STANDARD BRIDGE BEAM SPECIFICATION (2006)

6. TOLERANCES



6.1. DIMENSIONS AT TIME OF ERECTION

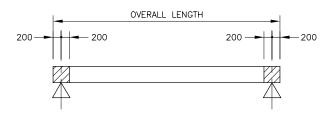
ACTUAL OVERALL LENGTH AND SQUARENESS

- a. THE UNDERSIDE OF THE BEAM FOR THE END 700mm SHALL BE TRUE PLANE.
- THE BEAM END SURFACES SHALL LIE WITHIN THE "TOLERANCE BOXES" SHOWN IN DIAGRAM A
- c. PLANE SURFACE, DEVIATION FROM A 1.5m STRAIGHT EDGE
- d. BEAM HOGGING (SEE SPECIFICATION)
- CROSS SECTION DIMENSIONS UP TO 0.5m ... CROSS SECTION DIMENSIONS 0.5m TO 2.0m +10mm
- g. HORIZONTAL BOW OF LONGITUDINAL AXIS

6.1. DIMENSIONS AT TIME OF ERECTION a. LONGITUDINAL STEEL ARRANGEMENT LOCATION OF AN ITEM IN RELATION TO ANY OTHER ITEM WITHIN ITS GROUP OR TO

7. HANDLING

EXTREMES OF VERTICAL LIFTING POINTS OR GROUND SUPPORT SHOWN HATCHED. CENTRAL SUPPORT POSITION AS SHOWN IS PREFERRED (BEAM IN UPRIGHT POSITION AT ALL TIMES)



BEAM SUPPORT & LIFTING POINTS NITS

8. METHOD OF MANUFACTURE

BEAMS SHALL BE MANUFACTURED UNDER FACTORY CONDITIONS

9. SURFACE FINISHES

BEAMS

CHECKED

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DESIGN DRAWN

APPROVED

DATE

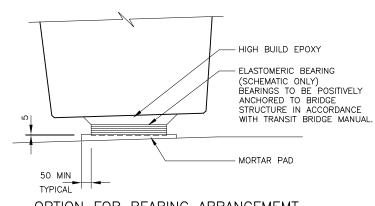
CLIENT

TOP SURFACE OF FLANGE TYPE B CONSTRUCTION JOINT IN DIRECT CONTACT WITH INSITU DIAPHRAGM TYPE B CONSTRUCTION JOINT HIDDEN FORMED SURFACE F1 ALL OTHER FORMED SURFACE

DIAPHRAGM SURFACE FINISH TO BE BASIC FINISH IN ACCORDACE WITH LTNZ STANDARD BEAM SPECIFICATION (2006)

10. BEARING DESIGN DATA

SPAN		REACTION (kN)		ROTATION (x 10 ⁻³ RADIANS)				
(m)	DEAD LOAD (UNFACTORED)	LIVE LOAD (1.35 HN x I)	OVERLOAD (HO x I)	LIVE LOAD (1.35 HN x I)	OVERLOAD (HO x I)			
20	430	465	570	1.7	2.0			
22.5	475	485	590	2.2	2.5			

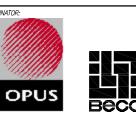


OPTION FOR BEARING ARRANGEMENT

ORIGINATOR: **OPUS**

NZ TRANSPORT AGENCY

WAKA KOTAHI



50 ID SLEEVE CAST INTO ABUTMENT

WALL & DIAPHRAGM

RB32 GALV. -

TO SUIT SEISMIC REQUIREMENTS AS PER OTHER SIDE EXCEPT NO GAP BETWEEN WALL & RUBBER PAD — RB NUT & RB HALF NUT GALV. -150x150x32 GALV. PLATE 150x150x50 RUBBER PAD GLUED GAP TO SUIT SPAN ARRANGEMENT

OPTION FOR LINKAGE BAR DETAIL 1:20

STATUS

ABUTMENT BEAM DIAPHRAGM

STANDARD PRECAST CONCRETE BRIDGE BEAMS

1. REFER TO DRAWING SO.02 FOR GENERAL NOTES.

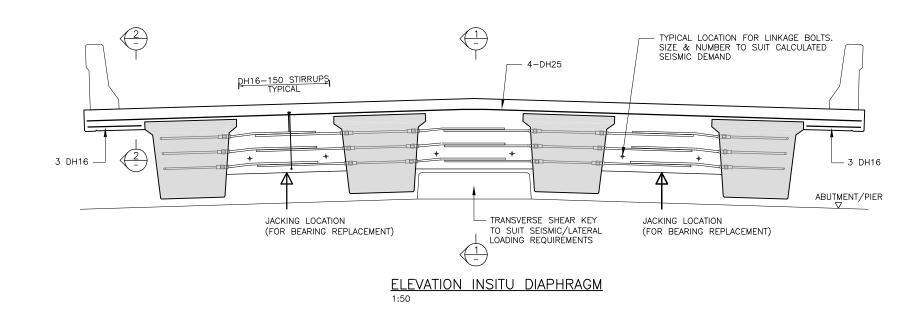
2. REFER TO DRAWING SO.03 FOR CONCRETE NOTES.

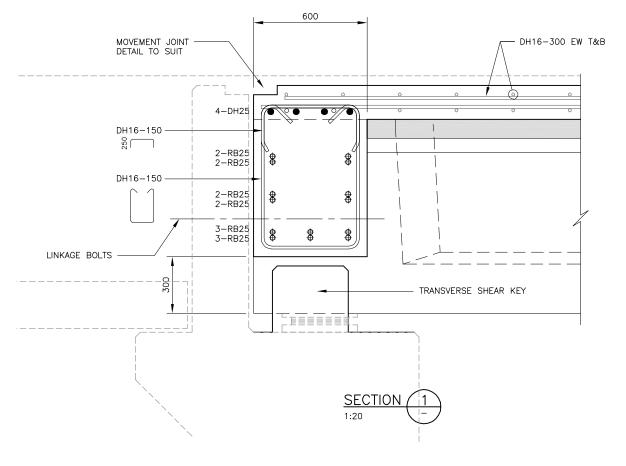
SUPER T BEAM 1025 DEEP - 20m & 22.5m SPAN UNIT DATA

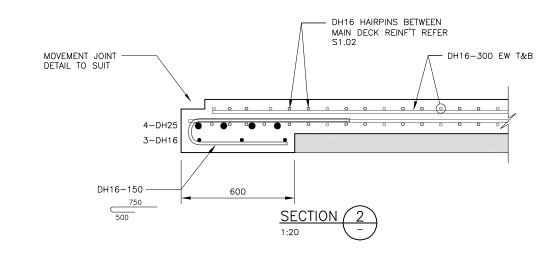
5	FOR PU	BLICATION	FILE 0242S105						
AS SH	OWN	PLOT DATE	drawing no. \$1.05	CODE	SHEET	REVISION O	III		



- 1. REFER TO DRAWING SO.02 FOR GENERAL NOTES.
- 2. REFER TO DRAWING SO.03 FOR CONCRETE NOTES.







				BY	CHECKED	DATE	
			DESIGN				
			DRAWN				
			APPROVED				
			1				
			DATE This drawing and its contents are the property of Land Transport New Zealand. Any unauthorised employment or reproduction, in full or in part, is forbidden.				
AMENDMENT	APP'D	DATE					



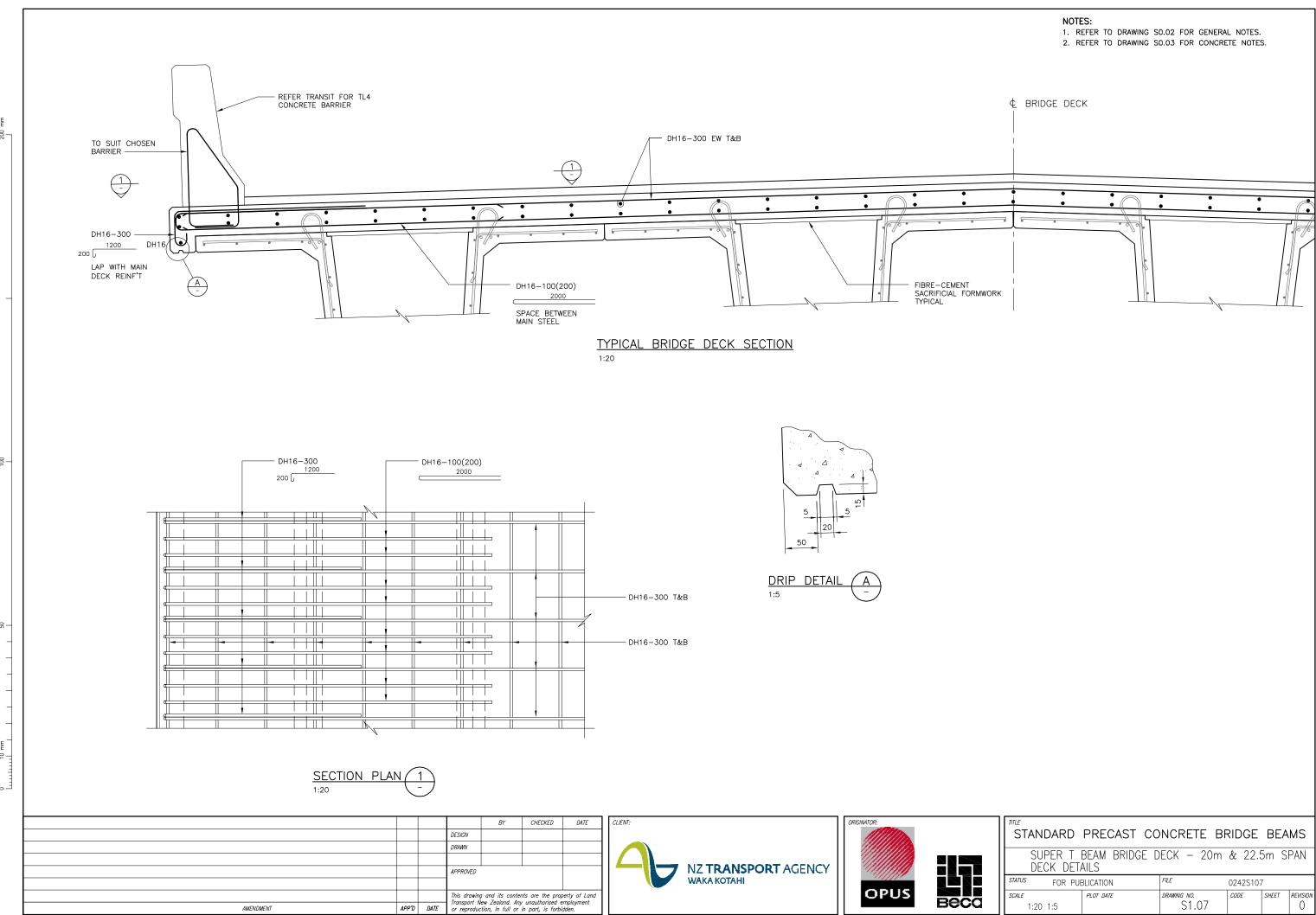


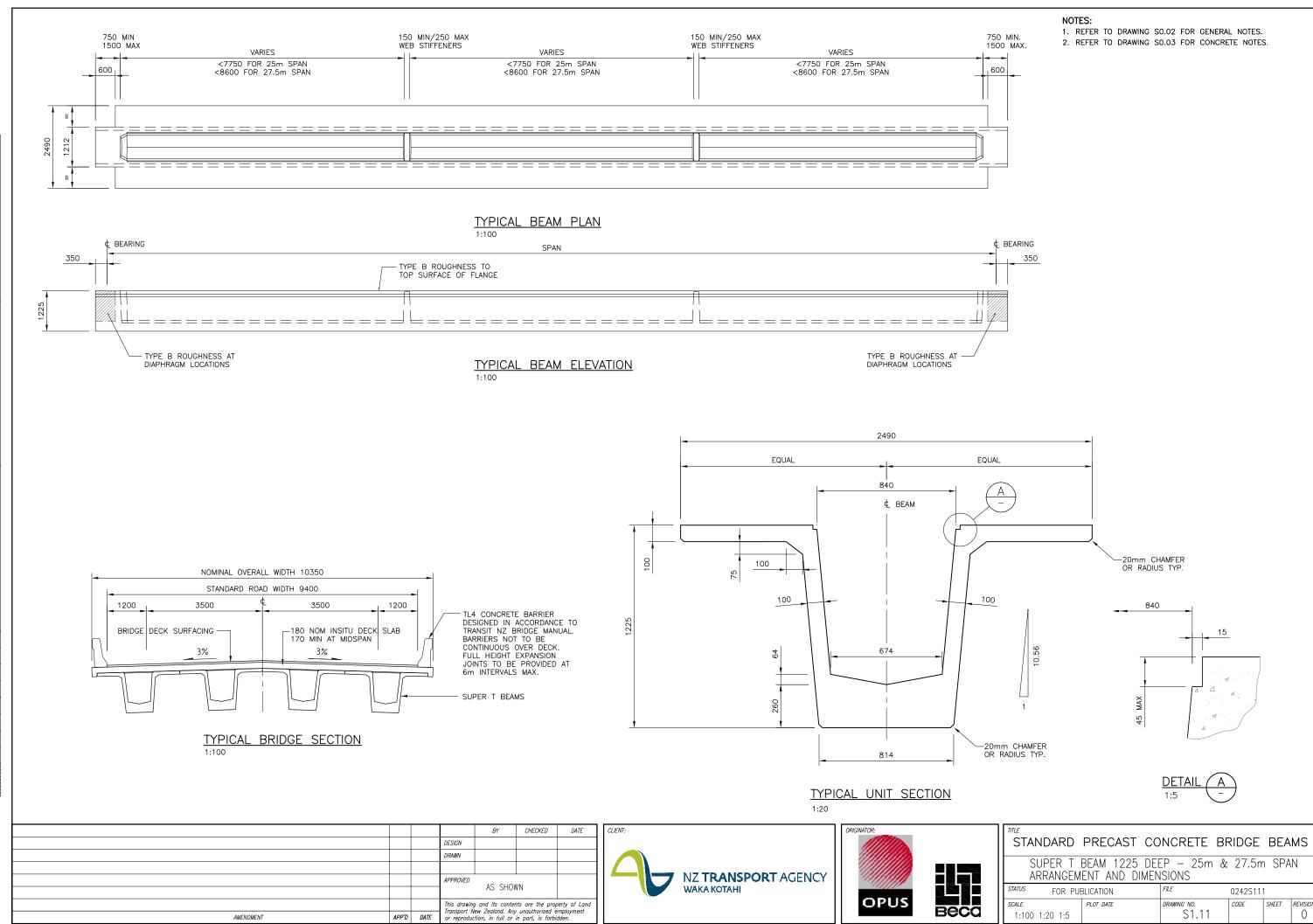


ı	TITLE			
l	CTANIDARD	PRFCAST	CONCRETE	BRIDGE BEAMS
l	JIMUMILU	INLUASI	CONCILL	DIVIDOR DEAMS
ŀ				
ı	SLIPER T	RFAM 1025	DFFP = 20m	& 225m SPAN

SUPER T BEAM 1025 DEEP — 20m & 22.5m SPAN INSITU END DIAPHRAGM DETAILS

us FOR PUB	LICATION	FILE	0242S1	06		WFNT.
£ 1:50, 1:20	PLOT DATE	DRAWING NO.	CODE	SHEET	REVISION	מטכווו



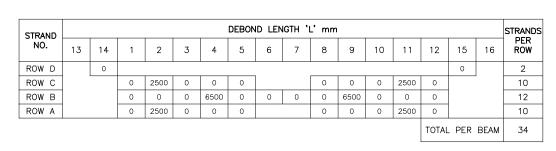


€ PIER

& BEARING

KEY

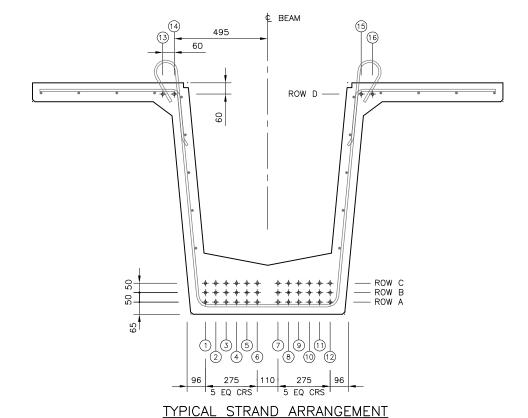
- 1. REFER TO DRAWING SO.02 FOR GENERAL NOTES.
- 2. REFER TO DRAWING SO.03 FOR CONCRETE NOTES.



NOTE: THE MANUFACTURERS CAN CHOOSE TO HAVE 2 STRANDS IN ROW D AND STRESSED TO THE VALUE SPECIFIED IN CLAUSE 2.d ON DRAWING S1.15 OR 4 STRANDS IN ROW D AND STRESSED TO 50% OF THE VALUE SPECIFIED IN CLAUSE 2.d ON DRAWING S1.15

STRAND LAYOUT AND DEBONDING SCHEDULE

PRESTRESSING DETAILS - 25m SPAN



¢ BEAM

ROW D -

7 9 0

275 96 S EQ CRS

— ROW B

495

99

1 3 5

5 EQ CRS

275 110

TYPICAL STRAND ARRANGEMENT

60

LEGEND:							
O PRESTRESS STRAND BO BEAM (NO DEBONDING)	ONDED FOR FULL LENGTH OF						
PRESTRESS STRAND IS DEBONDED AT THE SPECIFIED LENGTH 'L' (5000 MEASURED FROM CONCRETE FACE) EACH END OF BEAM, AS INDICATED BELOW.							
L 5000							
TYPICAL	BEAM						

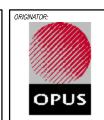
STRAND		DEBOND LENGTH 'L' mm									STRANDS						
NO.	13	14	1	2	3	4	5	6	7	8	9	10	11	12	15	16	PER ROW
ROW D	0	0											•		0	0	4
ROW C			0	7000	0	0	0	0	0	0	0	0	7000	0			12
ROW B			0	0	0	3000	0	3000	3000	0	3000	0	0	0			12
ROW A			0	7000	0	0	0	0	0	0	0	0	7000	0			12
													•	TOTAL	PER	ВЕАМ	40

STRAND LAYOUT AND DEBONDING SCHEDULE

PRESTRESSING DETAILS - 27.5m SPAN

				BY	CHECKED	DATE	
			DESIGN				
			DRAWN				
			APPROVED				
			This drawing and its contents are the property of Land Transport New Zealand. Any unauthorised employment or reproduction, in full or in part, is forbidden.				
AMENDMENT	APP'D	DATE					







A2 ESTIMATED HOG AT 100 DAYS AFTER TRANSFER +40mm +50mm ESTIMATED INSTANT AMENDED DEFLECTION AT CASTING OF TOP SLAB C PERMITTED TOP SLAB THICKNESS AT MIDSPAN 180mm ±10mm

BEAM PRECAMBER

DESCRIPTION

ESTIMATE HOG OF BEAM

AT TRANSFER

STANDARD PRECAST CONCRETE BRIDGE BEAMS

SUPER T BEAM 1225 DEEP - 25m & 27.5m SPAN	
PRESTRESSING DETAILS	

¢ PIER

¢ BEARING

27.5

+30mm

SPAN (m)

25

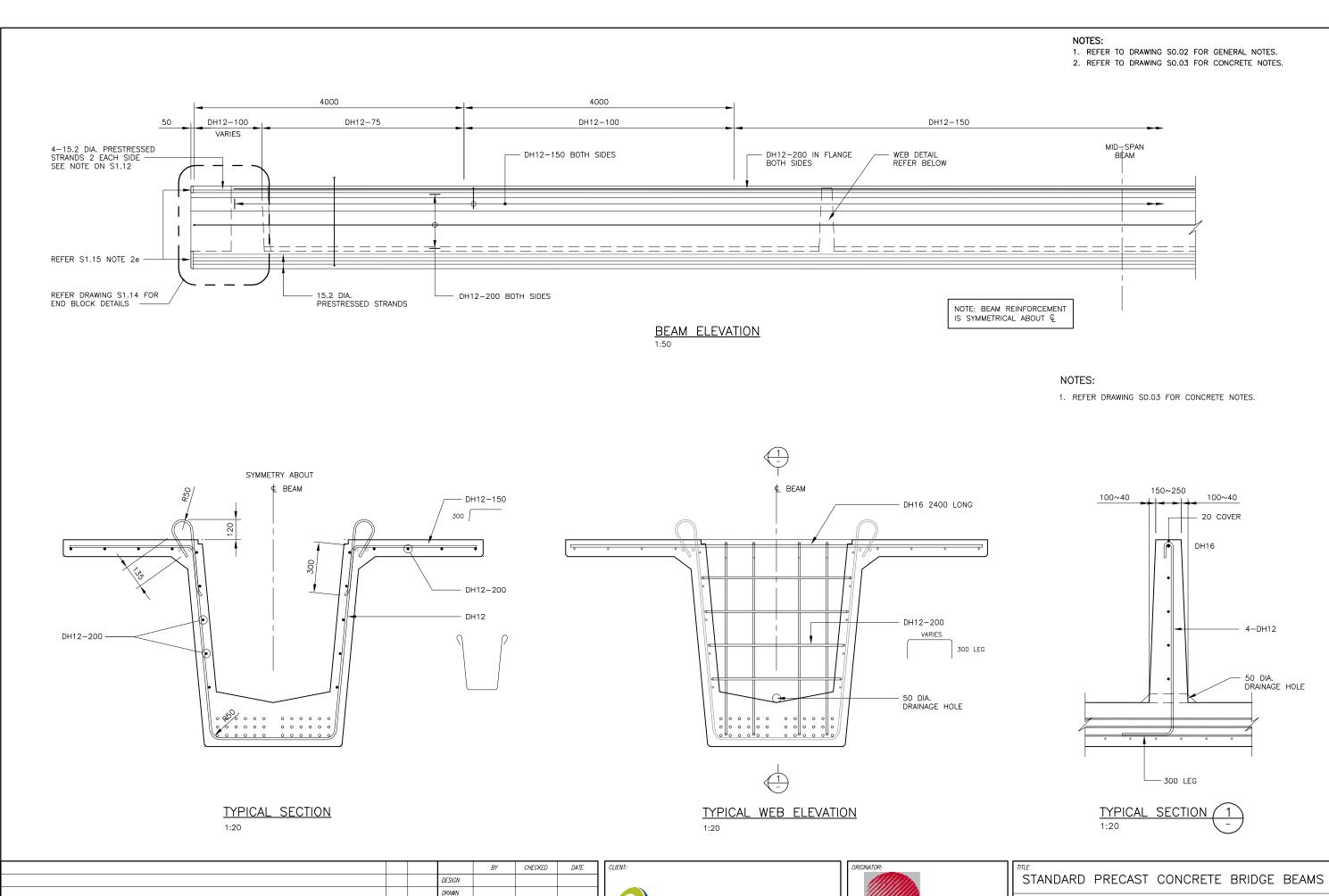
+20mm

FINISHED

DECK PROFILE

DECK SLAB
BEAM

tatus for pue	BLICATION	FILE 0242S112				WFNT.
1:20	PLOT DATE	DRAWING NO. \$1.12	CODE	SHEET	REVISION	игооа



DESIGN

DRAWN

APPROVED

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Transport New Zealand. Any unauthorised employment or reproduction, in full or in part, is forbidden.

