Groundwater Monitoring Plan - Peka Peka to Ōtaki Project

FCCL-EV-MPN-008

Final C – September 2017

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Certification Record

| Revision | Action | Name | Position | Date | Signature |
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| | On behalf | of GWRC: | 1 22 | | |

1 PROJECT INTRODUCTION

1.1 Project Scope

This Groundwater Management Plan (GMP) has been developed for the State Highway 1 Peka Peka to Ōtaki Expressway. Works will occur over a 200 week period commencing 25 November 2016. The works will entail 12km, 4-lane Expressway, consisting of:

- 1.4Mm3 Earthworks
- 9km local road
- 10 No. Bridges, including 330m, Ōtaki River Crossing
- Ōtaki Intersection split
- East-West connections Ōtaki, Te Horo
- Grade separation Taylors Road
- 1.6km railway realignment

The works will follow a general programme of enabling works and site establishment, followed by rail realignment and bridge construction and then road construction.

The key Project parameters are given in Table 1.

Table 1: Contract details

| Item | Details |
|-------------------------------------|--|
| Project Name | Peka Peka to Ōtaki Expressway |
| Nature of Project | 12 Kms of new expressway and 10 new bridges |
| State Highway Classification | SH 1 |
| Commencement | 25 November 2016 |
| Project End Date | 07 January 2021 |
| Project Manager | Craig Pitchford (NZTA) |
| Principals Advisor - Engineer | Muir Coup |
| Principals Advisor – Engineer's Rep | Ron McFadyen (Opus) |
| Contractor | Fletcher Construction |
| Contract Manager | Andrew Goldie (Fletcher) |
| Councils with Jurisdiction | Greater Wellington Regional Council (GWRC) Kāpiti Coast District |
| | Council (KCDC) |

The potential for expressway construction to have an impact on groundwater was identified in the resource consent applications and also addressed in the Board of Inquiry conditions. The main impact is thought to be on groundwater users on the south bank of the Ōtaki River from a lowering of groundwater due to the excavation to create