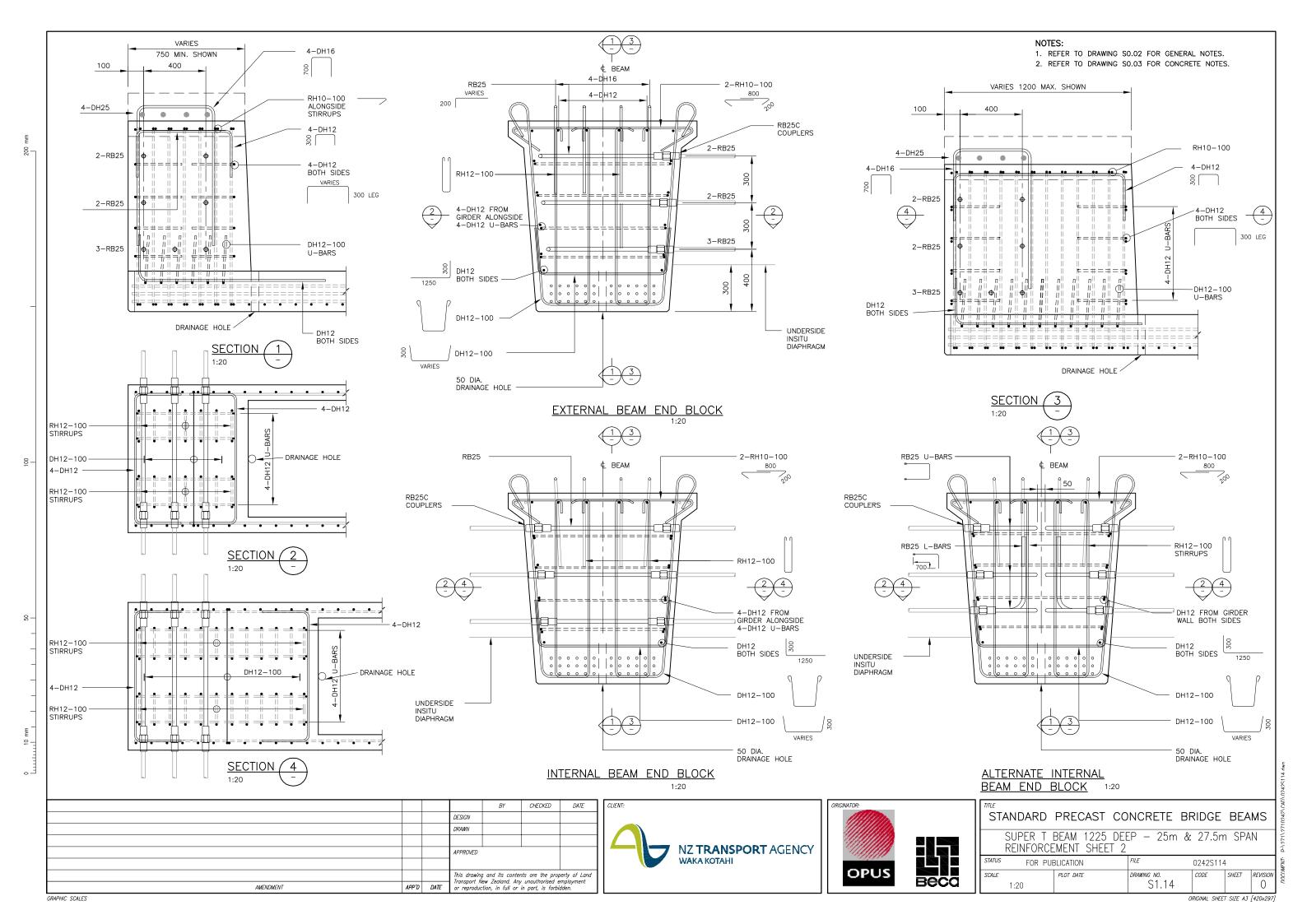






SUPER T BEAM 1225 DEEP — 25m & 27.5m SPAN REINFORCEMENT SHEET 1

STATUS	FOR PUE	BLICATION	FILE		MFNT.		
<i>SCALE</i> 1:50	1:20	PLOT DATE	DRAWING NO. \$1.13	CODE	SHEET	REVISION O	WCCIN



a. ALL REINFORCEMENT SHALL BE GRADE 500E TO AS/NZS4671

b. ALL PRESTRESSING STRAND SHALL BE 15.2mm DIAMETER LOW RELAXATION STRESS RELIEVED SUPER GRADE 7 WIRE STRAND COMPLYING WITH AS/NZS 4672 OR BS 5896

c. MINIMUM BREAKING LOAD OF STRAND 250 kN

d. FORCE IN STRANDS IMMEDIATELY PRIOR TO TRANSFER SHALL BE 185 kN. RELAXATION PRIOR TO TRANSFER SHALL BE ACCOUNTED FOR IN THE JACKING FORCE REQUIRED TO ACHIEVE THIS VALUE. TYPICALLY RELAXATION PRIOR TO TRANSFER IS IN THE ORDER OF 1%. WHERE CURING AT ELEVATED TEMPERATURES IS EMPLOYED, HIGHER RELAXATION RATES MAY RESULT AND DUE ALLOWANCE FOR THIS SHALL BE MADE BY THE PRECASTER IN DETERMINING THE JACKING FORCE REQUIRED TO ACHIEVE THE MINIMUM FORCE STATED ABOVE.

e. ENDS OF STRAND SHALL BE CUT FLUSH AND PROTECTED WITH A MINIMUM OF 5mm

f. UPWARD DEFLECTION OF GIRDERS DUE TO PRESTRESS IS GIVEN IN THE BEAM HOG TABLE. THESE ARE ESTIMATES ONLY. ESTIMATES ARE MADE FOR HOG AT TRANSFER AND AT 100 DAYS WITH DUE ALLOWANCE FOR INCREASE IN HOG DUE TO CREEP OF CONCRETE UNDER SUSTAINED LOAD.

g. COMPONENTS PREFIXED RB ARE REIDBAR ITEMS. REIDBAR SHALL BE GRADE 500E TO

3. CONCRETE COVER (MINIMUM)

COVER TO ALL PRESTRESSING COMPONENTS - 40mm COVER TO ALL REINFORCEMENT EXPOSED SURFACE - 40mm COVER TO ALL REINFORCEMENT INTERNAL SURFACE - 30mm COVER ADJACENT TO CORED HOLES - 30mm COVER TO BRIDGE DECK & ALL CAST INSITU CONCRETE - 50mm COVER TO BARRIER FIXING STEEL (WITHIN BARRIER)

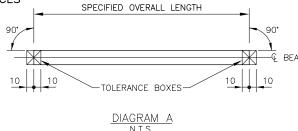
4. DESIGN LOADING

HN-H0-72 (INCLUDING SLS LIVE LOAD FACTOR OF 1.35)

5. SPECIFICATION

THIS DESIGN IS BASED ON LTNZ STANDARD BRIDGE BEAM SPECIFICATION (2006)

6. TOLERANCES



6.1. DIMENSIONS AT TIME OF ERECTION

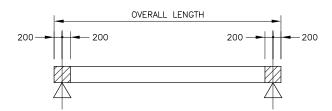
ACTUAL OVERALL LENGTH AND SQUARENESS

- a. THE UNDERSIDE OF THE BEAM FOR THE END 700mm SHALL BE TRUE PLANE.
- b. THE BEAM END SURFACES SHALL LIE WITHIN THE "TOLERANCE BOXES" SHOWN IN DIAGRAM A
- c. PLANE SURFACE, DEVIATION FROM A 1.5m STRAIGHT EDGE
- d. BEAM HOGGING (SEE SPECIFICATION)
- CROSS SECTION DIMENSIONS UP TO 0.5m CROSS SECTION DIMENSIONS 0.5m TO 2.0m
- HORIZONTAL BOW OF LONGITUDINAL AXIS

- 6.1. DIMENSIONS AT TIME OF ERECTION a. LONGITUDINAL STEEL ARRANGEMENT b. LOCATION OF AN ITEM IN RELATION TO ANY OTHER ITEM WITHIN ITS GROUP OR TO

7. HANDLING

EXTREMES OF VERTICAL LIFTING POINTS OR GROUND SUPPORT SHOWN HATCHED. CENTRAL SUPPORT POSITION AS SHOWN IS PREFERRED (BEAM IN UPRIGHT POSITION AT ALL TIMES)



BEAM SUPPORT & LIFTING POINTS NITS

8. METHOD OF MANUFACTURE

BEAMS SHALL BE MANUFACTURED UNDER FACTORY CONDITIONS

9. SURFACE FINISHES

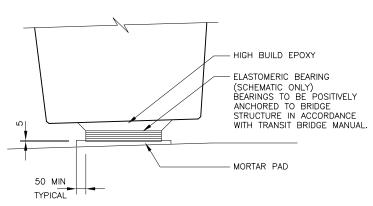
BEAMS

TOP SURFACE OF FLANGE TYPE B CONSTRUCTION JOINT IN DIRECT CONTACT WITH INSITU DIAPHRAGM TYPE B CONSTRUCTION JOINT HIDDEN FORMED SURFACE F1 ALL OTHER FORMED SURFACE

DIAPHRAGM SURFACE FINISH TO BE BASIC FINISH IN ACCORDACE WITH LTNZ STANDARD BEAM SPECIFICATION (2006)

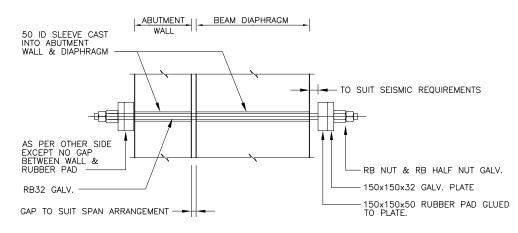
10. BEARING DESIGN DATA

SPAN		REACTION (kN)	ROTATION (x 10 ⁻³ RADIANS)				
(m)	DEAD LOAD (UNFACTORED)	LIVE LOAD (1.35 HN x I)			OVERLOAD (HO x I)		
25	540	510	610	2.0	2.1		
27.5	585	530	630	2.5	2.7		



OPTION FOR BEARING ARRANGEMENT

1:20

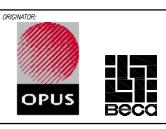


OPTION FOR LINKAGE BAR DETAIL

1:20

				BY	CHECKED	DATE
			DESIGN			
			DRAWN			
			APPROVED			
			This drawing and its contents are the property Transport New Zealand. Any unauthorised emple or reproduction, in full or in part, is forbidden			oerty of Land
AMENDMENT	APP'D	DATE				empioyment idden.





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STANDARD	PRFCAST	CONCRETE	RRIDGE	REAMS
SIMIDAND	INLOASI	CONCINETE	DIVIDUL	DLAIVIO
CLIDED T	DEAM 100E	DEED 25.00	0. 07 5	CDAN

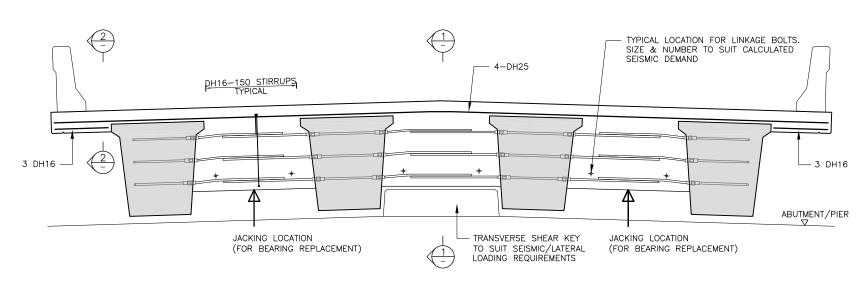
1. REFER TO DRAWING SO.02 FOR GENERAL NOTES.

2. REFER TO DRAWING SO.03 FOR CONCRETE NOTES.

SUPER T	BEAM	1225	DEEP	_	25m	&	27.5m	SPAN
UNIT DATA	4							

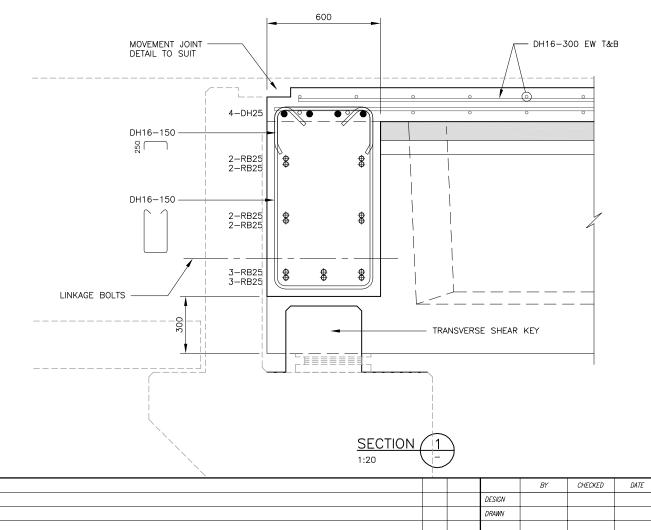
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4LE	PLOT DATE	DRAWING NO.	CODE	SHEET	REVISION	טכווו
AS SHOWN		S1.15			0	U

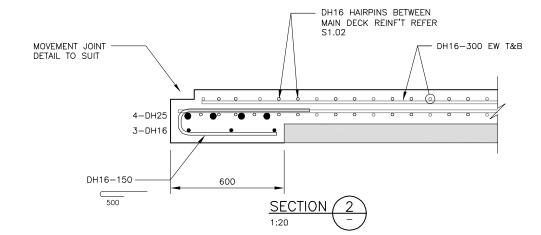
- 1. REFER TO DRAWING SO.02 FOR GENERAL NOTES.
- 2. REFER TO DRAWING SO.03 FOR CONCRETE NOTES.



ELEVATION INSITU DIAPHRAGM 1:50

APPROVED







CLIENT:

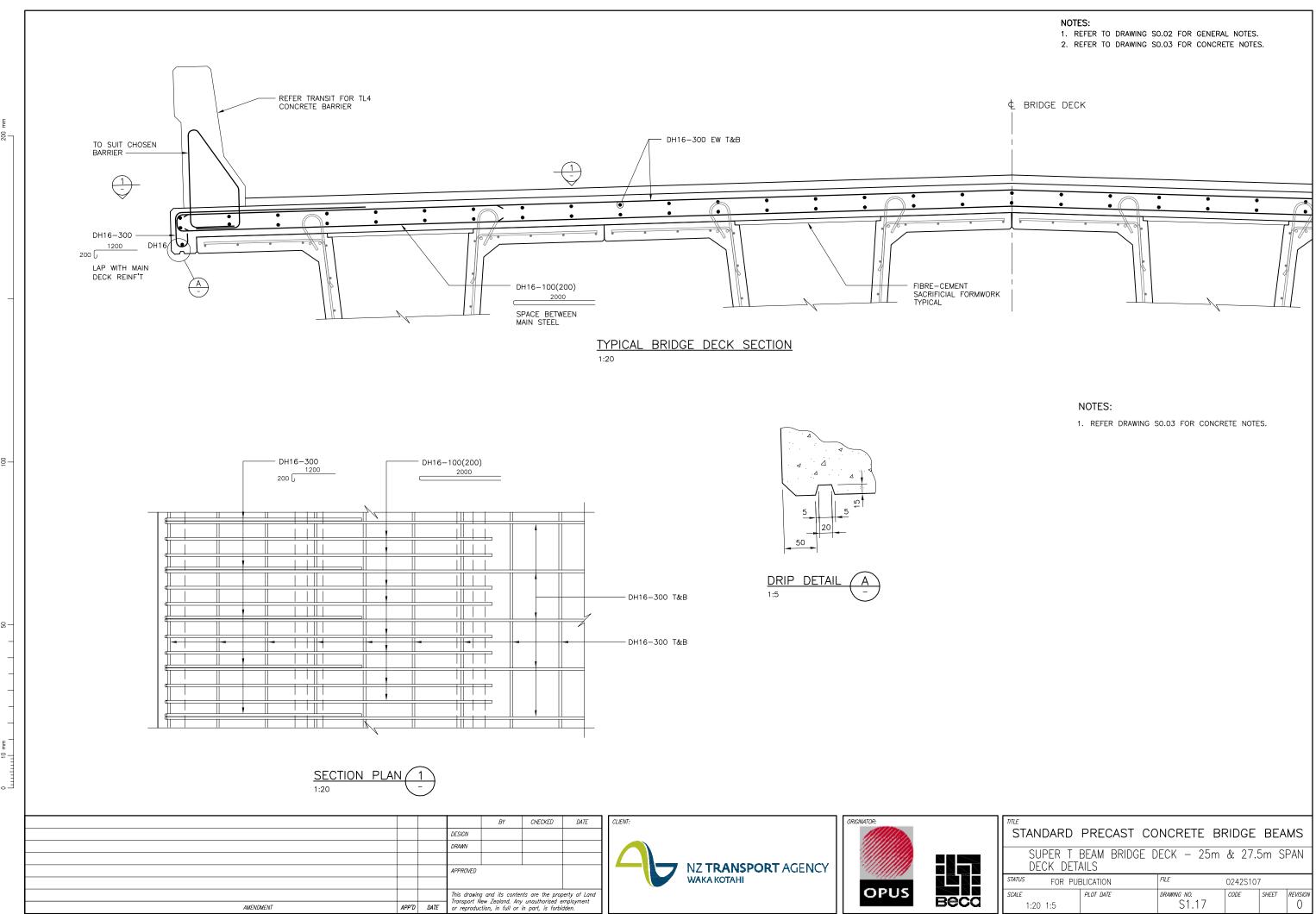


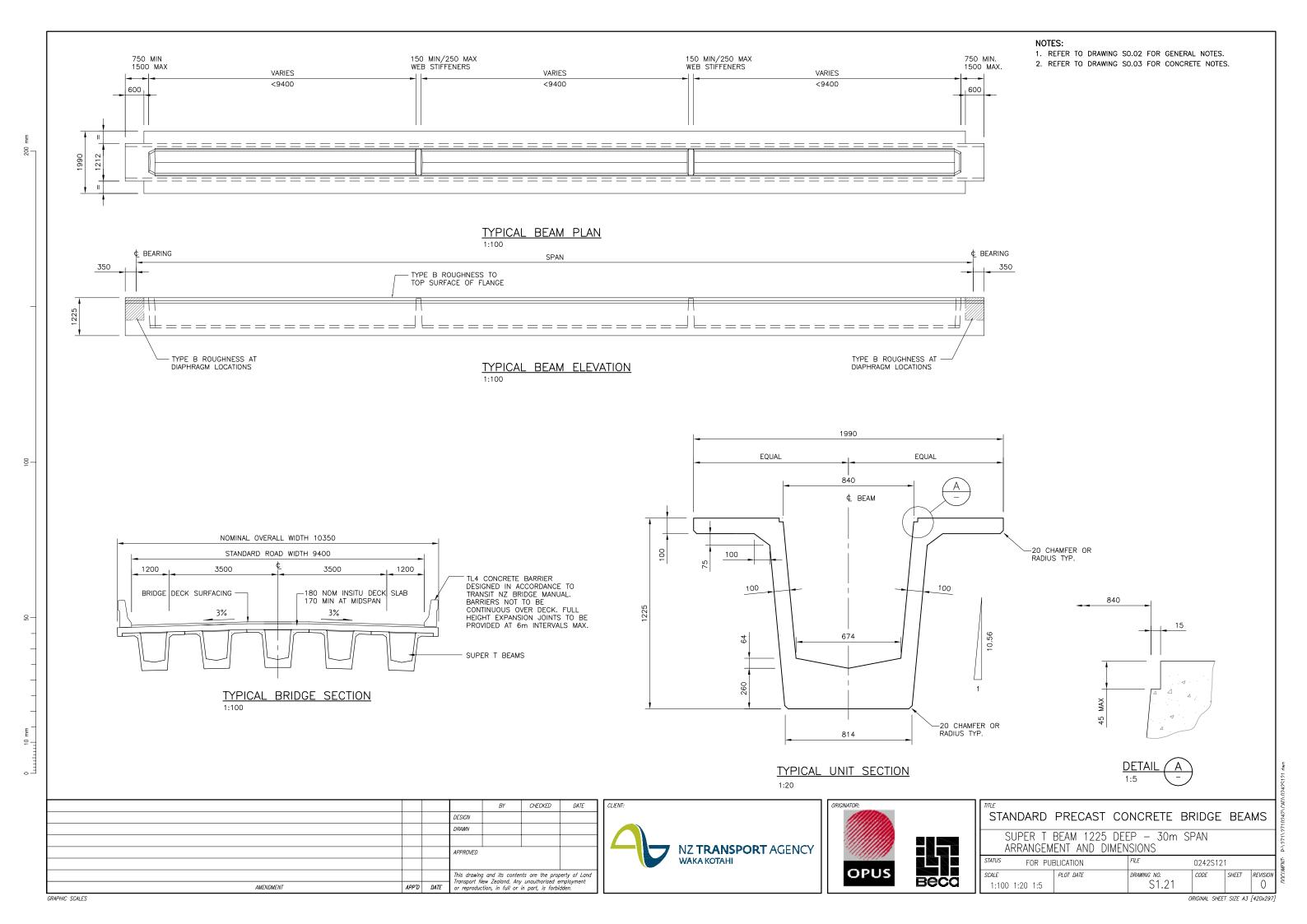
Beca

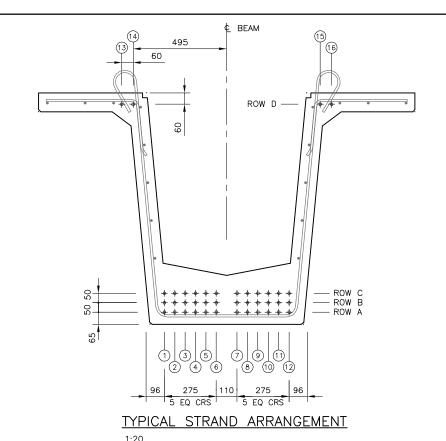
STANDARD PRECAST CONCRETE BRIDGE BEAMS

SUPER T BEAM 1225 DEEP - 25m & 27.5m SPAN END DIAPHRAGM DETAILS

TATUS	FOR PUE	BLICATION	FILE 0242S115				WFNT.
CALE 1:50	1:20	PLOT DATE	DRAWING NO.	CODE	SHEET	REVISION	пост
1.50	1.20		31.10			0	



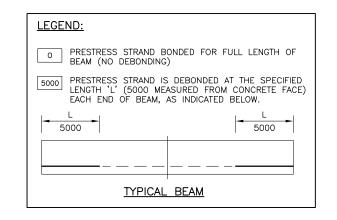




STRAND	DEBOND LENGTH 'L' mm								STRANDS								
NO.	13	14	1	2	3	4	5	6	7	8	9	10	11	12	15	16	PER ROW
ROW D	0	0													0	0	4
ROW C			0	2500	0	5000	0	0	0	0	5000	0	2500	0			12
ROW B			0	5000	0	2500	0	0	0	0	2500	0	5000	0			12
ROW A			0	0	0	0	0	0	0	0	0	0	0	0			12
														TOTAL	. PER	BEAM	40

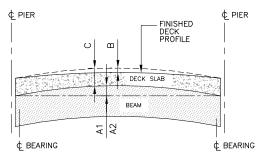
STRAND LAYOUT AND DEBONDING SCHEDULE

PRESTRESSING DETAILS 30m SPAN



NOT

- 1. REFER TO DRAWING SO.02 FOR GENERAL NOTES.
- 2. REFER TO DRAWING SO.03 FOR CONCRETE NOTES.



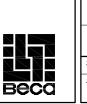
BEAM PRECAMBER

KEY	DESCRIPTION	SPAN (m)		
		30		
A1	ESTIMATE HOG OF BEAM AT TRANSFER	+35mm		
A2	ESTIMATED HOG AT 100 DAYS AFTER TRANSFER	+65mm		
В	ESTIMATED INSTANT AMENDED DEFLECTION AT CASTING OF TOP SLAB	+30mm		
С	PERMITTED TOP SLAB THICKNESS AT MIDSPAN	180mm ±10mm		

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			DESIGN			
			DRAWN			
			APPROVED			
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AMENDMENT	APP'D	DATE				



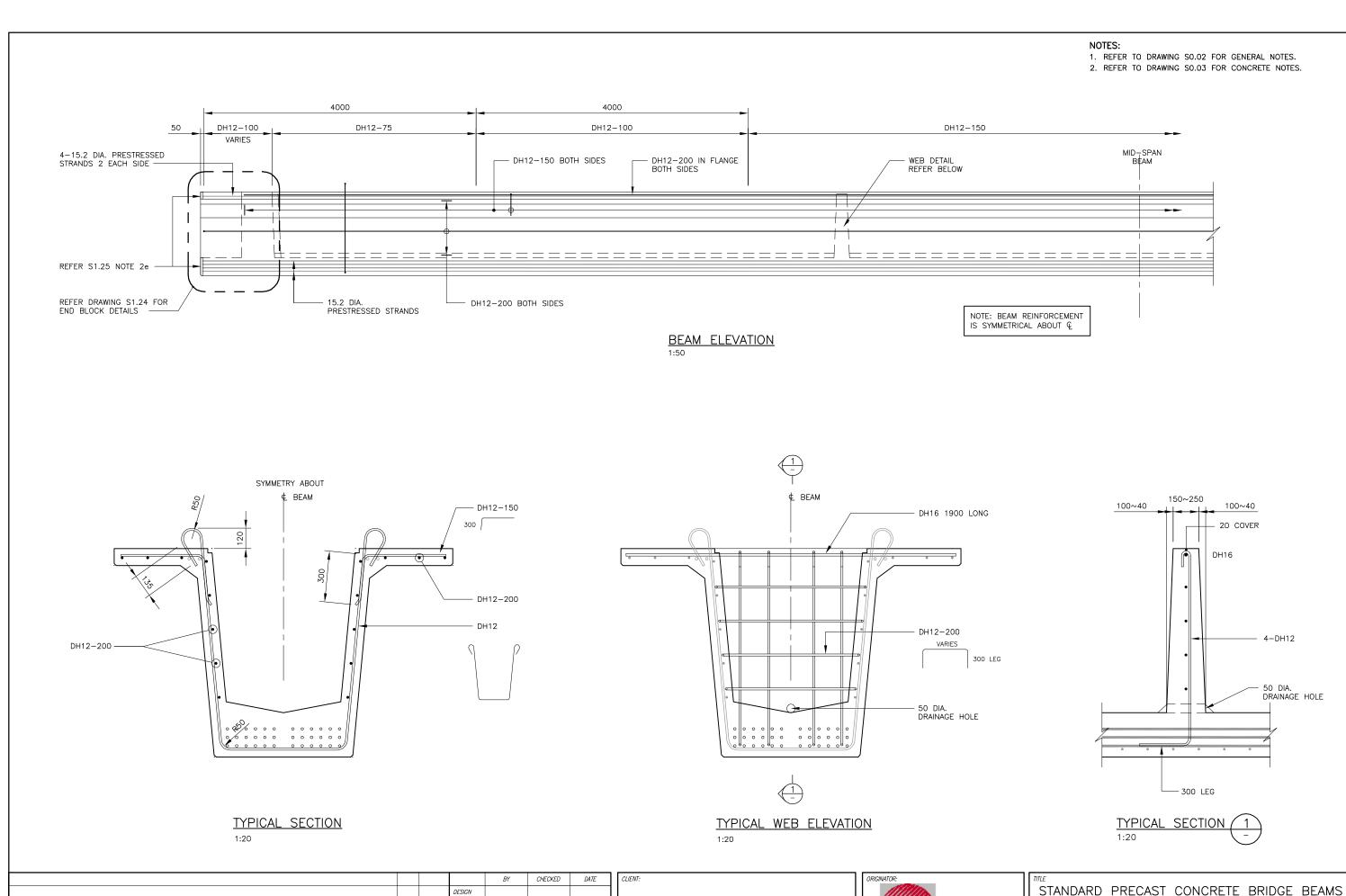




STANDARD	PRECAST	CONCRETE	BRIDGE BEAMS
SUPER T	BEAM 1225	DEEP - 30m	SPAN

RESTRESSING DETAILS	
NESTINESSING DETNIES	

ATUS	FOR PUE	BLICATION	FILE 0242S122				WFNT.
CALE	1:20	PLOT DATE	DRAWING NO. \$1.22	CODE	SHEET	REVISION O	מטטמ



DRAWN **APPROVED** APP'D DATE

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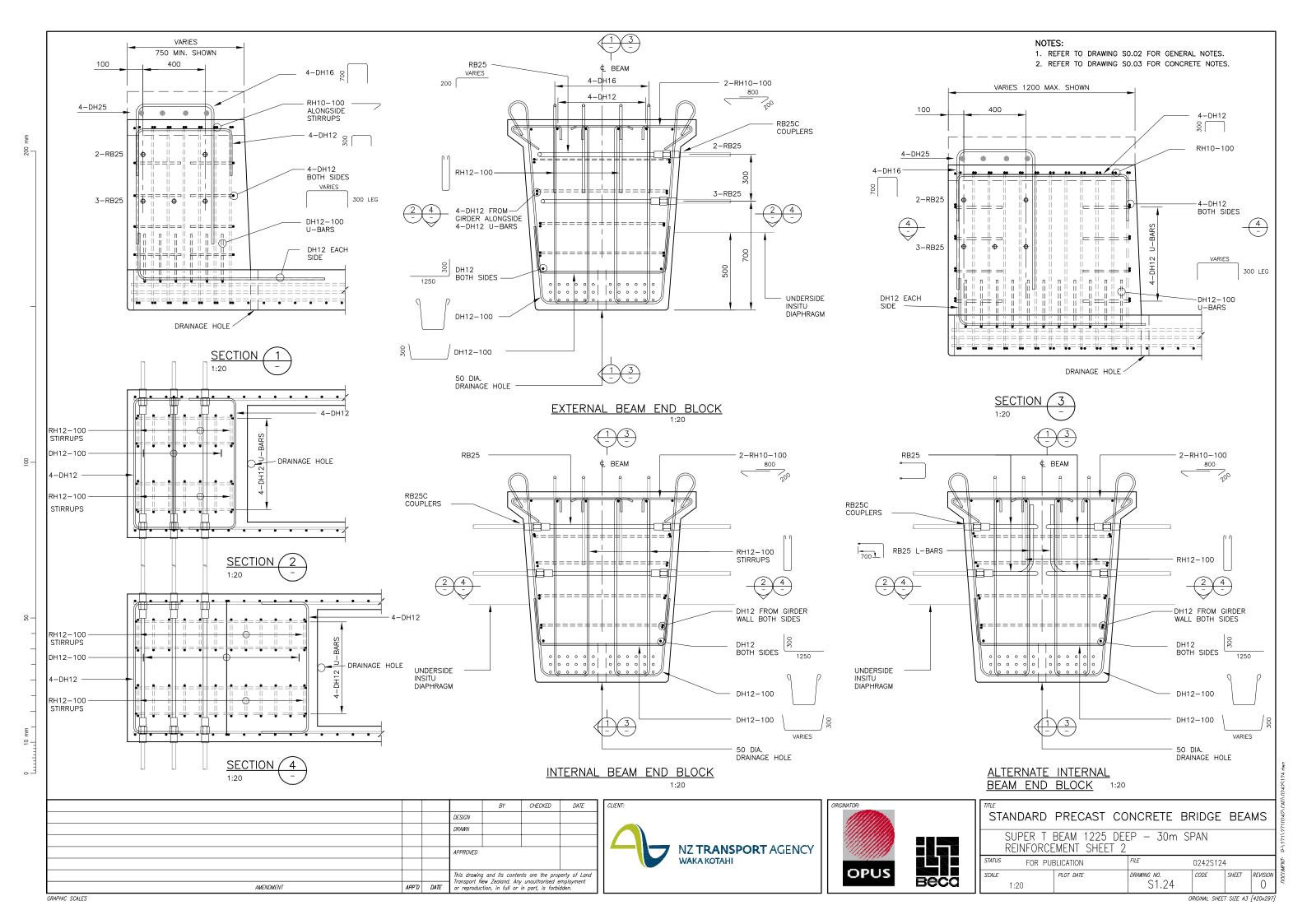






SUPER T BEAM	1225 DEEP	- 30m SPAN
REINFORCEMENT	SHEET 1	

STATUS	FOR PUE	BLICATION	FILE	0242S123	3		WFNT.
<i>SCALE</i> 1:50	1:20	PLOT DATE	DRAWING NO. \$1.23	CODE	SHEET	REVISION O	III



- a. ALL REINFORCEMENT SHALL BE GRADE 500E TO AS/NZS4671
- b. ALL PRESTRESSING STRAND SHALL BE 15.2mm DIAMETER LOW RELAXATION STRESS RELIEVED SUPER GRADE 7 WIRE STRAND COMPLYING WITH AS/NZS 4672 OR BS 5896
- c. MINIMUM BREAKING LOAD OF STRAND 250 kN
- d. FORCE IN STRANDS IMMEDIATELY PRIOR TO TRANSFER SHALL BE 185 kN. RELAXATION PRIOR TO TRANSFER SHALL BE ACCOUNTED FOR IN THE JACKING FORCE REQUIRED TO ACHIEVE THIS VALUE. TYPICALLY RELAXATION PRIOR TO TRANSFER IS IN THE ORDER OF 1%. WHERE CURING AT ELEVATED TEMPERATURES IS EMPLOYED, HIGHER RELAXATION RATES MAY RESULT AND DUE ALLOWANCE FOR THIS SHALL BE MADE BY THE PRECASTER IN DETERMINING THE JACKING FORCE REQUIRED TO ACHIEVE THE MINIMUM FORCE STATED ABOVE.
- e. ENDS OF STRAND SHALL BE CUT FLUSH AND PROTECTED WITH A MINIMUM OF 5mm EPOXY MORTAR.
- f. UPWARD DEFLECTION OF GIRDERS DUE TO PRESTRESS IS GIVEN IN THE BEAM HOG TABLE. THESE ARE ESTIMATES ONLY. ESTIMATES ARE MADE FOR HOG AT TRANSFER AND AT 100 DAYS WITH DUE ALLOWANCE FOR INCREASE IN HOG DUE TO CREEP OF CONCRETE UNDER SUSTAINED LOAD.
- g. COMPONENTS PREFIXED RB ARE REIDBAR ITEMS. REIDBAR SHALL BE GRADE 500E TO

3. CONCRETE COVER (MINIMUM)

COVER TO ALL PRESTRESSING COMPONENTS - 40mm COVER TO ALL REINFORCEMENT EXPOSED SURFACE - 40mm COVER TO ALL REINFORCEMENT INTERNAL SURFACE - 30mm COVER ADJACENT TO CORED HOLES - 30mm COVER TO BRIDGE DECK & ALL CAST INSITU CONCRETE - 50mm COVER TO BARRIER FIXING STEEL (WITHIN BARRIER)

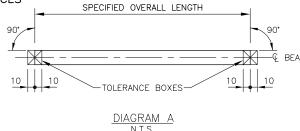
4. DESIGN LOADING

HN-H0-72 (INCLUDING SLS LIVE LOAD FACTOR OF 1.35)

5. SPECIFICATION

THIS DESIGN IS BASED ON LTNZ STANDARD BRIDGE BEAM SPECIFICATION (2006)

6. TOLERANCES



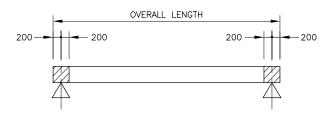
6.1. DIMENSIONS AT TIME OF ERECTION

ACTUAL OVERALL LENGTH AND SQUARENESS

- a. THE UNDERSIDE OF THE BEAM FOR THE END 700mm SHALL BE TRUE PLANE.
- b. THE BEAM END SURFACES SHALL LIE WITHIN THE "TOLERANCE BOXES" SHOWN IN DIAGRAM A
- c. PLANE SURFACE, DEVIATION FROM A 1.5m STRAIGHT EDGE
- d. BEAM HOGGING (SEE SPECIFICATION)
- CROSS SECTION DIMENSIONS UP TO 0.5m
- CROSS SECTION DIMENSIONS 0.5m TO 2.0m g. HORIZONTAL BOW OF LONGITUDINAL AXIS
- 6.1. DIMENSIONS AT TIME OF ERECTION
 - a. LONGITUDINAL STEEL ARRANGEMENT b. LOCATION OF AN ITEM IN RELATION TO ANY OTHER ITEM WITHIN ITS GROUP OR TO

7. HANDLING

EXTREMES OF VERTICAL LIFTING POINTS OR GROUND SUPPORT SHOWN HATCHED. CENTRAL SUPPORT POSITION AS SHOWN IS PREFERRED (BEAM IN UPRIGHT POSITION AT ALL TIMES)



BEAM SUPPORT & LIFTING POINTS NTS

8. METHOD OF MANUFACTURE

BEAMS SHALL BE MANUFACTURED UNDER FACTORY CONDITIONS

9. SURFACE FINISHES

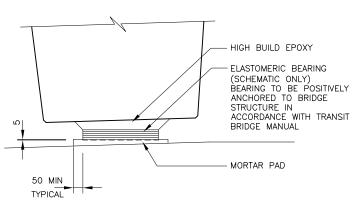
BEAMS

TOP SURFACE OF FLANGE TYPE B CONSTRUCTION JOINT IN DIRECT CONTACT WITH INSITU DIAPHRAGM TYPE B CONSTRUCTION JOINT HIDDEN FORMED SURFACE F1 ALL OTHER FORMED SURFACE

DIAPHRAGM SURFACE FINISH TO BE BASIC FINISH IN ACCORDACE WITH LTNZ STANDARD BEAM SPECIFICATION (2006)

10. BEARING DESIGN DATA

SPAN		REACTION (kN)		ROTATION (x	10 ⁻³ RADIANS)
(m)	DEAD LOAD (UNFACTORED)	LIVE LOAD (1.35 HN x I)	OVERLOAD (HO x I)	LIVE LOAD (1.35 HN x I)	OVERLOAD (HO × I)
30	580	440	515	2.7	2.8



OPTION FOR BEARING ARRANGEMENT 1:20

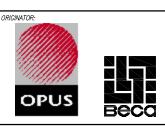
GAP TO SUIT SPAN ARRANGEMENT OPTION FOR LINKAGE BAR DETAIL 1:20

STATU

ABUTMENT BEAM DIAPHRAGM

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50 ID SLEEVE CAST INTO ABUTMENT

WALL & DIAPHRAGM

AS PER OTHER SIDE EXCEPT NO GAP

BETWEEN WALL &
RUBBER PAD —

RB32 GALV. -

E				
STANDARD	PRECAST	CONCRETE	BRIDGE	REAMS
317111071110	111207101	OOMOMENE	BINDOL	<i>DL</i> ,
CLIDED T	DEAM 1225	DEED 30m	CDAN	

TO SUIT SEISMIC REQUIREMENTS

RB NUT & RB HALF NUT GALV.

150x150x50 RUBBER PAD GLUED

-150x150x32 GALV. PLATE

1. REFER TO DRAWING SO.02 FOR GENERAL NOTES.

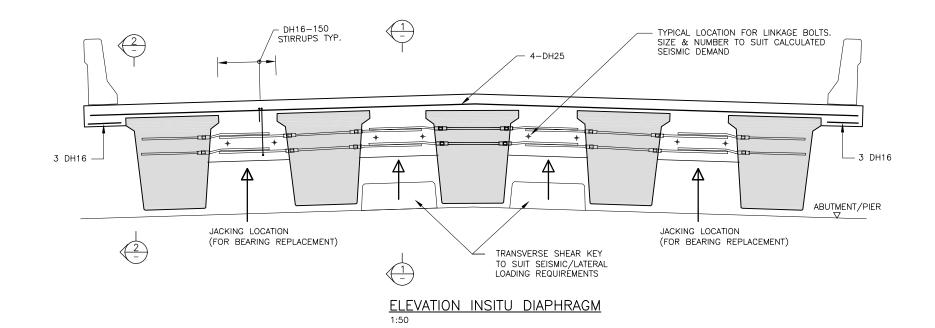
2. REFER TO DRAWING SO.03 FOR CONCRETE NOTES.

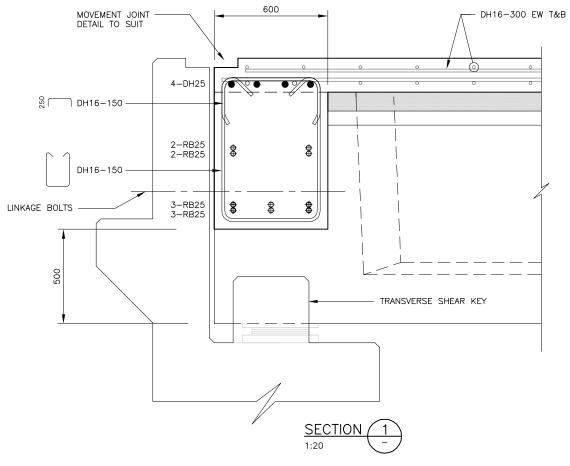
SUPER T BEA	M 1225	DEEP	_	30m	SPAN	
UNIT DATA						

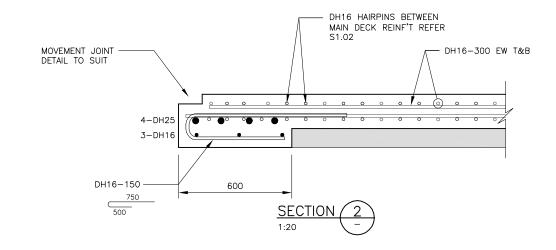
ror pue	BLICATION	FILE	0242S125				
AS SHOWN	PLOT DATE	DRAWING NO. \$1.25	CODE	SHEET	REVISION O		



- 1. REFER TO DRAWING SO.02 FOR GENERAL NOTES.
- 2. REFER TO DRAWING SO.03 FOR CONCRETE NOTES.







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			APPROVED			
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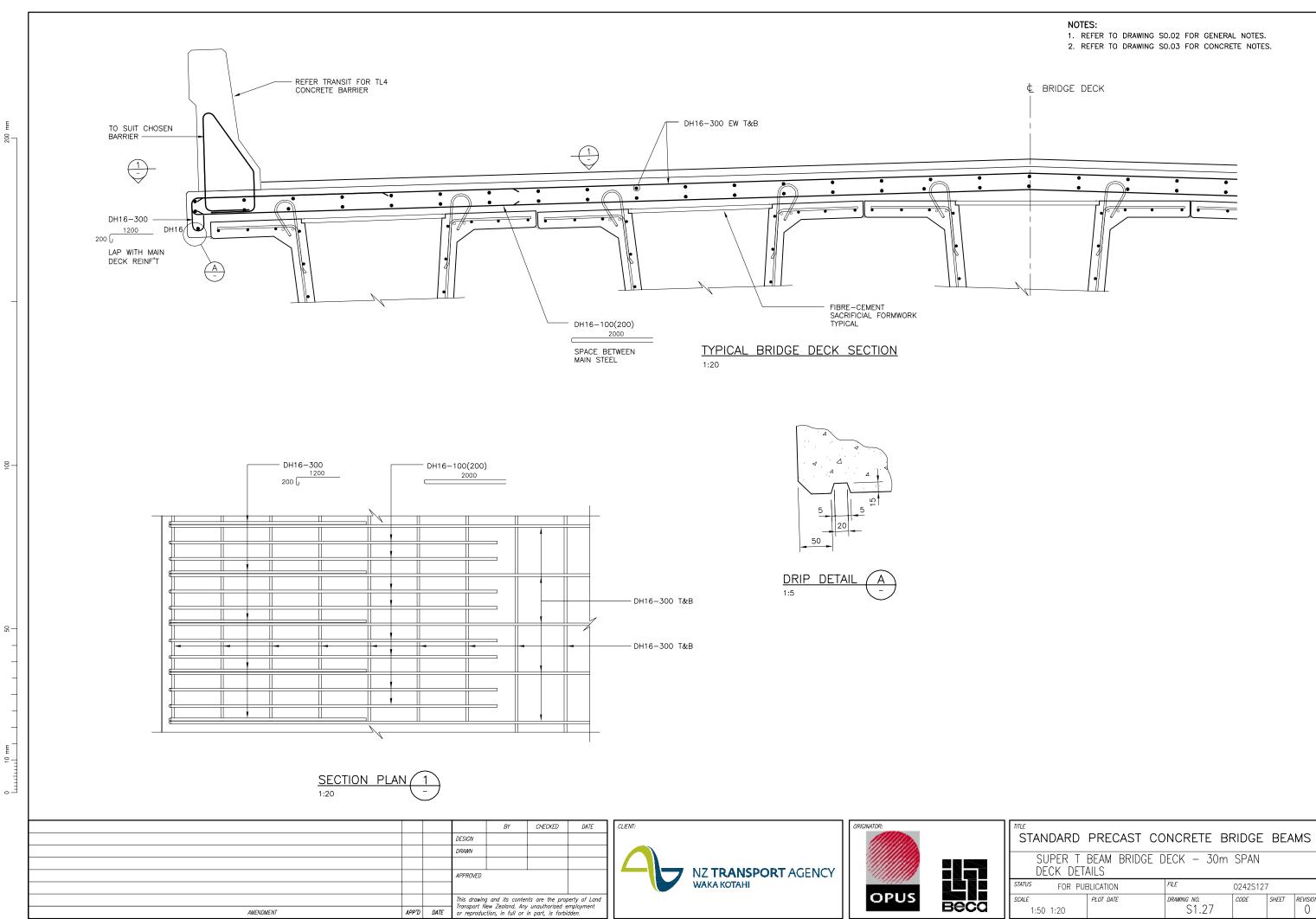


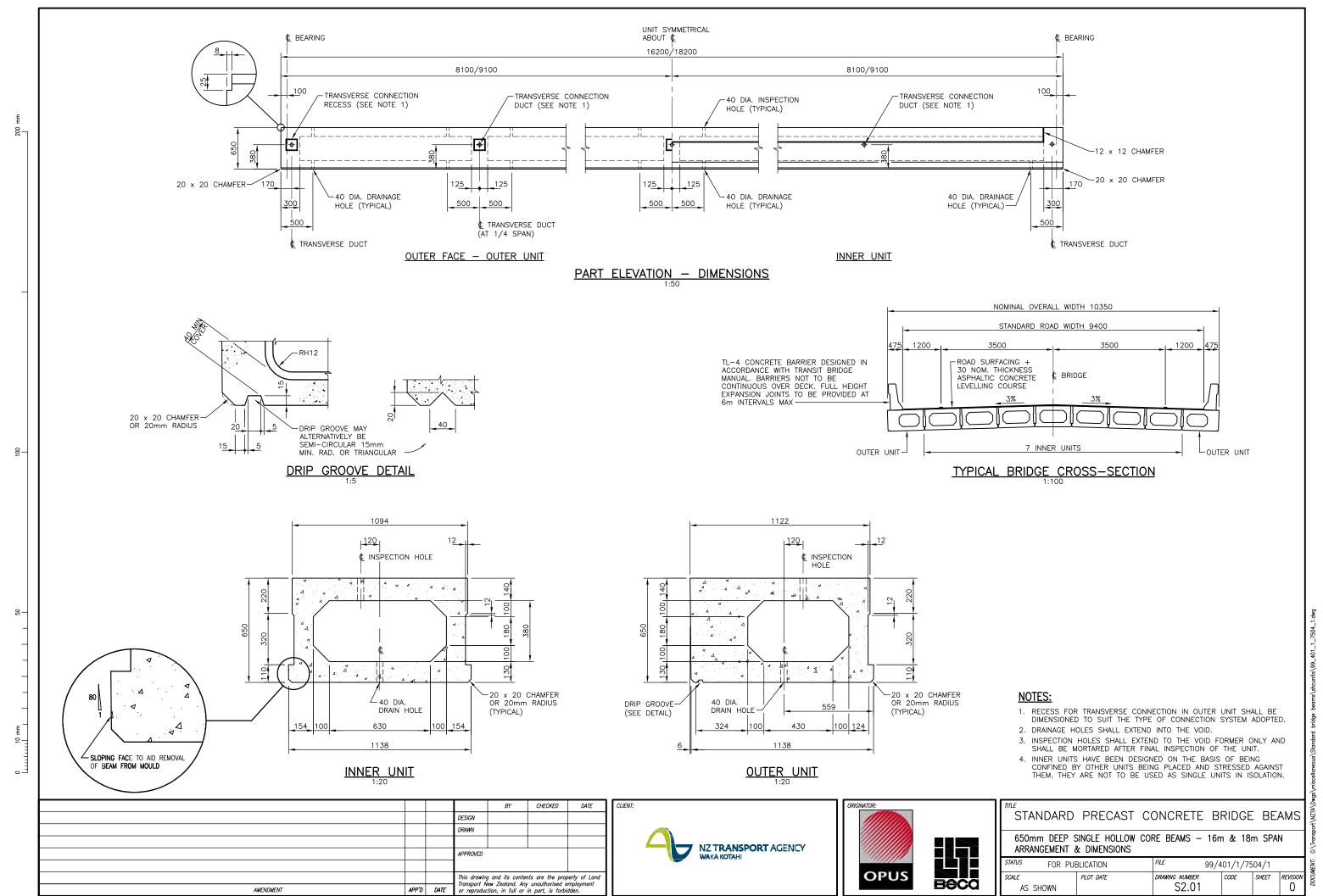
Beca

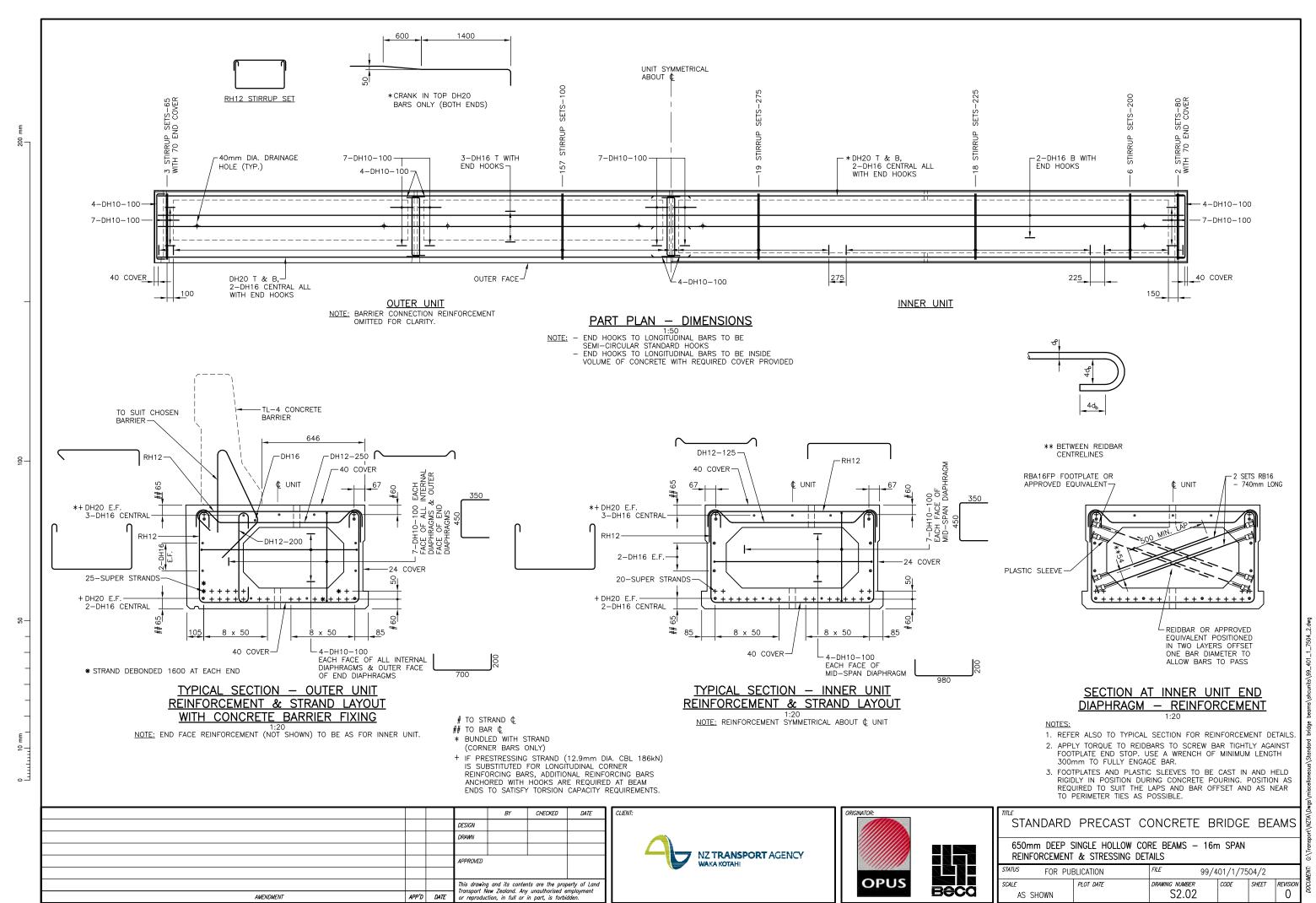
TLE				
STANDARD	PRECAST	CONCRETE	BRIDGE	BEAMS
017111071110	11120/101	OOMONETE	BINDOL	
SLIPER T	RFAM 1225	DEED - 30	m SPAN	

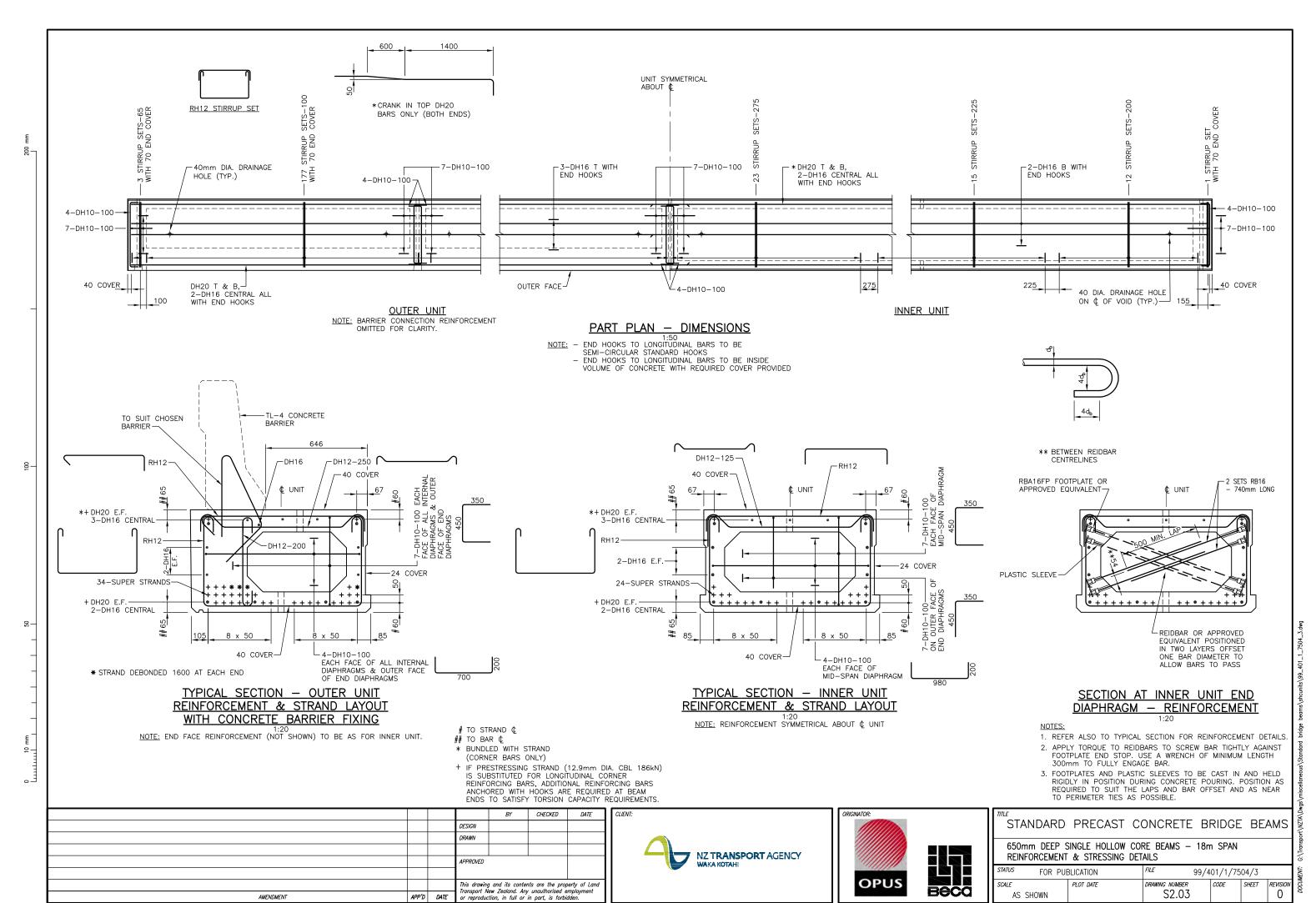
END DIAPHRAGM DETAILS	SUPER T BEAM	1225 DEEP	- 30m SPAN
	END DIAPHRAGM	DETAILS	

FOR PUBLICATION			FILE 0242S126				WFNT.
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ALL SUPERSTRANDS SHALL BE 12.7mm SUPER 7 WIRE STRANDS, COMPLYING WITH AS/NZS 4672, AND ASSUMED TO HAVE A MINIMUM BREAKING LOAD OF 184KN PER STRAND WITH INTIAL LOADING AS FOLLOWS:

TOP TWO STRANDS TO BE INITIALLY LOADED TO 127KN PER STRAND

 OTHER STRANDS TO BE INITIALLY LOADED TO 130KN PER STRAND STRANDS SHALL BE RELEASED SLOWLY AND AFTER RELEASE SHALL BE CUT AND GROUND FLUSH

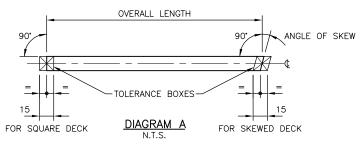
WITH THE CONCRETE AT THE END OF THE UNIT. A THICK COATING OF HIGH BUILD EPOXY PAINT SHALL BE APPLIED AFTER GRINDING BEFORE THE UNIT LEAVES THE CASTING YARD.

2. TOLERANCES

TOLERANCES ARE TO BE IN ACCORDANCE WITH NZS 3109:1997 TABLE 5.1 UNLESS STATED OTHERWISE BELOW.

2.1 DIMENSIONS AT TIME OF ERECTION

ACTUAL OVERALL LENGTH AND SQUARENESS.
THE UNIT END SURFACES SHALL LIE WITHIN THE TOLERANCE BOXES SHOWN IN DIAGRAM A.



a. OVERALL LENGTH	±12mm
b. PLANE SURFACE DEVIATION FROM 1.5m STRAIGHT EDGE	±6mm
c. CROSS-SECTIONAL DIMENSION (OVERALL)	±8mm
d. DIFFERENCE IN LEVEL OF TOP SURFACE BETWEEN ADJACENT UNITS IN PLACE	±15mm
e. HORIZONTAL DEVIATION (SEE SPECIFICATION)	±6mm
f. SMALLEST WEB THICKNESS —	+6mm,-4mi
g. SMALLEST FLANGE THICKNESS —	±6mm
h. DIAPHRAGM THICKNESS —	±12mm
j. HOGGING VARIATION (SEE SPECIFICATION) —	±15mm
k. MAXIMUM HOG	<u>→</u> 25mm

2.2 LOCATION OF STEEL AND CAST-IN ITEMS

a. PRESTRESSING STRANDS IN ANY DIRECTION $\pm 3mm$ b. LOCATION OF AN ITEM IN RELATION TO ANY OTHER WITHIN ITS GROUP ±10mm c. TRANSVERSE DUCT POSITION ±12mm d. VOID FORMERS

±12mm

24mm

65mm

40mm UNLESS SHOWN OTHERWISE

3. CONCRETE COVER

COVER TO ALL PRESTRESSING COMPONENTS COVER TO ALL REINFORCING STEEL COVER ADJACENT TO VOIDS COVER ADJACENT TO SHEAR KEYS COVER BARRIER FIXING STEEL (WITHIN BARRIER)

4. CONCRETE STRENGTH

MINIMUM COMPRESSIVE STRENGTH AT TRANSFER 30MPa SPECIFIED COMPRESSIVE STRENGTH AT 28 DAYS 50MPa INFILL CONCRETE BETWEEN UNITS 30MPa MORTAR BACKFILL TO TRANSVERSE STRAND ANCHORAGE POCKETS 50MPa NON-SHRINK GROUT TO TRANSVERSE PRESTRESSING STRAND DUCTS 40MPa

5. DESIGN LOADING

HN-HO-72 (INCLUDING SLS LIVE LOAD FACTOR OF 1.35)

6. SPECIFICATION

THIS DESIGN IS BASED ON MATERIALS AND WORKMANSHIP IN ACCORDANCE WITH THE LTNZ STANDARD BRIDGE BEAM SPECIFICATION (2007).

7. HANDLING

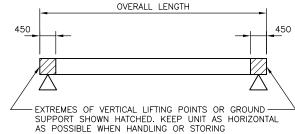
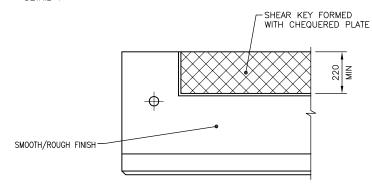


DIAGRAM B

8. SURFACE FINISHES

a. TOP SURFACE - BROOM FINISH.

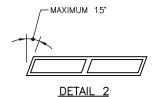
b. SIDE AND UNDERSIDE SURFACE - SMOOTH/ROUGH FINISH EXCEPT SHEAR KEY. SEE



DETAIL 1

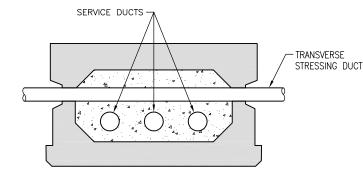
9. SKEW

THE MAXIMUM PERMISSIBLE SKEW OF THE UNITS SHALL BE 15' UNLESS A SPECIFIC LIVE LOAD ANALYSIS IS MADE. THE END DIAPHRAGMS OF THE UNIT SHALL BE SKEWED TO THE REQUIRED ANGLE - SEE DETAIL 2



STIRRUPS SHALL BE PLACED PARALLEL TO THE LINE OF SKEW WITHIN 1m OF EACH END DIAPHRAGM. STIRRUPS ALONG THE SPAN SHALL BE PLACED NORMAL TO LONGITUDINAL STEEL WITH THE SKEW/NORMAL STIRRUP INTERFACE HAVING ADDITIONAL STIRRUPS IN A FAN ARRANGEMENT WITH THE SPECIFIED MAXIMUM STIRRUP SPACING ON THE OUTSIDE OF THE 'FAN'.

- 1. CABLES AND SMALL SERVICES MAY BE ACCOMMODATED IN THE HOLLOW CORE CABLES AND SMALL SERVICES MAY BE ACCOMMODATED IN THE HOLLOW CORE BUT NOWHERE ELSE IN THE UNIT. THE SERVICES DUCTS ARE TO BE NO GREATER THAN 100mm IN DIAMETER AND A CLEARANCE OF 40mm FROM TRANSVERSE STRESSING DUCTS SHALL BE MAINTAINED. THE TOTAL CROSS—SECTIONAL AREA OF CABLES AND SERVICE DUCTS WITHIN A UNIT SHALL NOT EXCEED 8% OF THE CROSS—SECTIONAL AREA OF THE UNIT INTERNAL VOID. NO TWO CABLES OR SERVICE DUCTS SHALL BE POSITIONED CLOSER TOGETHER THAN THE DIAMETER OF THE SMALLER CABLE OR DUCT OR 50mm. AT END AND INTERNAL DIABLEAUER. INTERNAL DIAPHRAGMS A MINIMUM CLEARANCE OF 50mm SHALL BE PROVIDED BETWEEN THE CABLES/SERVICES DUCTS AND THE BASE OF THE VOID
- 2. AN ALLOWANCE FOR TOLERANCES HAS BEEN MADE IN THE NOMINAL OVERALL WIDTH DIMENSION SHOWN IN THE TYPICAL SECTIONS. UNITS ARE SPACED AT 1.150m CENTRES TO ALLOW A WORKING TOLERANCE ON WIDTH & STRAIGHTNESS.
- 3. IN THE JACKING OF AN ASSEMBLED BRIDGE DECK, JACKS BEARING ON UNITS CONTAINING SERVICE DUCTS SHALL BE POSITIONED TO BEAR UNDER THE WEBS OF THE UNITS. ONE JACK PER UNIT TO BE PROVIDED AT EACH END OF THE DECK WHEN JACKING.



DETAIL 3 - SECTION AT END DIAPHRAGM

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			APPROVED			
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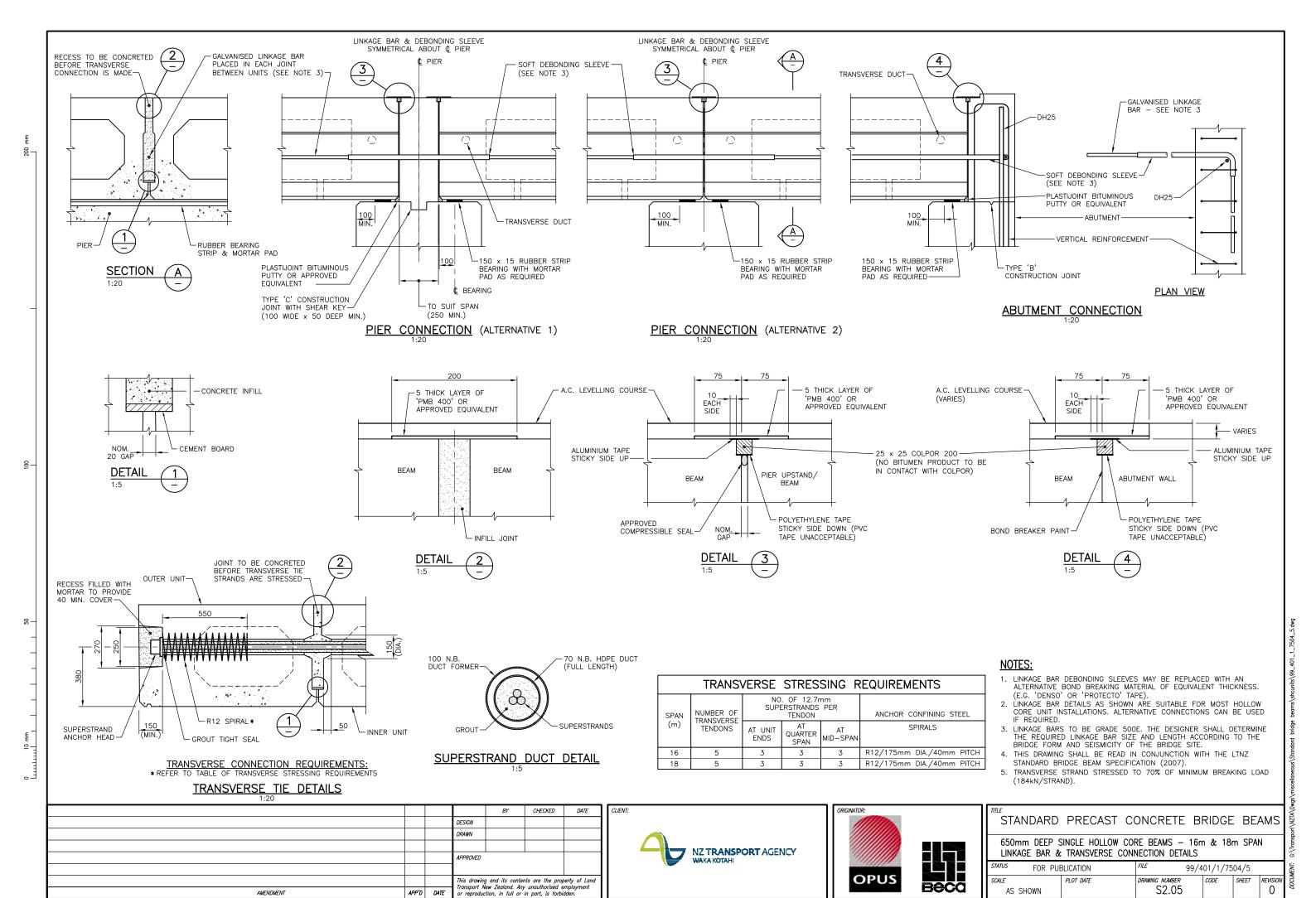


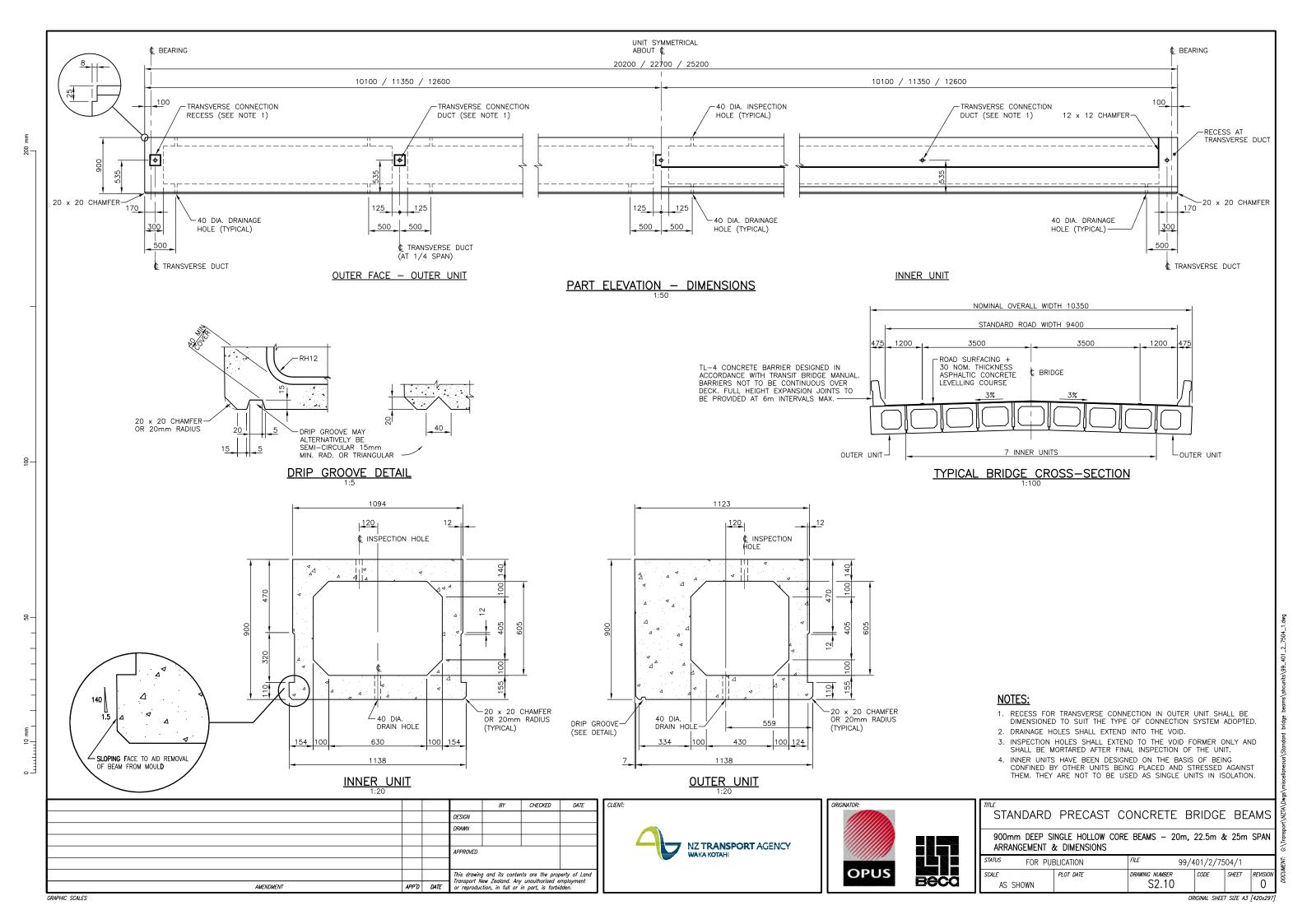


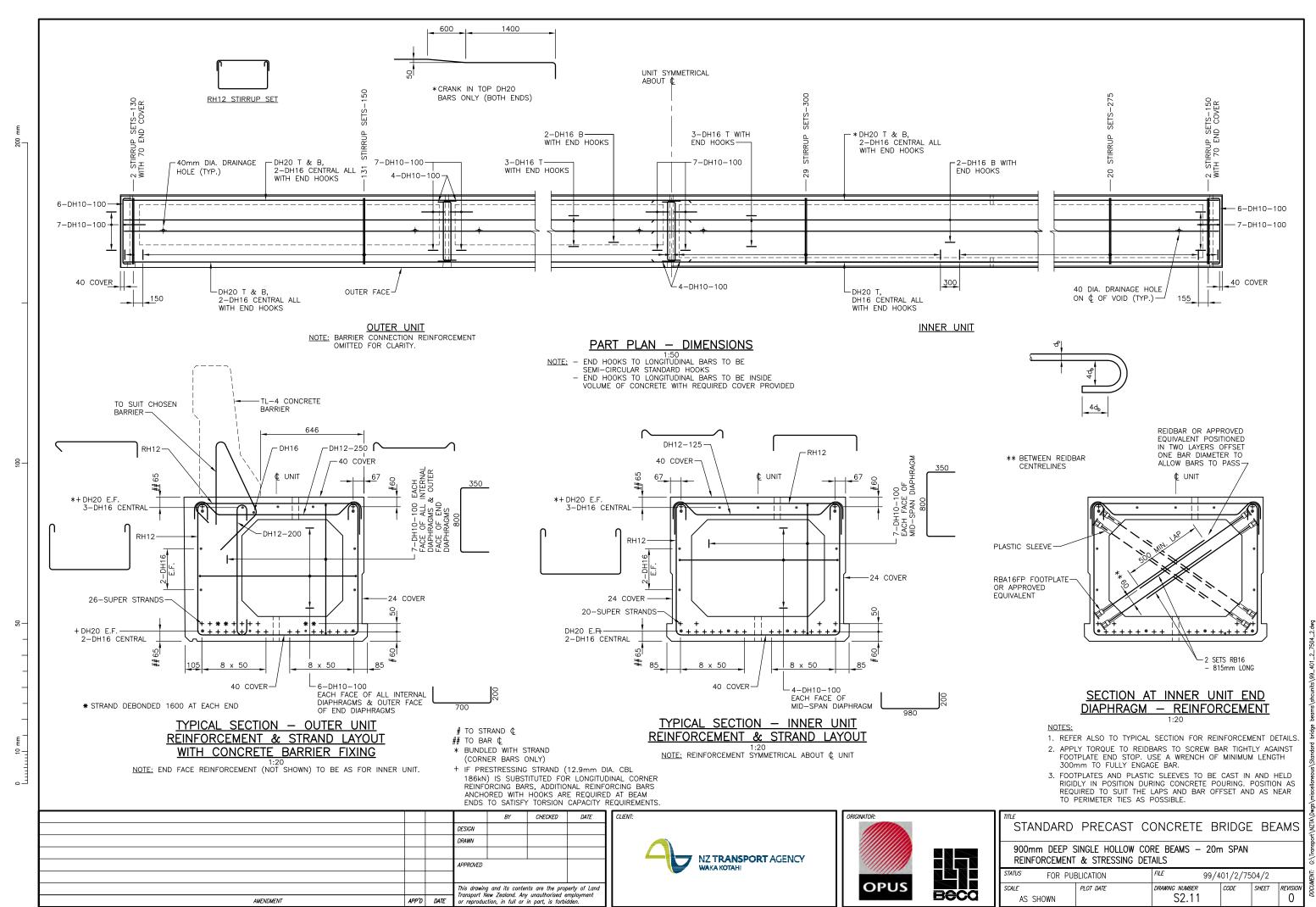
STANDARD PRECAST CONCRETE BRIDGE BEAMS

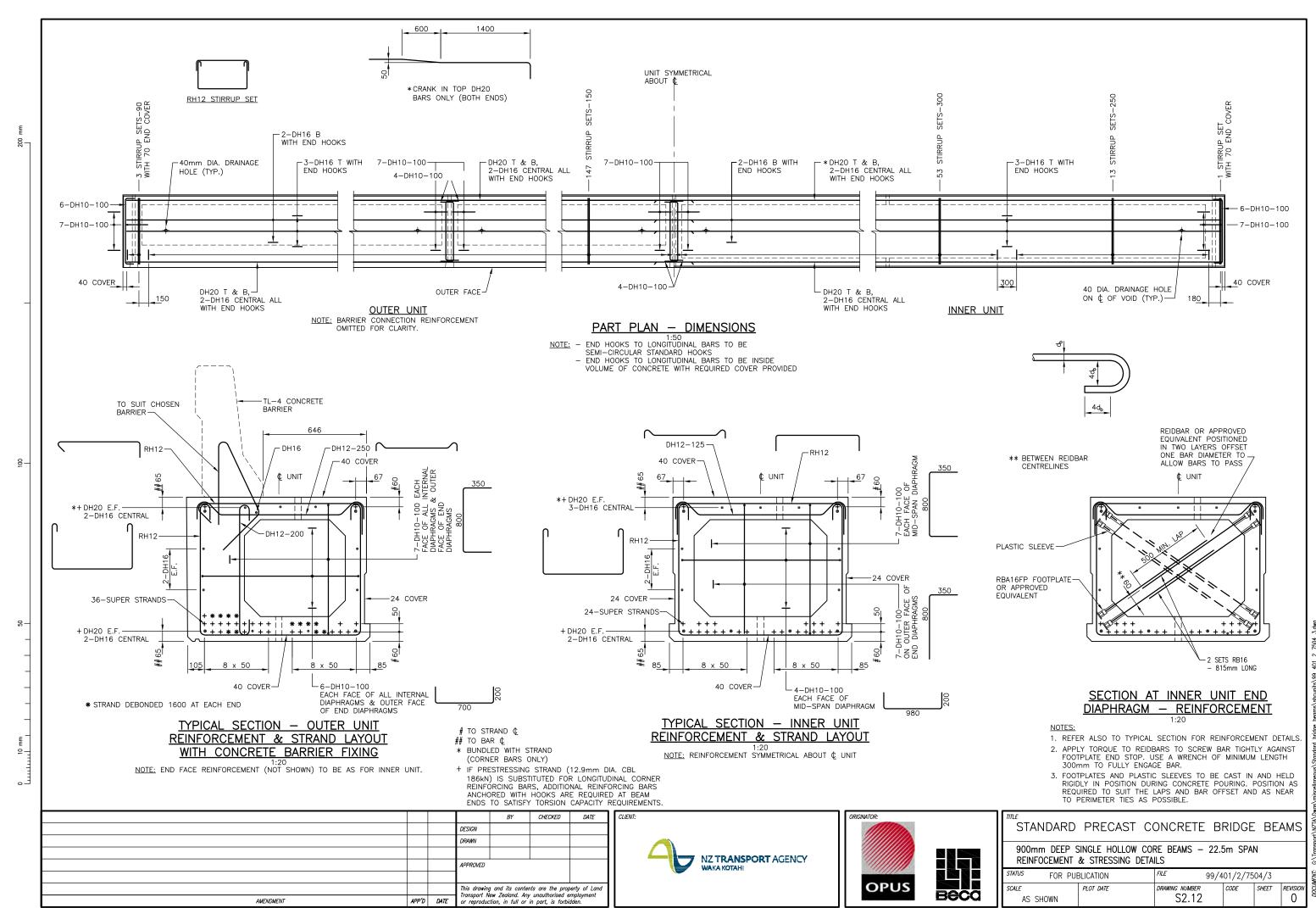
650mm DEEP SINGLE HOLLOW CORE BEAMS - 16m & 18m SPAN UNIT DATA

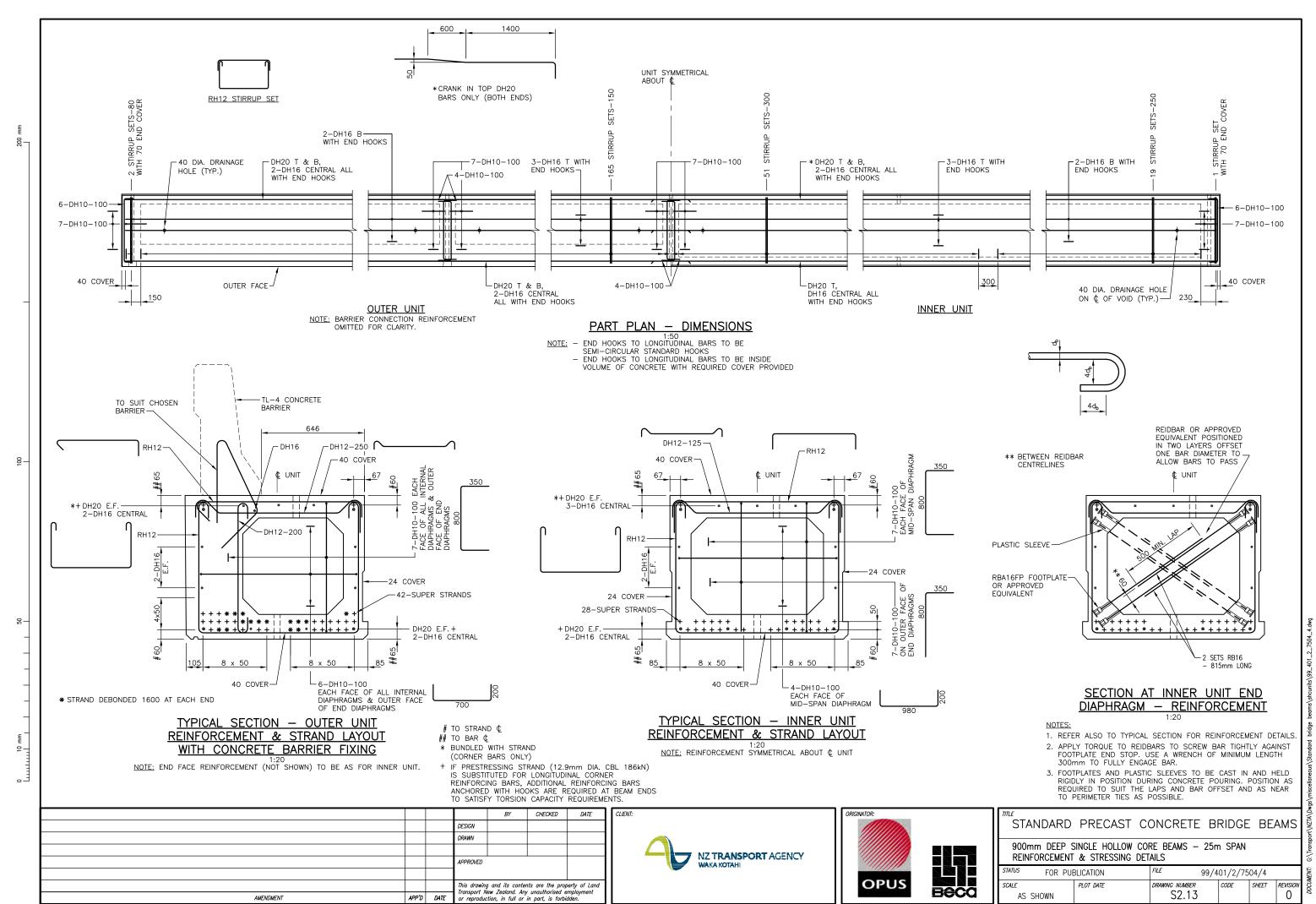
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TOP TWO STRANDS TO BE INITIALLY LOADED TO 127KN PER STRAND
 OTHER STRANDS TO BE INITIALLY LOADED TO 130KN PER STRAND

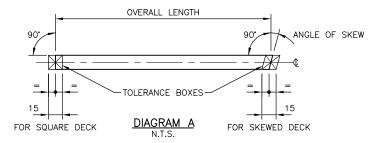
STRANDS SHALL BE RELEASED SLOWLY AND AFTER RELEASE SHALL BE CUT AND GROUND FLUSH WITH THE CONCRETE AT THE END OF THE UNIT. A THICK COATING OF HIGH BUILD EPOXY PAINT SHALL BE APPLIED AFTER GRINDING BEFORE THE UNIT LEAVES THE CASTING YARD.

2. TOLERANCES

TOLERANCES ARE TO BE IN ACCORDANCE WITH NZS 3109:1997 TABLE 5.1 UNLESS STATED OTHERWISE BELOW.

2.1 DIMENSIONS AT TIME OF ERECTION

ACTUAL OVERALL LENGTH AND SQUARENESS.
THE UNIT END SURFACES SHALL LIE WITHIN THE TOLERANCE BOXES SHOWN IN DIAGRAM A.



a. OVERALL LENGTH —	-	±12mm
b. PLANE SURFACE DEVIATION FROM 1.5m STRAIGHT EDGE		±6mm
c. CROSS-SECTIONAL DIMENSION (OVERALL)		±8mm
d. DIFFERENCE IN LEVEL OF TOP SURFACE BETWEEN ADJACENT UNITS IN PLACE	-	±15mm
e. HORIZONTAL DEVIATION (SEE SPECIFICATION)	-	±6mm
f. SMALLEST WEB THICKNESS —	-	+6mm,-4mr
g. SMALLEST FLANGE THICKNESS —	-	±6mm
h. DIAPHRAGM THICKNESS —		±12mm
n. DIAPHRAGM THICKNESS ———————————————————————————————————		±12mm ±15mm
	-	

±3mm

±10mm

±12mm

±12mm

40mm

30mm

24mm

65mm

30MPa

50MPa

30MPa

50MPa

40MPa

2.2 LOCATION OF STEEL AND CAST-IN ITEMS

a. PRESTRESSING STRANDS IN ANY DIRECTION

b. LOCATION OF AN ITEM IN RELATION TO ANY OTHER WITHIN ITS GROUP

c. TRANSVERSE DUCT POSITION

d. VOID FORMERS

3. CONCRETE COVER

COVER TO ALL PRESTRESSING COMPONENTS COVER TO ALL REINFORCING STEEL COVER ADJACENT TO VOIDS COVER ADJACENT TO SHEAR KEYS COVER TO BARRIER FIXING STEEL (WITHIN BARRIER)

4. CONCRETE STRENGTH

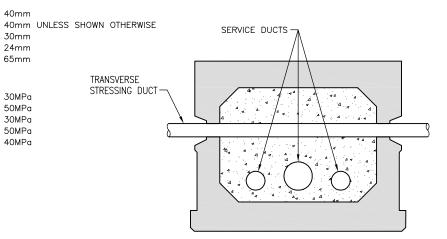
MINIMUM COMPRESSIVE STRENGTH AT TRANSFER SPECIFIED COMPRESSIVE STRENGTH AT 28 DAYS INFILL CONCRETE BETWEEN UNITS MORTAR BACKFILL TO TRANSVERSE STAND ANCHORAGE POCKETS NON-SHRINK GROUT TO TRANSVERSE PRESTRESSING STRAND DUCTS

DESIGN LOADING

HN-HO-72 (INCLUDING SLS LIVE LOAD FACTOR OF 1.35)

6. SPECIFICATION

THIS DESIGN IS BASED ON MATERIALS AND WORKMANSHIP IN ACCORDANCE WITH THE LTNZ STANDARD BRIDGE BEAM SPECIFICATION (2007).



DETAIL 3 - SECTION AT END DIAPHRAGM

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			DRAWN			
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7. HANDLING

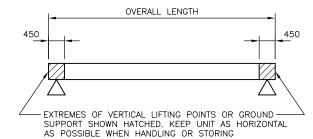
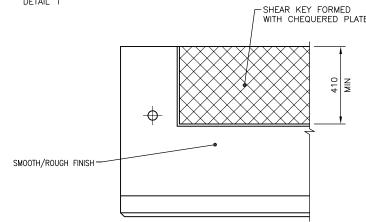


DIAGRAM B

8. SURFACE FINISHES

a. TOP SURFACE - BROOM FINISH.

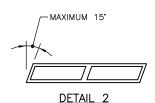
b. SIDE AND UNDERSIDE SURFACE - SMOOTH/ROUGH FINISH EXCEPT SHEAR KEY. SEE



DETAIL 1

9. SKEW

THE MAXIMUM PERMISSIBLE SKEW OF THE UNITS SHALL BE 15' UNLESS A SPECIFIC LIVE LOAD ANALYSIS IS MADE. THE END DIAPHRAGMS OF THE UNIT SHALL BE SKEWED TO THE REQUIRED ANGLE - SEE DETAIL 2.



STIRRUPS SHALL BE PLACED PARALLEL TO THE LINE OF SKEW WITHIN 1m OF EACH END DIAPHRAGM. STIRRUPS ALONG THE SPAN SHALL BE PLACED NORMAL TO LONGITUDINAL STEEL WITH THE SKEW/NORMAL STIRRUP INTERFACE HAVING ADDITIONAL STIRRUPS IN A FAN ARRANGEMENT WITH THE SPECIFIED MAXIMUM STIRRUP SPACING ON THE OUTSIDE OF THE 'FAN'.

- 1. CABLES AND SMALL SERVICES MAY BE ACCOMMODATED IN THE HOLLOW CORE CABLES AND SMALL SERVICES MAY BE ACCOMMODATED IN THE HOLLOW CORE BUT NOWHERE ELSE IN THE UNIT. THE SERVICES DUCTS ARE TO BE NO GREATER THAN 150mm IN DIAMETER AND A CLEARANCE OF 40mm FROM TRANSVERSE STRESSING DUCTS SHALL BE MAINTAINED. THE TOTAL CROSS—SECTIONAL AREA OF CABLES AND SERVICE DUCTS WITHIN A UNIT SHALL NOT EXCEED 8% OF THE CROSS—SECTIONAL AREA OF THE UNIT INTERNAL VOID. NO TWO CABLES OR SERVICE DUCTS SHALL BE POSITIONED CLOSER TOGETHER THAN THE DIAMETER OF THE SMALLER CABLE OR DUCT OR 50mm. AT END AND INTERNAL DIABLEAUER. INTERNAL DIAPHRAGMS A MINIMUM CLEARANCE OF 50mm SHALL BE PROVIDED BETWEEN THE CABLES/SERVICES DUCTS AND THE BASE OF THE VOID
- 2. AN ALLOWANCE FOR TOLERANCES HAS BEEN MADE IN THE NOMINAL OVERALL WIDTH DIMENSION SHOWN IN THE TYPICAL SECTIONS. UNITS ARE SPACED AT 1.150m CENTRES TO ALLOW A WORKING TOLERANCE ON WIDTH & STRAIGHTNESS.
- 3. IN THE JACKING OF AN ASSEMBLED BRIDGE DECK, JACKS BEARING ON UNITS CONTAINING SERVICE DUCTS SHALL BE POSITIONED TO BEAR UNDER THE WEBS OF THE UNITS. ONE JACK PER UNIT TO BE PROVIDED AT EACH END OF THE DECK WHEN JACKING.

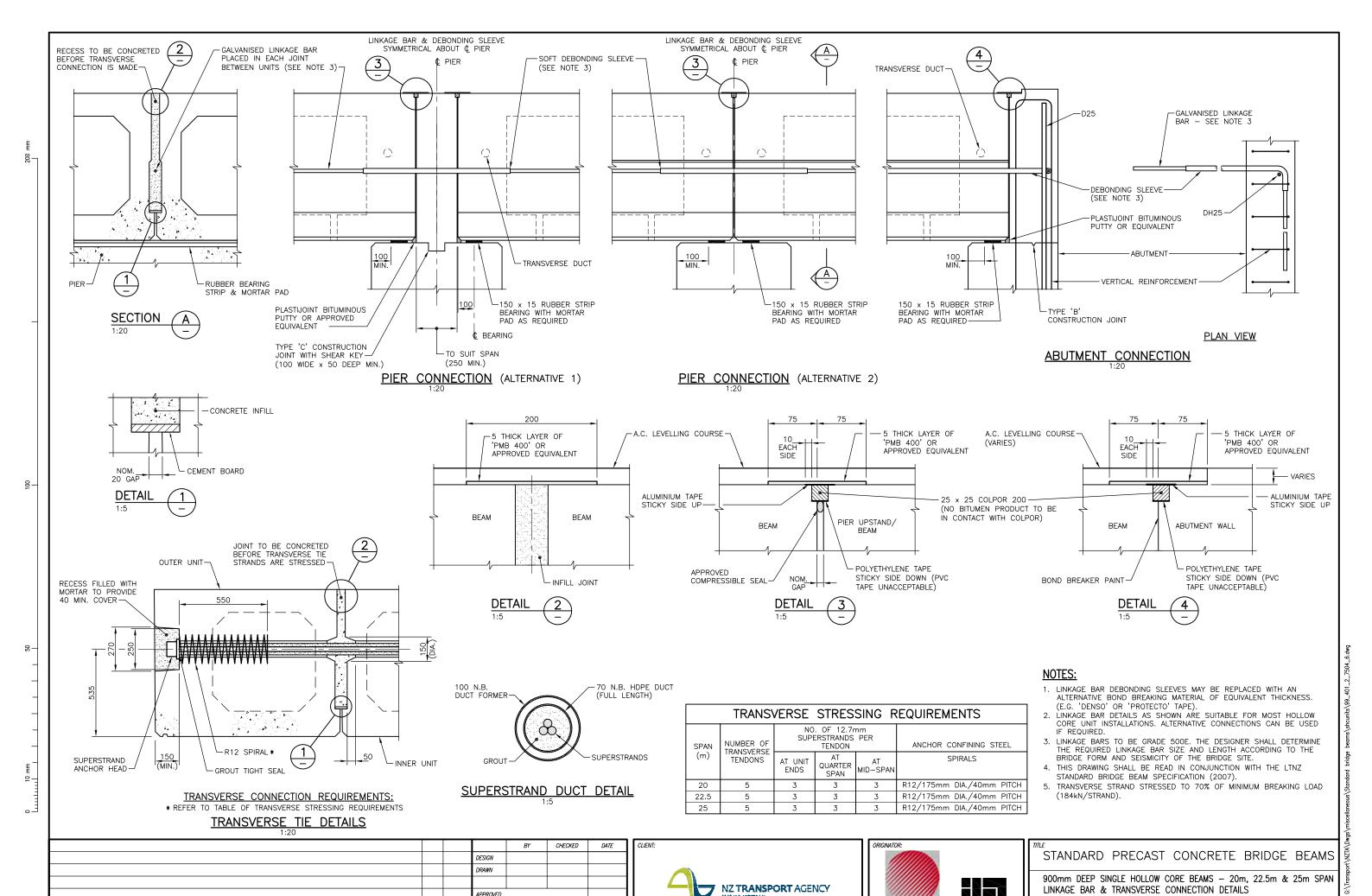




STANDARD PRECAST CONCRETE BRIDGE BEAMS

900mm DEEP SINGLE HOLLOW CORE BEAMS - 20m, 22.5m & 25m SPAN UNIT DATA

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WAKA KOTAHI

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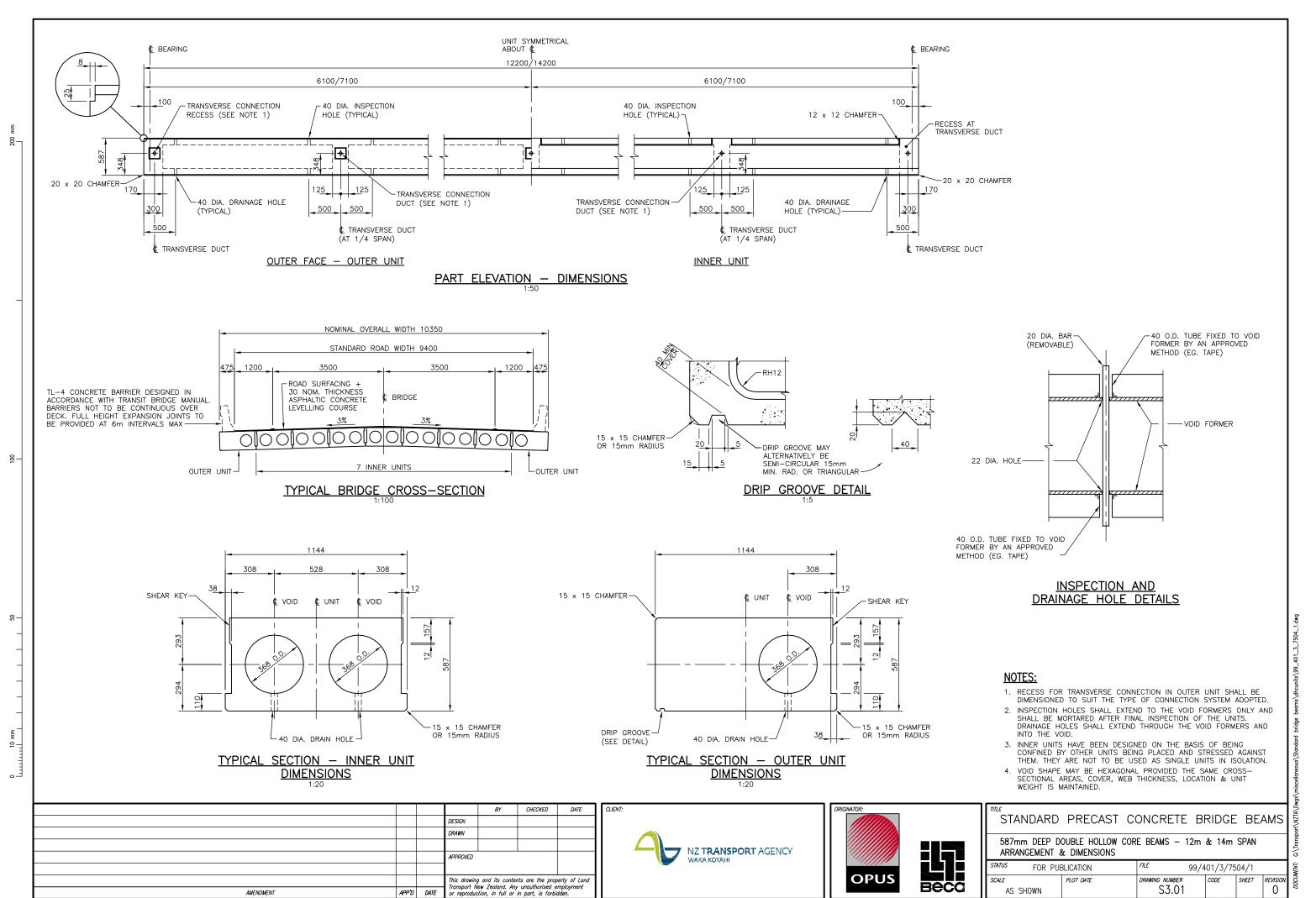
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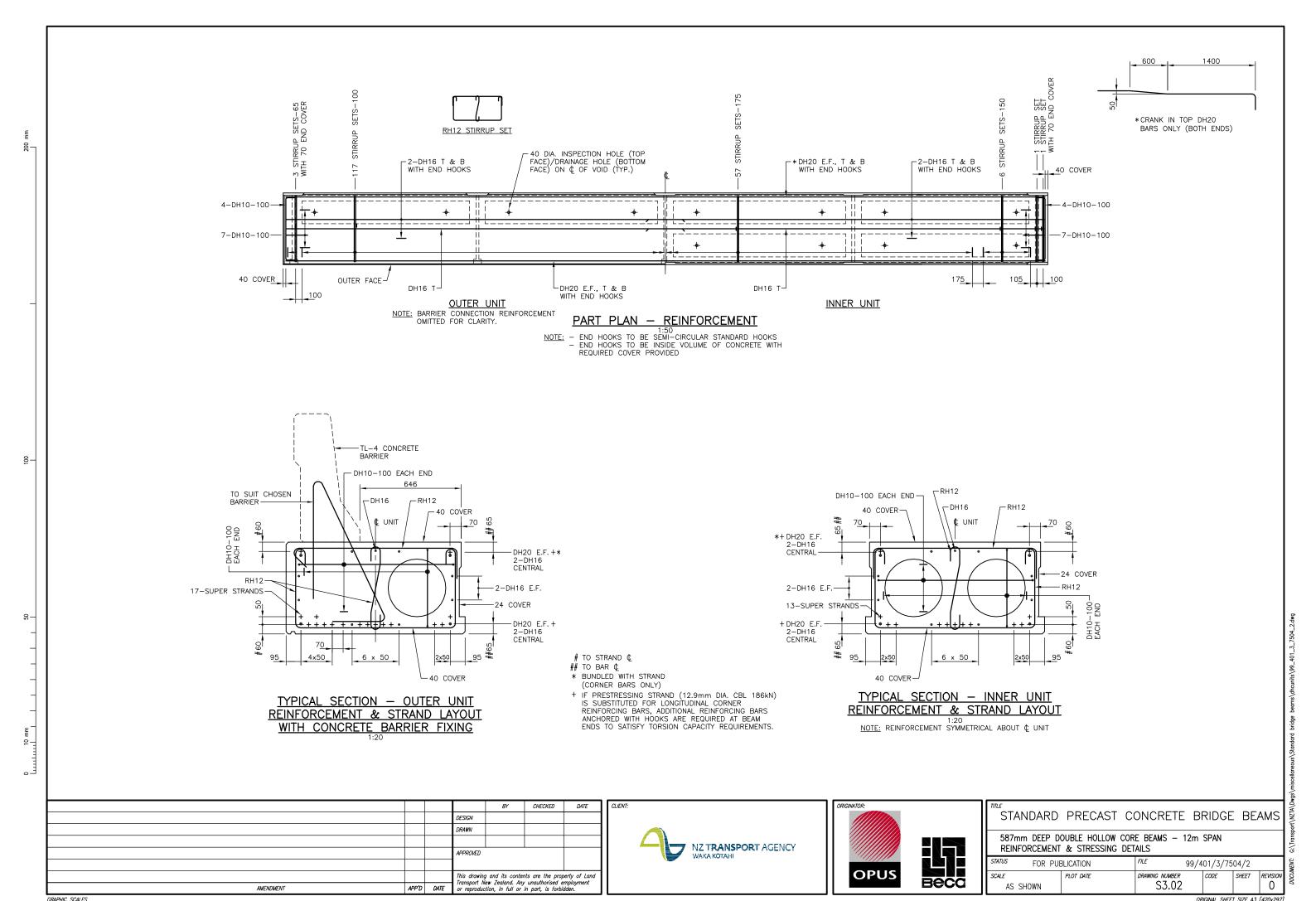
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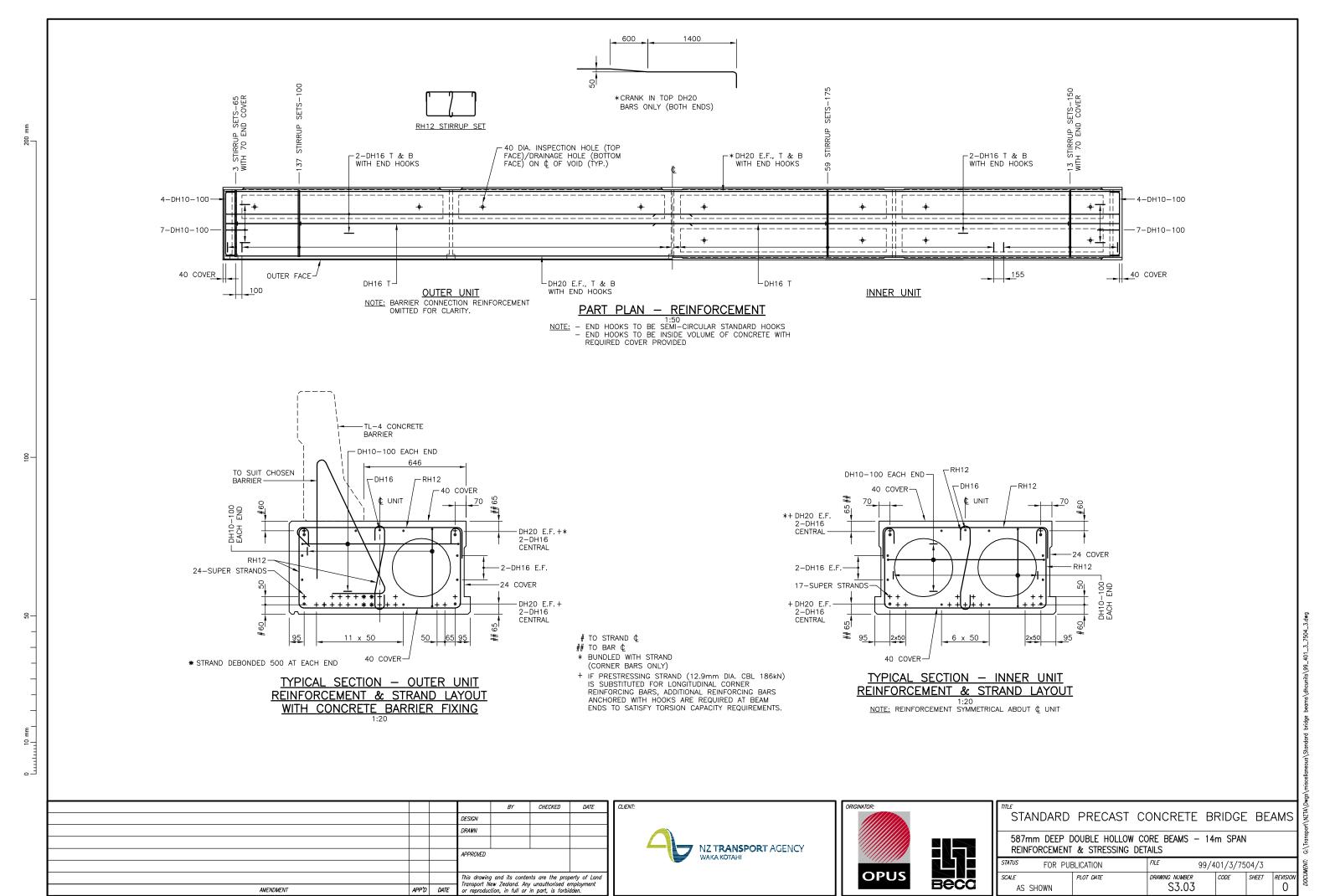
SCALE

AS SHOWN

AMENDMENT







ORIGINAL SHEET SIZE A3 [420x297]

GRAPHIC SCALES

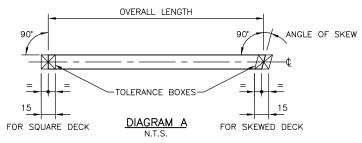
STRANDS SHALL BE RELEASED SLOWLY AND AFTER RELEASE SHALL BE CUT AND GROUND FLUSH WITH THE CONCRETE AT THE END OF THE UNIT. A THICK COATING OF HIGH BUILD EPOXY PAINT SHALL BE APPLIED AFTER GRINDING BEFORE THE UNIT LEAVES THE CASTING YARD.

2. TOLERANCES

TOLERANCES ARE TO BE IN ACCORDANCE WITH NZS 3109:1997 TABLE 5.1 UNLESS STATED OTHERWISE BELOW.

2.1 DIMENSIONS AT TIME OF ERECTION

ACTUAL OVERALL LENGTH AND SQUARENESS.
THE UNIT END SURFACES SHALL LIE WITHIN THE TOLERANCE BOXES SHOWN IN DIAGRAM A.



a. OVERALL LENGTH -	±12mm
b. PLANE SURFACE DEVIATION FROM 1.5m STRAIGHT EDGE	±6mm
c. CROSS-SECTIONAL DIMENSION (OVERALL)	±8mm
d. DIFFERENCE IN LEVEL OF TOP SURFACE BETWEEN ADJACENT UNITS IN PLACE	——— ±15mm
e. HORIZONTAL DEVIATION (SEE SPECIFICATION)	———— ±6mm
f. SMALLEST WEB THICKNESS —	→ +6mm,-4mm
g. SMALLEST FLANGE THICKNESS —	———— ±6mm
h. DIAPHRAGM THICKNESS —	——— ±12mm
j. HOGGING VARIATION (SEE SPECIFICATION)	— ±15mm
k. MAXIMUM HOG	25mm

2.2 LOCATION OF STEEL AND CAST-IN ITEMS

a. PRESTRESSING STRANDS IN ANY DIRECTION ±3mm b. LOCATION OF AN ITEM IN RELATION TO ANY OTHER WITHIN ITS GROUP c. TRANSVERSE DUCT POSITION ±12mm

3. CONCRETE COVER

d. VOID FORMERS

COVER TO ALL PRESTRESSING COMPONENTS COVER TO ALL REINFORCING STEEL COVER ADJACENT TO VOIDS COVER BETWEEN VOIDS AND SHEAR KEYS COVER TO BARRIER FIXING STEEL (WITHIN BARRIER)

4. CONCRETE STRENGTH

MINIMUM COMPRESSIVE STRENGTH AT TRANSFER 30MPa SPECIFIED COMPRESSIVE STRENGTH AT 28 DAYS 50MPa INFILL CONCRETE BETWEEN UNITS 30MPa MORTAR BACKFILL TO TRANSVERSE STRAND ANCHORAGE POCKETS 50MPa NON-SHRINK GROUT TO TRANSVERSE PRESTRESSING STRAND DUCTS 40MPa

5. DESIGN LOADING

HN-HO-72 (INCLUDING SLS LIVE LOAD FACTOR OF 1.35)

6. SPECIFICATION

THIS DESIGN IS BASED ON MATERIALS AND WORKMANSHIP IN ACCORDANCE WITH THE LTNZ STANDARD BRIDGE BEAM SPECIFICATION (2007).

NOTES:

40mm UNIESS SHOWN OTHERWISE

±12mm

40mm

30mm

24mm

65mm

- 1. CABLES AND SMALL SERVICES MAY BE ACCOMMODATED IN THE HOLLOW CORES BUT NOWHERE ELSE IN THE UNIT. THE SERVICE DUCTS ARE TO BE LIMITED TO 100mm O.D. WITH NO MORE THAN ONE DUCT IN EACH HOLLOW CORE. CLEARANCE TO THE TRANSVERSE DUCTS SHALL BE NOT LESS THAN 40mm.
- 2. AN ALLOWANCE FOR TOLERANCES HAS BEEN MADE IN THE NOMINAL OVERALL WIDTH DIMENSION SHOWN IN THE TYPICAL SECTIONS. UNITS ARE SPACED AT 1.150m CENTRES TO ALLOW A WORKING TOLERANCE ON WIDTH & STRAIGHTNESS

7. HANDLING

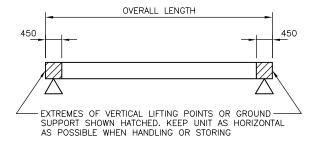
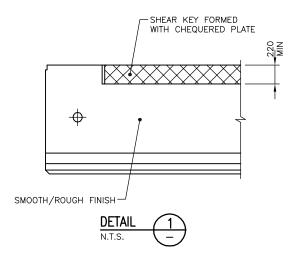


DIAGRAM B

8. SURFACE FINISHES

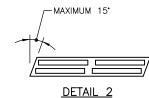
a. TOP SURFACE - BROOM FINISH.

b. SIDE AND UNDERSIDE SURFACE - SMOOTH/ROUGH FINISH EXCEPT SHEAR KEY. SEE



9. SKEW

THE MAXIMUM PERMISSIBLE SKEW OF THE UNITS SHALL BE 15' UNLESS A SPECIFIC LIVE LOAD ANALYSIS IS MADE. THE CORES SHALL BE STAGGERED TO ALLOW SKEW OF THE TRANSVERSE DUCT. THE END OF THE UNIT SHALL BE SKEWED TO THE REQUIRED ANGLE — SEE DETAIL 2.

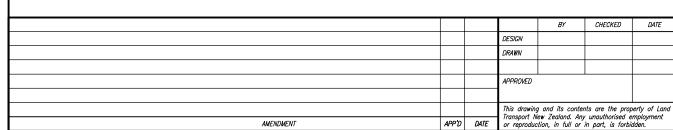


STIRRUPS SHALL BE PLACED PARALLEL TO THE LINE OF SKEW WITHIN 1m OF EACH END DIAPHRAGM. STIRRUPS ALONG THE SPAN SHALL BE PLACED NORMAL TO LONGITUDINAL STEEL WITH THE SKEW/NORMAL STIRRUP INTERFACE HAVING ADDITIONAL STIRRUPS IN A FAN ARRANGEMENT WITH THE SPECIFIED MAXIMUM STIRRUP SPACING ON THE OUTSIDE OF THE 'FAN'.

10. VOID FORMERS

SURFACES OF VOIDS ARE TO BE RENDERED IMPERMEABLE TO WATER PENETRATION FOR THE DESIGN LIFE OF THE UNIT, EITHER BY SURFACE TREATMENT OR USE OF HOLLOW OR LIGHTWEIGHT SOLID VOID FORMERS OR SUITABLE MATERIAL. PROPOSED METHOD SHALL BE TO THE ENGINEER'S CONSENT.

MAXIMUM WEIGHT OF VOID FORMER = 6kg/m









STANDARD PRECAST CONCRETE BRIDGE BEAMS

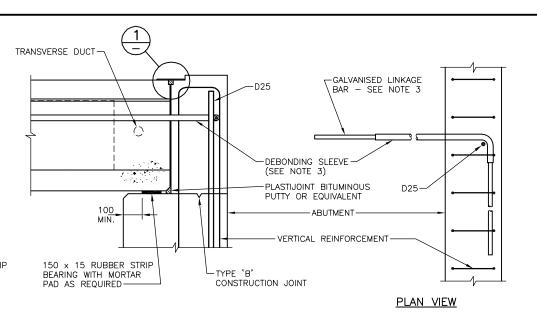
587mm DEEP DOUBLE HOLLOW CORE BEAMS - 12m & 14m SPAN UNIT DATA

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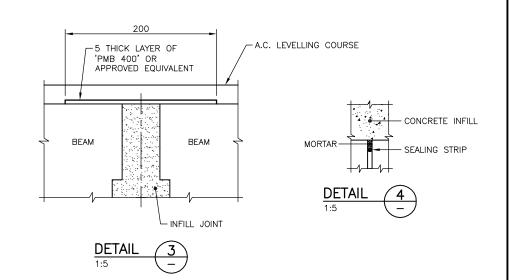
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ABUTMENT CONNECTION



NOTES:

- LINKAGE BAR DEBONDING SLEEVES MAY BE REPLACED WITH AN ALTERNATIVE BOND BREAKING MATERIAL OF EQUIVALENT THICKNESS. (E.G. 'DENSO' OR 'PROTECTO' TAPE).
- 2. LINKAGE BAR DETAILS AS SHOWN ARE SUITABLE FOR MOST DOUBLE HOLLOW CORE UNIT INSTALLATIONS. ALTERNATIVE CONNECTIONS CAN BE USED IF REQUIRED.
- 3. LINKAGE BARS TO BE GRADE 500E. THE DESIGNER SHALL DETERMINE THE REQUIRED LINKAGE BAR SIZE AND LENGTH ACCORDING TO THE BRIDGE FORM AND SEISMICITY OF THE BRIDGE SITE.
- 4. THIS DRAWING SHALL BE READ IN CONJUNCTION WITH THE LTNZ STANDARD BRIDGE BEAM SPECIFICATION (2007).
- ALTERNATIVE DETAILS TO THOSE SHOWN MAY BE USED WHEN APPROPRIATE.
- TRANSVERSE STRAND STRESSED TO 70% OF MINIMUM BREAKING LOAD (184kN/STRAND).



NZ TRANSPORT AGENCY

WAKA KOTAHI



STANDARD PRECAST CONCRETE BRIDGE BEAMS

587mm DEEP DOUBLE HOLLOW CORE - 12m &14m SPAN LINKAGE BAR & TRANSVERSE CONNECTION DETAILS

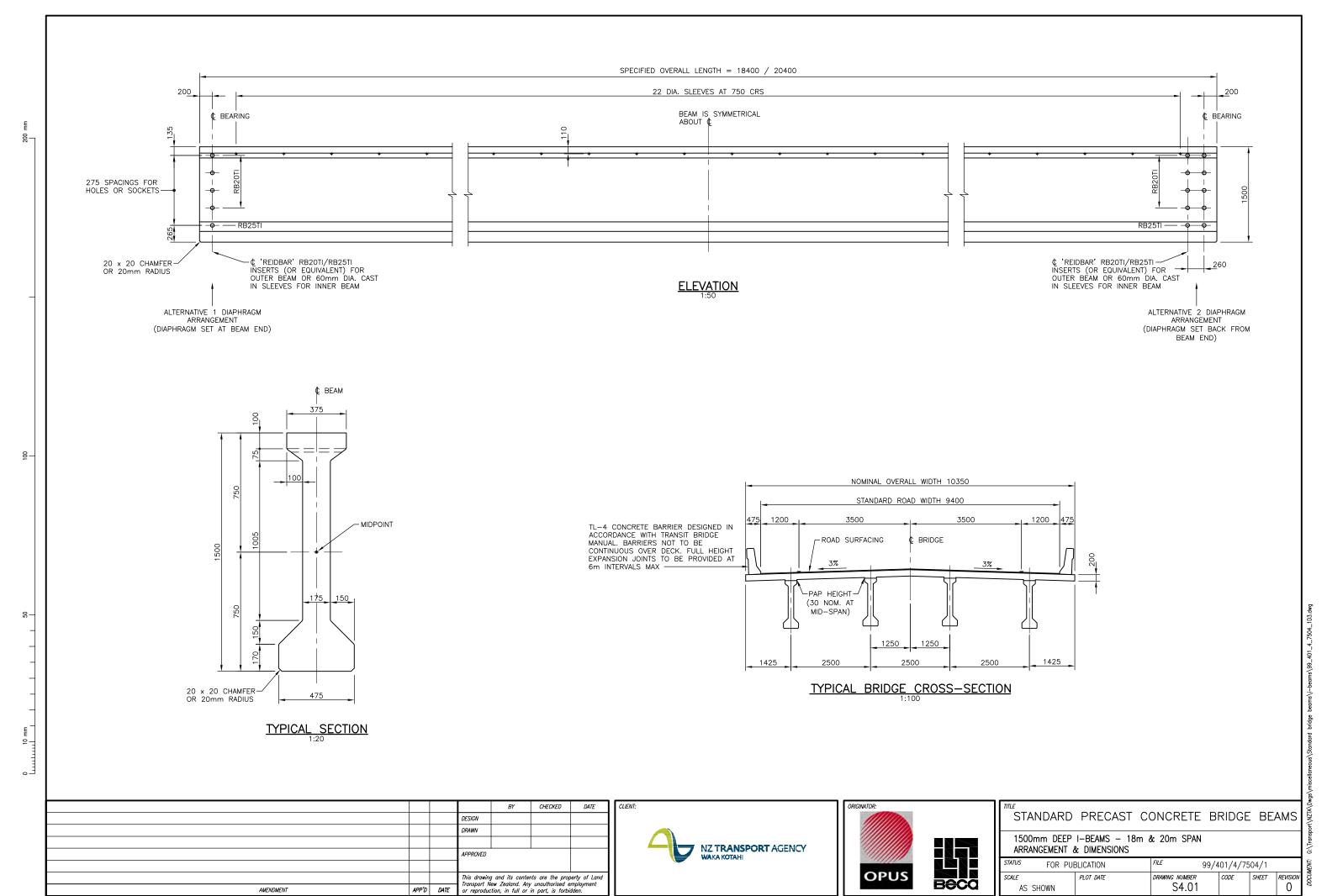
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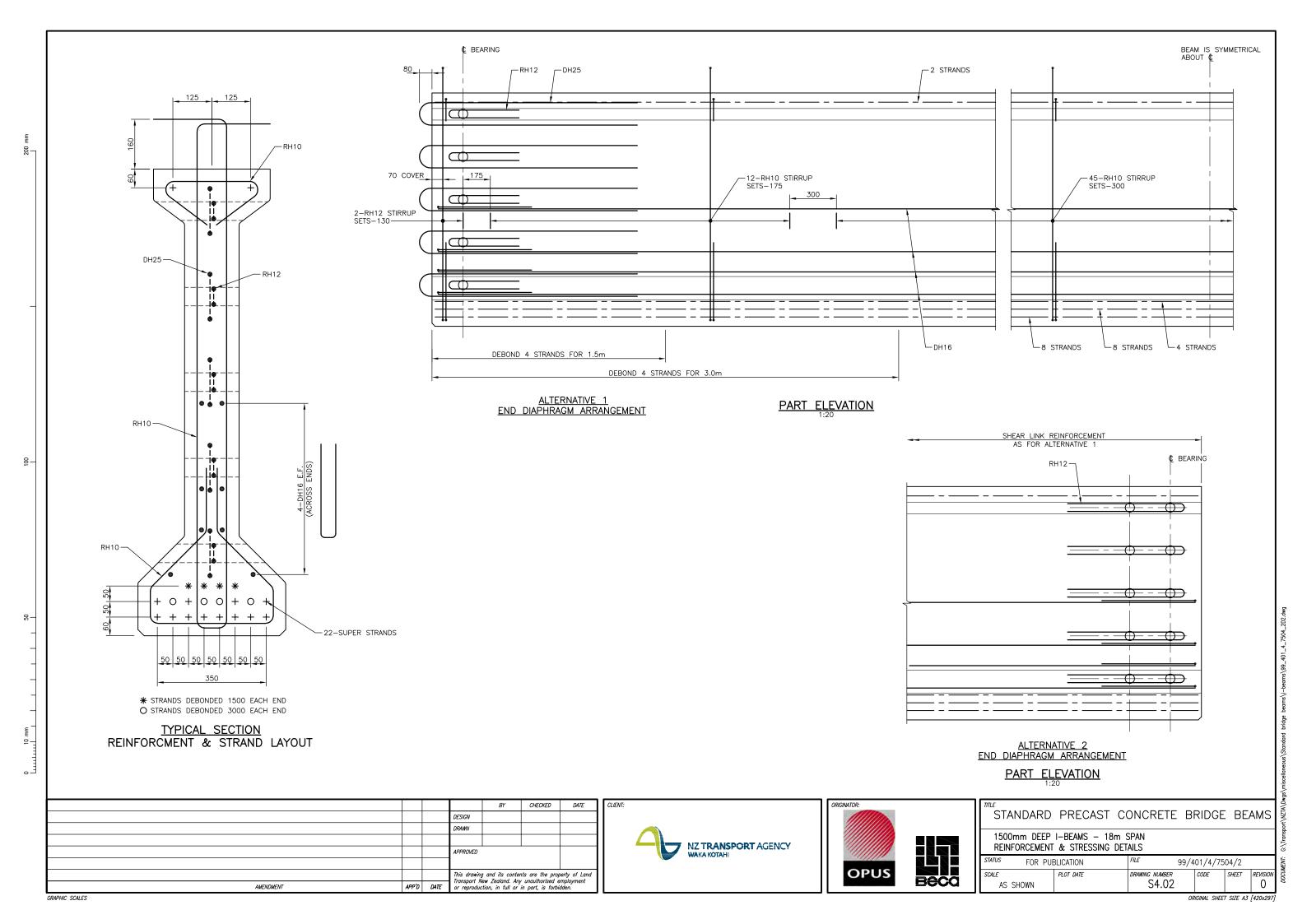
ORIGINAL SHFFT SIZF A.3 [420x297]

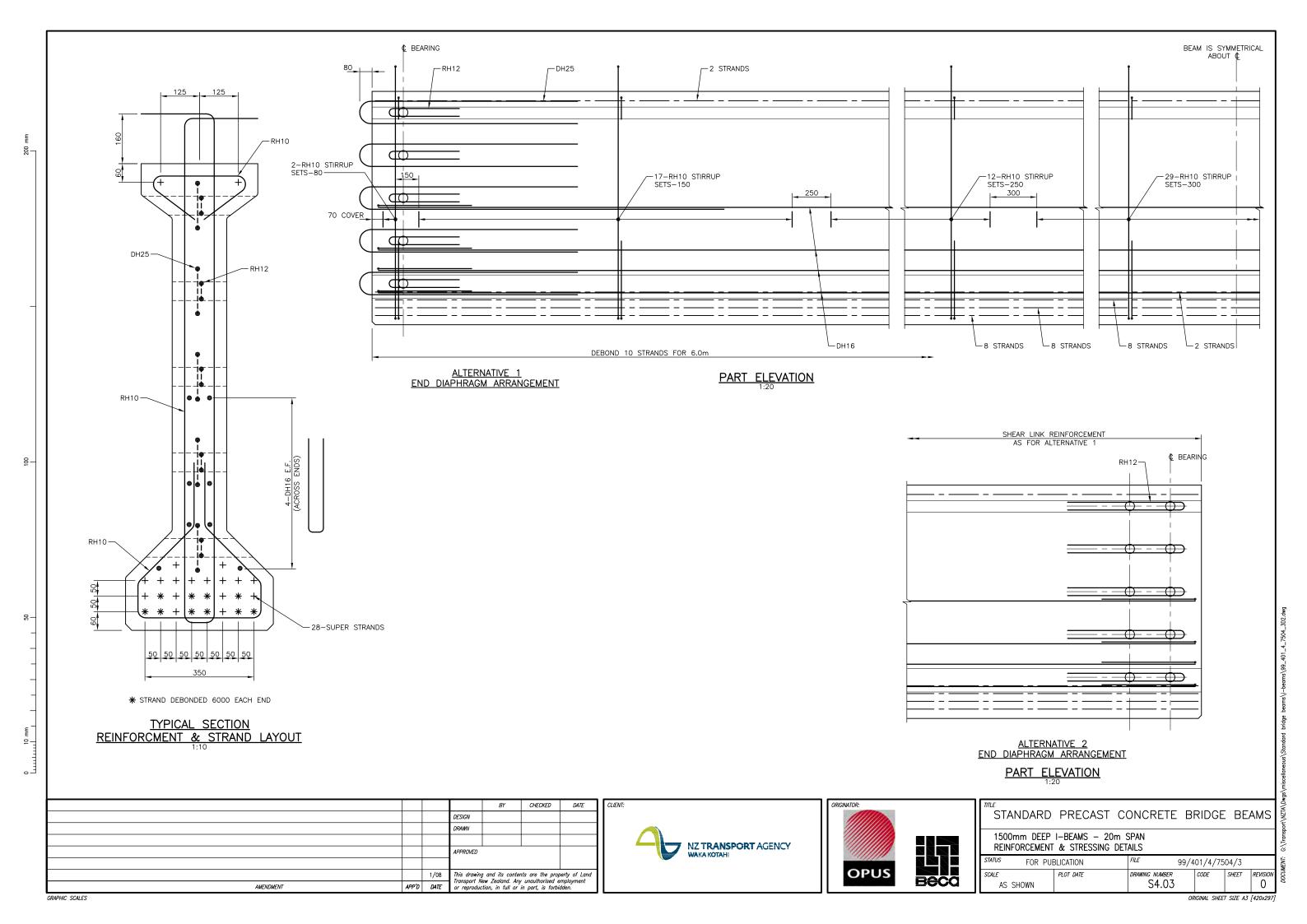
GRAPHIC SCALES

AMENDMENT

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AT TRANSFER - PRETENSIONING PRECAST BEAMS AT 28 DAYS IN SITU CONCRETE (DECK SLAB, DIAPHRAGMS) AT 28 DAYS _ 40MPa

2. REINFORCEMENT & PRESTRESSING

ALL SUPERSTRANDS SHALL BE 12.7mm SUPER 7 WIRE STRANDS COMPLYING TO AS/NZS 4672 AND ASSUMED TO HAVE A MINIMUM BREAKING LOAD OF 184kN PER STRAND WITH INITIAL LOADING AS FOLLOWS:

TOP TWO STRANDS TO BE LOADED TO 125kN PER STRAND

OTHER STRANDS TO BE LOADED TO 136kN PER STRAND

3. CONCRETE COVER (MINIMUM)

COVER TO ALL PRESTESSING COMPONENTS
COVER TO REINFORCEMENT UNLESS SHOWN OTHERWISE - 40mm - 40mm COVER ADJACENT TO CORED HOLES COVER TO BRIDGE DECK & ALL CAST IN SITU CONCRETE - 50mm

COVER TO BARRIER FIXING STEEL (WITHIN BARRIER) - 50mm

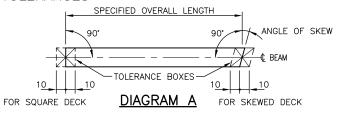
4. DESIGN LOADING

HN-HO-72 (INCLUDING SLS LIVE LOAD FACTOR OF 1.35)

5. SPECIFICATION

THIS DESIGN IS BASED ON LTNZ STANDARD BRIDGE BEAM SPECIFICATION (2007)

6. TOLERANCES



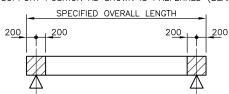
6.1. DIMENSIONS AT TIME OF ERECTION

ACTUAL OVERALL LENGTH AND SQUARENESS

a. THE UNDERSIDE OF THE BEAM FOR THE END 700mm SHALL BE TRUE PLANE. +5mm d. BEAM HOGGING (SEE SPECIFICATION) CROSS SECTION DIMENSIONS UP TO 0.5m +5mm CROSS SECTION DIMENSIONS 0.5m TO 2.0m...... . ±10mm HORIZONTAL BOW OF LONGITUDINAL AXIS .. 6.2. DIMENSIONS AT TIME OF ERECTION

a. LONGITUDINAL STEEL REINFORCEMENT.....b. LOCATION OF AN ITEM IN RELATION TO ANY OTHER ITEM WITHIN ITS GROUP OR TO THE MIDPOINT

EXTREMES OF VERTICAL LIFTING POINTS OR GROUND SUPPORT SHOWN HATCHED.
CENTRAL SUPPORT POSITION AS SHOWN IS PREFERRED (BEAM VERTICAL AT ALL TIMES).



BEAM SUPPORT & LIFTING POINTS

8. METHOD OF MANUFACTURE

BEAMS SHALL BE MANUFACTURED UNDER FACTORY CONDITIONS

9. SURFACE FINISHES

BEAMS a. TOP SURFACE

AS FOR TYPE B CONSTRUCTION JOINT (AS SPECIFIED IN NZS 3109)

b. SIDE SURFACE

FOR HATCHED AREAS ON DIAGRAM B
INNER BEAM BOTH SIDES — AS FOR TYPE B CONSTRUCTION JOINT AT
AREA OF CONTACT WITH DIAPHRAGMS
OUTER BEAM, INNER SIDE ONLY — AS FOR TYPE B CONSTRUCTION JOINT

AT AREA OF CONTACT WITH DIAPHRAGMS
REMAINING SIDE SURFACE ALL BEAMS — SMOOTH FINISH

ALTERNATIVE 1 ARRANGEMENT — AS FOR TYPE B CONSTRUCTION JOINT
ALTERNATIVE 2 ARRANGEMENT — SMOOTH FINISH WITH STRANDS CUT FLUSH

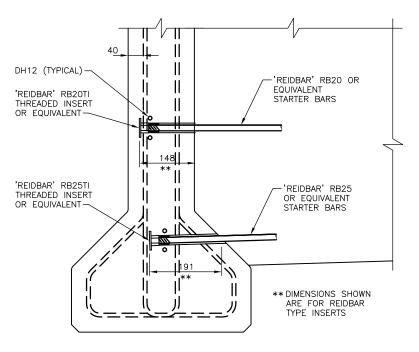
ALIERWATIVE Z ARKANGEMENT - SMOUTH FINISH WITH STRANDS CUT FLUSH AND PROTECTED WITH A MINIMUM OF 5mm EPOXY MORTAR UNDERSIDE SURFACE - SMOOTH FINISH
DIAPHRAGM SURFACE FINISH TO BE BASIC FINISH IN ACCORDANCE WITH LTNZ STANDARD BRIDGE BEAM SPECIFICATION (2007).

10. BEARING DESIGN DATA

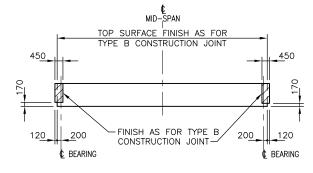
1	SPAN	F	REACTION (kN)	ROTATION (x10-6 RADIANS)			
	(m)	DEAD LOAD (UNFACTORED)	LIVE LOAD (1.35HN x I)	OVERLOAD (HO x I)	LIVE LOAD (1.35HN x I)	OVERLOAD (HO x I)	
	18	323	397	507	802	1001	
	20	357	406	510	1018	1251	

11. AGE AT DECK POURING

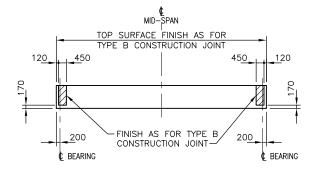
DECK TO BE POURED WITHIN 180 DAYS OF CASTING OF



TYPICAL END DIAPHRAGM STARTER BAR CONNECTION FOR OUTER BEAM



END DIAPHRAGM SET AT BEAM END (ALTERNATIVE 1 ARRANGEMENT)

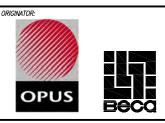


END DIAPHRAGM SET BACK FROM BEAM END (ALTERNATIVE 2 ARRANGEMENT)

DIAGRAM B

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			APPROVED			
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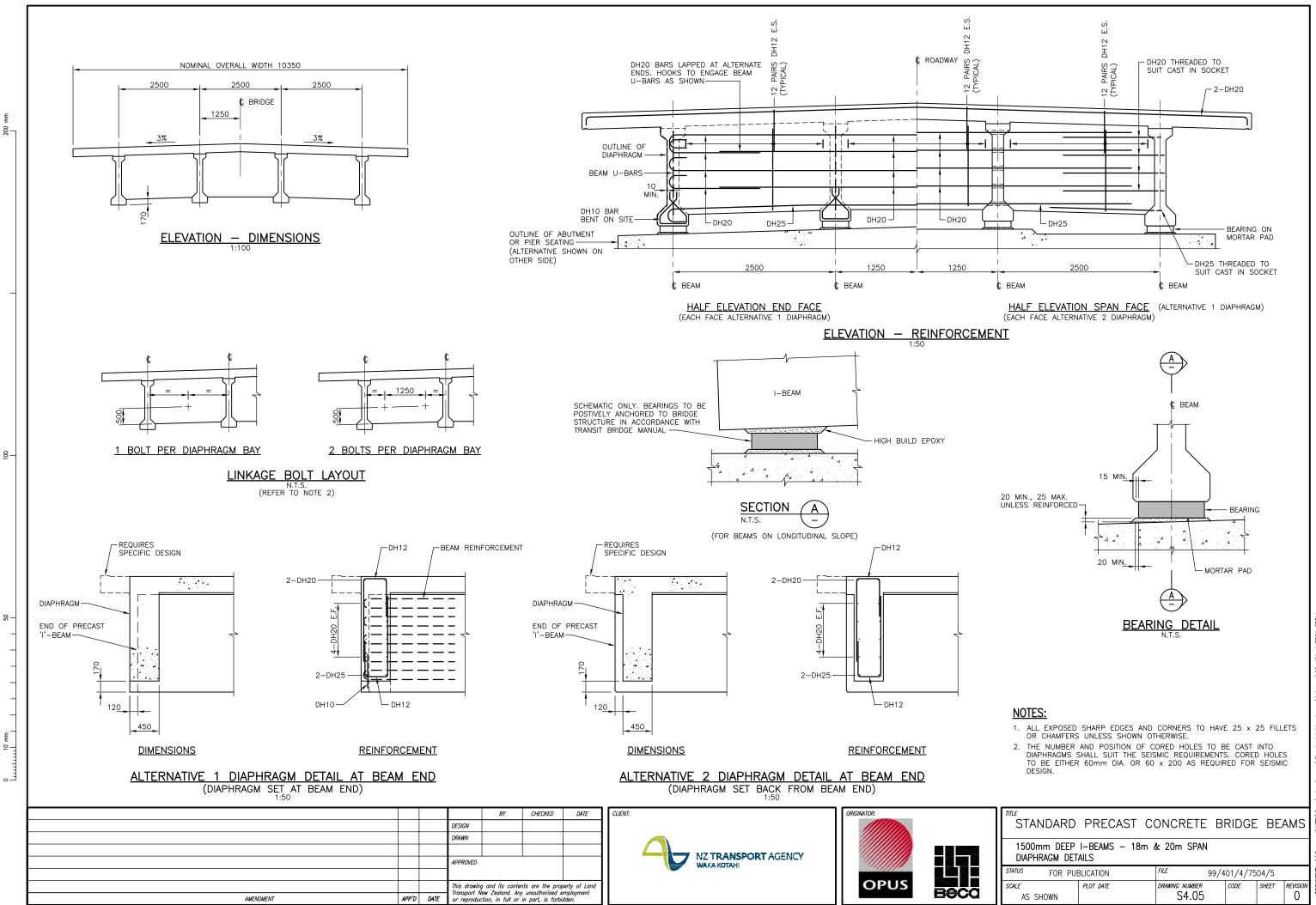
STANDARD PRECAST CONCRETE BRIDGE BEAMS 1500mm DEEP I-BEAMS - 18m & 20m SPAN	ort\NZTA\D
1500mm DEEP I-BEAMS - 18m & 20m SPAN	Transp

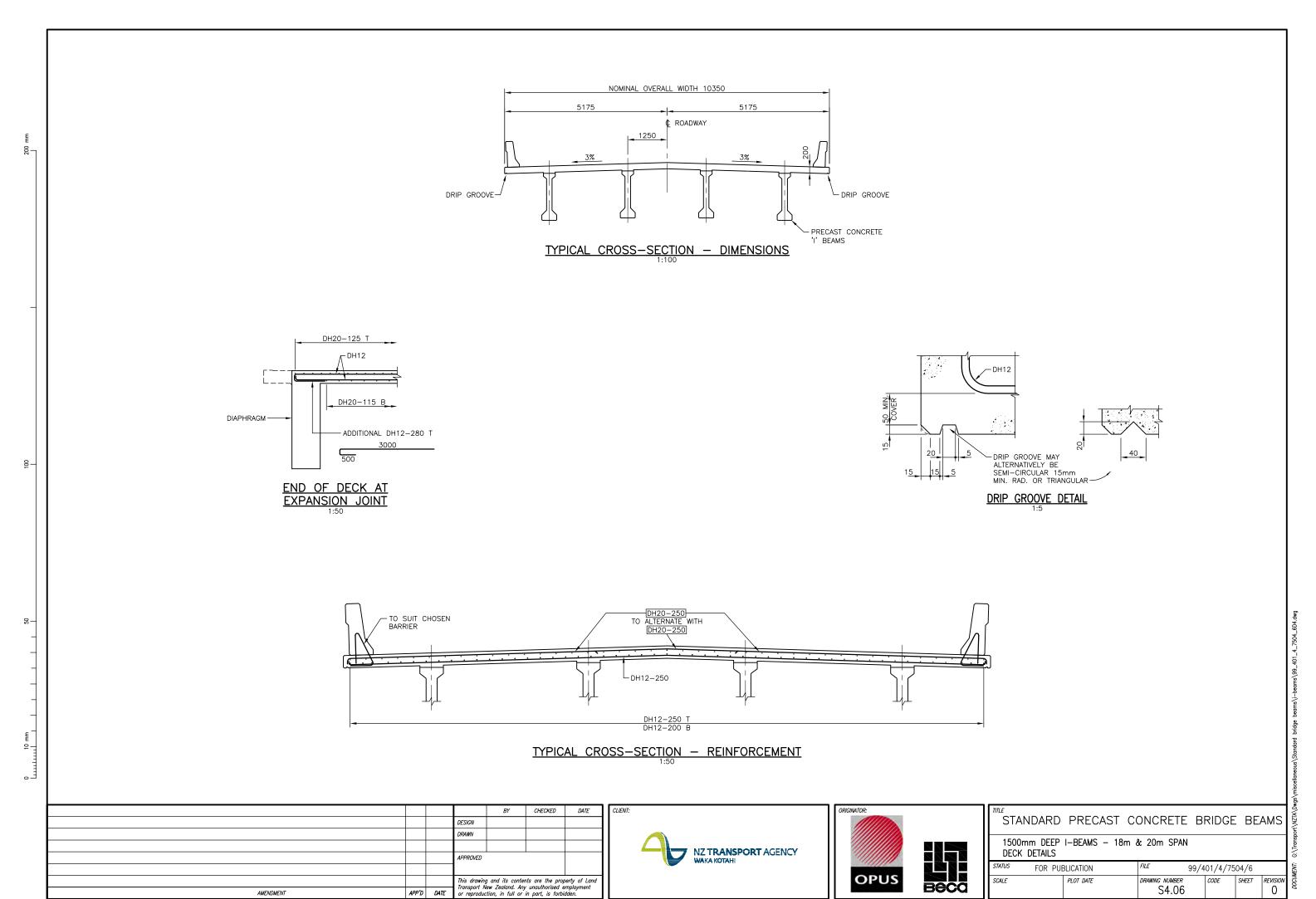
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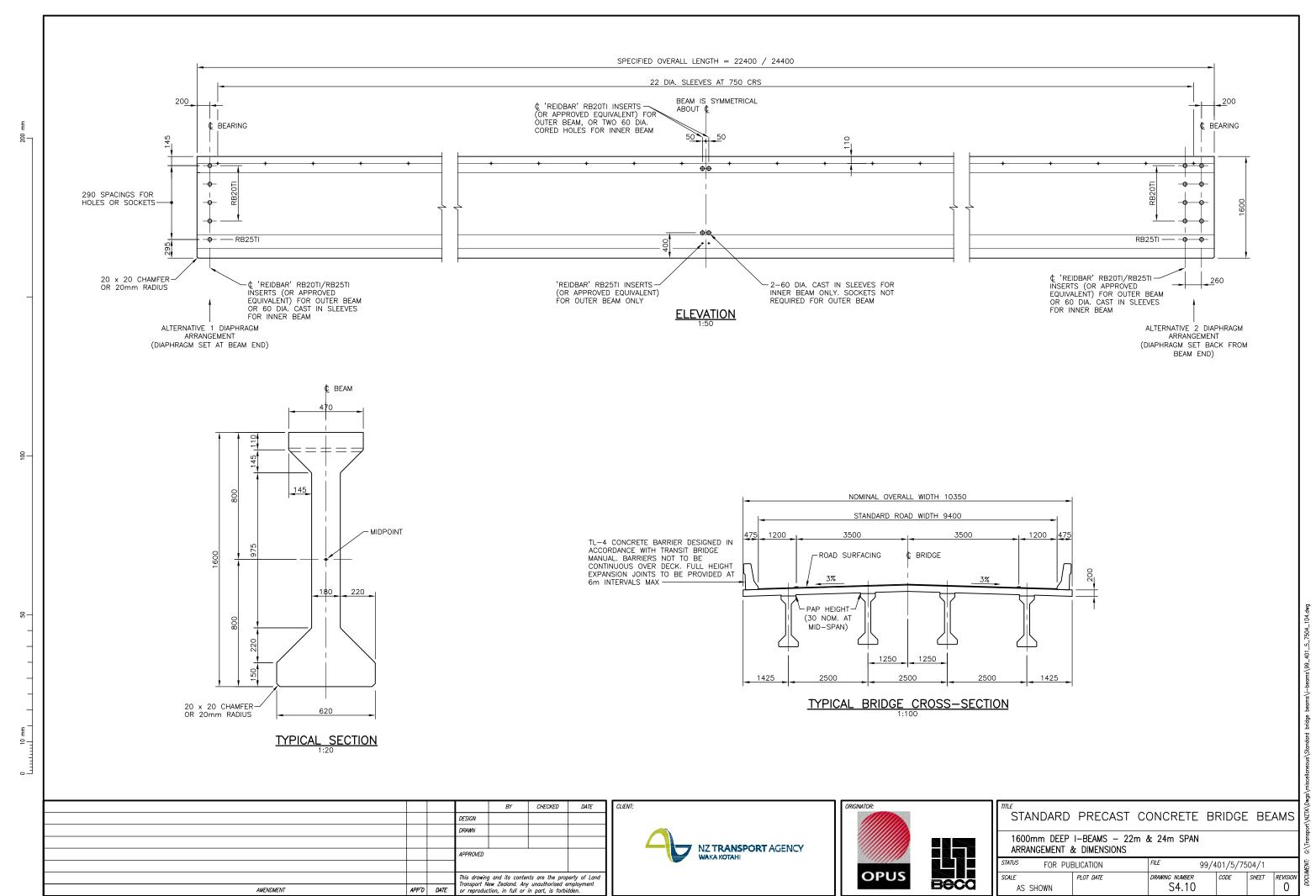
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ALE		PLOT DATE	DRAWING NUMBER		CODE	SHEET	R

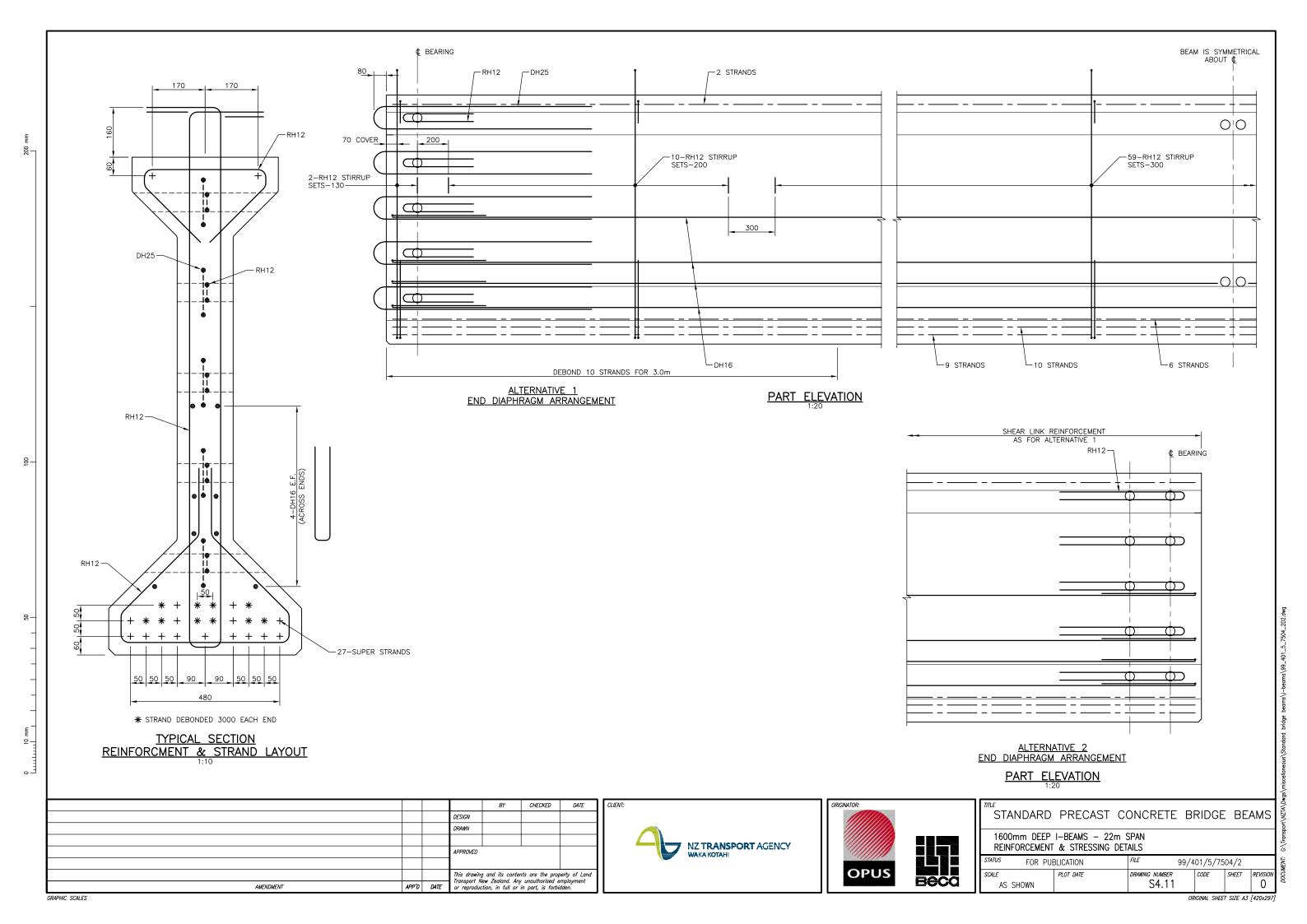
ORIGINAL SHEET SIZE A3 [420x297]

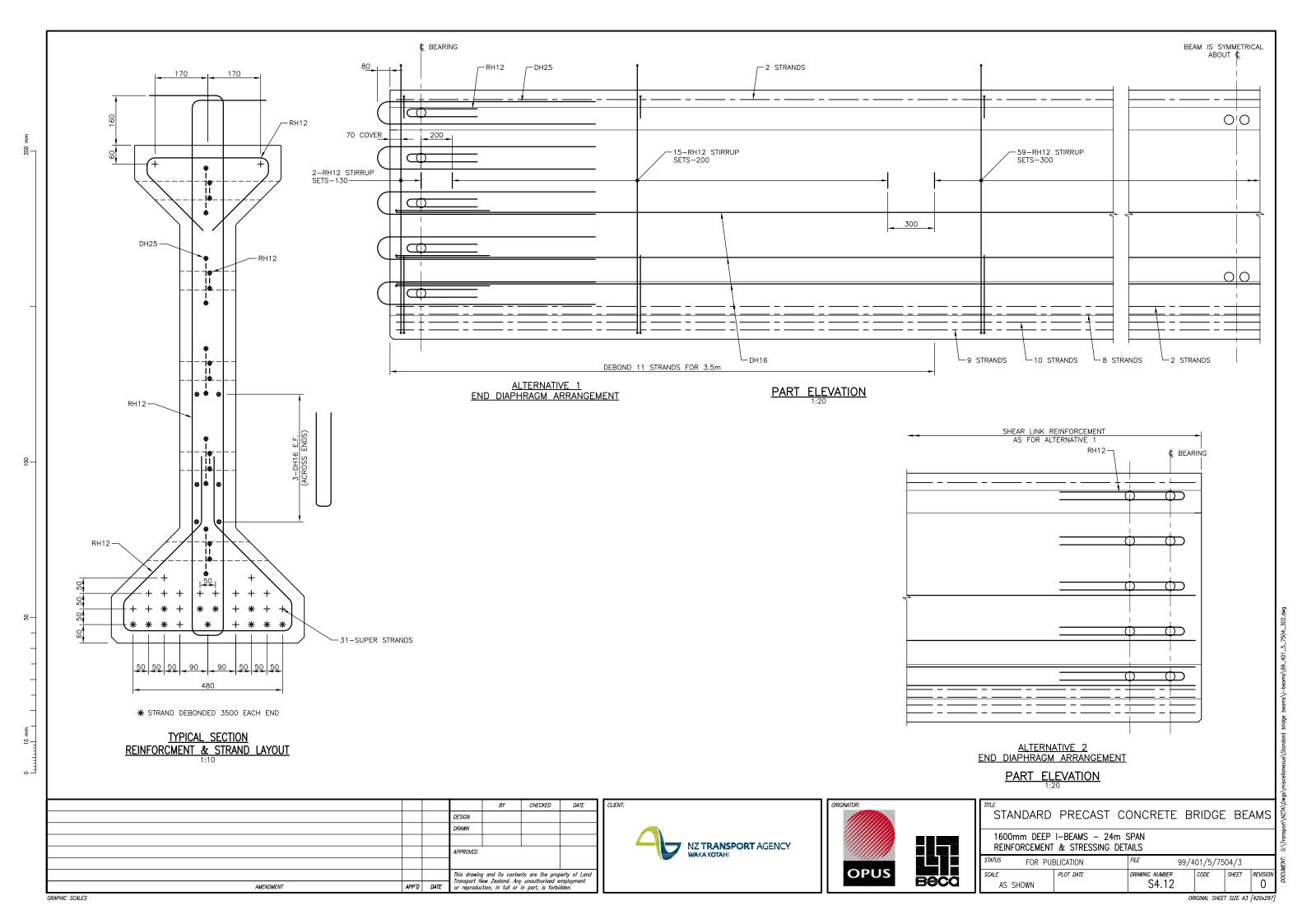
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2. REINFORCEMENT & PRESTRESSING

ALL SUPERSTRANDS SHALL BE 12.7mm SUPER 7 WIRE STRANDS COMPLYING TO AS/NZS 4672 AND ASSUMED TO HAVE A MINIMUM BREAKING LOAD OF 184kN PER STRAND WITH INITIAL LOADING AS FOLLOWS:

TOP TWO STRANDS TO BE LOADED TO 125kN PER STRAND

OTHER STRANDS TO BE LOADED TO 136kN PER STRAND

3. CONCRETE COVER (MINIMUM)

COVER TO ALL PRESTESSING COMPONENTS COVER TO REINFORCEMENT UNLESS SHOWN OTHERWISE - 40mm COVER ADJACENT TO CORED HOLES - 30mm COVER TO BRIDGE DECK & ALL CAST IN SITU CONCRETE COVER TO BARRIER FIXING STEEL (WITHIN BARRIER) - 50mm

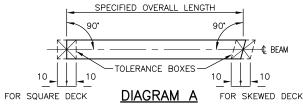
4. DESIGN LOADING

HN-HO-72 (INCLUDING SLS LIVE LOAD FACTOR OF 1.35)

5. SPECIFICATION

THIS DESIGN IS BASED ON LTNZ STANDARD BRIDGE BEAM SPECIFICATION (2007)

6. TOLERANCES



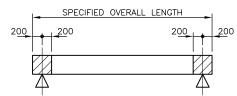
6.1. DIMENSIONS AT TIME OF ERECTION

OF THE BEAM ENDS.

ACTUAL OVERALL LENGTH AND SQUARENESS a. THE UNDERSIDE OF THE BEAM FOR THE END 700mm SHALL BE TRUE PLANE. ±5mm +5mm ±10mm HORIZONTAL BOW OF LONGITUDINAL AXIS ±20mm 6.2. DIMENSIONS AT TIME OF ERECTION a. LONGITUDINAL STEEL REINFORCEMENT......b. LOCATION OF AN ITEM IN RELATION TO ANY OTHER ITEM WITHIN ITS GROUP OR TO THE MIDPOINT . ±10mm

7. HANDLING

EXTREMES OF VERTICAL LIFTING POINTS OR GROUND SUPPORT SHOWN HATCHED. CENTRAL SUPPORT POSITION AS SHOWN IS PREFERRED (BEAM VERTICAL AT ALL TIMES).



c. PRETENSIONING PRESTRESSING STRANDS IN ANY DIRECTION

BEAM SUPPORT & LIFTING POINTS

8. METHOD OF MANUFACTURE

BEAMS SHALL BE MANUFACTURED UNDER FACTORY CONDITIONS

9. SURFACE FINISHES

BEAMS a. TOP SURFACE

AS FOR TYPE B CONSTRUCTION JOINT (AS SPECIFIED IN NZS 3109) b. SIDE SURFACE

FOR HATCHED AREAS ON DIAGRAM B INNER BEAM BOTH SIDES — AS FOR TYPE B CONSTRUCTION JOINT AT AREA OF CONTACT WITH DIAPHRAGMS

OUTER BEAM, INNER SIDE ONLY — AS FOR TYPE B CONSTRUCTION JOINT AT AREA OF CONTACT WITH DIAPHRAGMS REMAINING SIDE SURFACE ALL BEAMS - SMOOTH FINISH

c. END SURFACE

ALTERNATIVE 1 ARRANGEMENT — AS FOR TYPE B CONSTRUCTION JOINT
ALTERNATIVE 2 ARRANGEMENT — SMOOTH FINISH WITH STRANDS CUT FLUSH
AND PROTECTED WITH A MINIMUM OF 5mm EPOXY MORTAR
UNDERSIDE SURFACE — SMOOTH FINISH
DIAPHRAGM SURFACE FINISH TO BE BASIC FINISH IN ACCORDANCE WITH LTNZ STANDARD

AMENDMENT

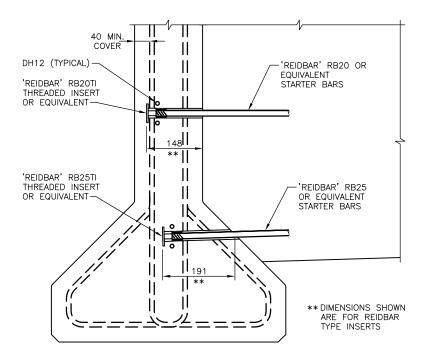
BRIDGE BEAM SPECIFICATION (2007).

10. BEARING DESIGN DATA

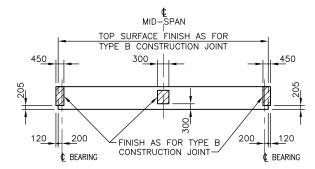
SPAN (m)	SPAN	F	REACTION (kN)	ROTATION (x10-6 RADIANS)			
	(m)	DEAD LOAD (UNFACTORED)	LIVE LOAD (1.35HN x I)	OVERLOAD (HO x I)	LIVE LOAD (1.35HN x I)	OVERLOAD (HO x I)	
	22	417	419	527	865	1045	
	24	452	435	541	1058	1259	

11. AGE AT DECK POURING

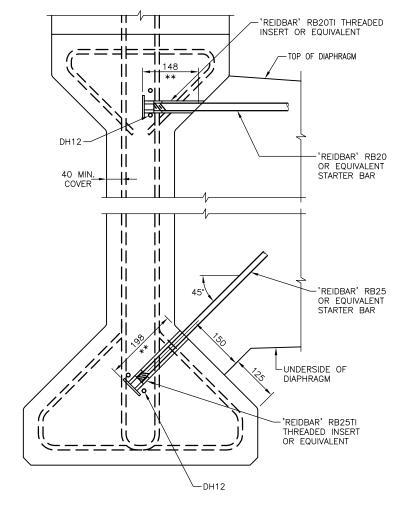
DECK TO BE POURED WITHIN 180 DAYS OF CASTING OF



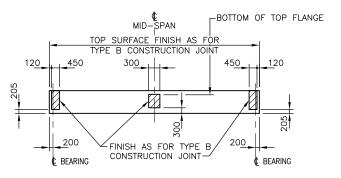
TYPICAL END DIAPHRAGM STARTER BAR CONNECTION FOR OUTER BEAM



END DIAPHRAGM SET AT BEAM END (ALTERNATIVE 1 ARRANGEMENT)

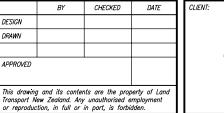


TYPICAL MID-SPAN DIAPHRAGM STARTER BAR CONNECTION FOR OUTER BEAM



END DIAPHRAGM SET BACK FROM BEAM END (ALTERNATIVE 2 ARRANGEMENT)

DIAGRAM B (SIDE ELEVATION)



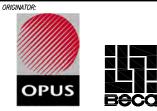
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DESIGN DRAWN

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APP'D DATE

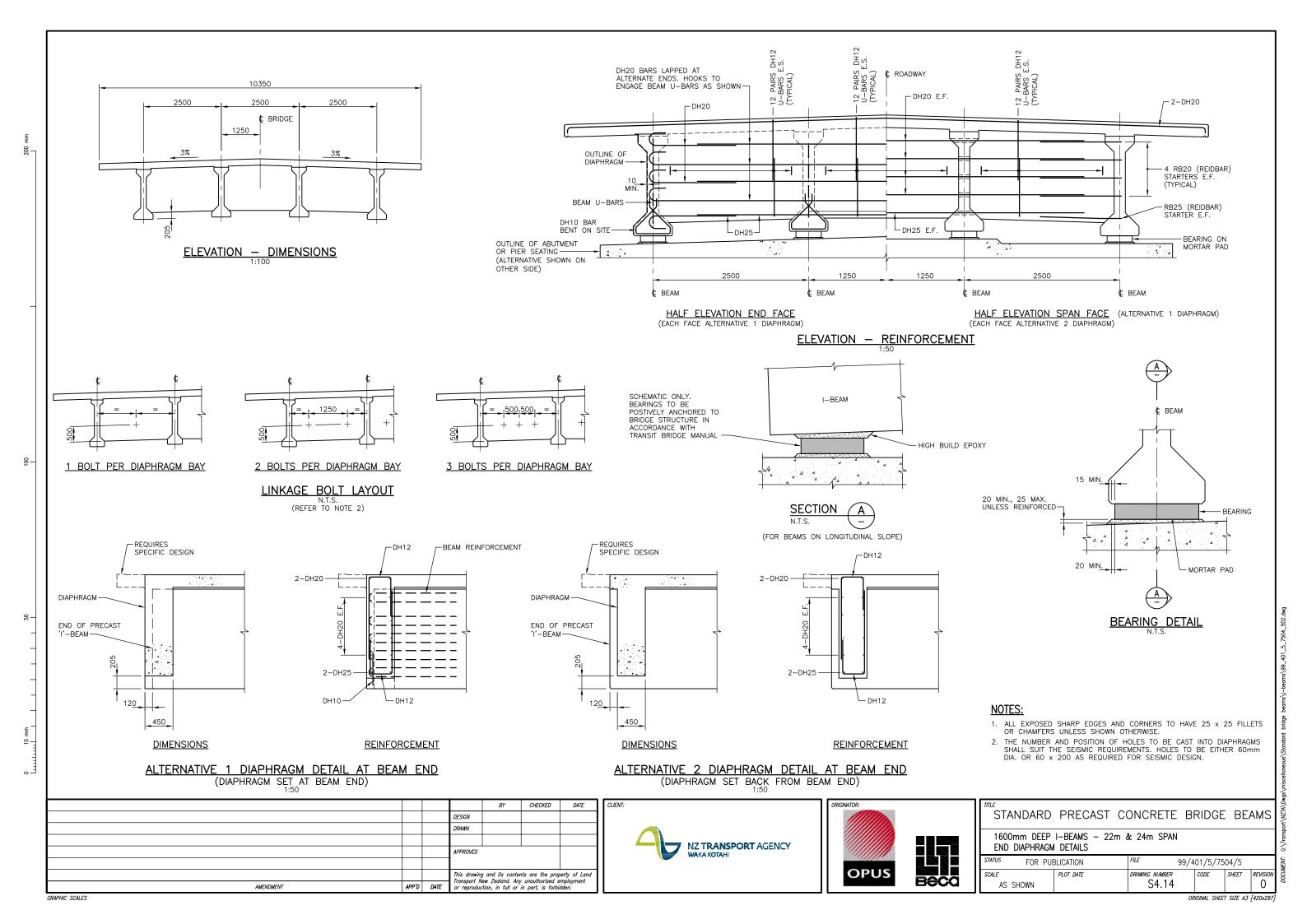


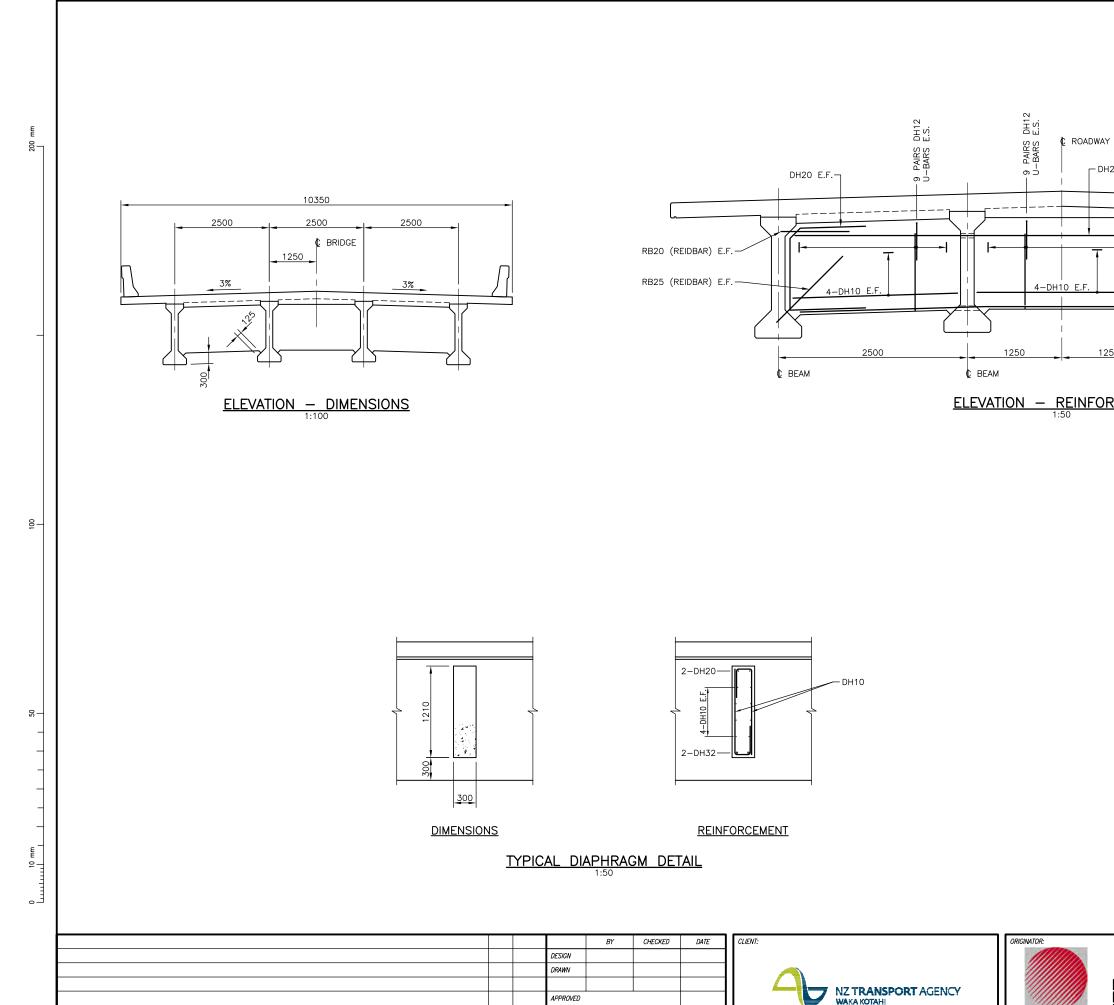


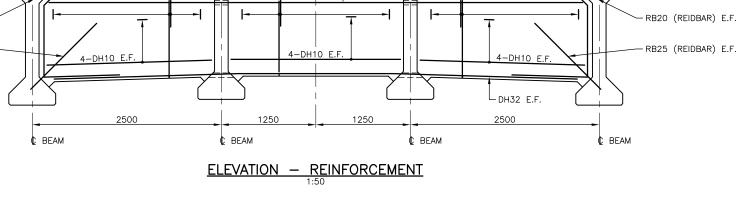
STANDARD PRECAST CONCRETE BRIDGE BEAMS

1600mm DEEP I-BEAMS - 22m & 24m SPAN

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DH20 E.F.

DH12 E.S.

⊢DH20 E.F.



ALL EXPOSED SHARP EDGES AND CORNERS TO HAVE 25 \times 25 FILLETS OR CHAMFERS UNLESS SHOWN OTHERWISE.

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			DESIGN					
			DRAWN					
			APPROVED					
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STANDARD PRECAST CONCRETE BRIDGE BEAMS

1600mm DEEP I-BEAMS - 22m & 24m SPAN MIDSPAN DIAPHRAGM DETAILS

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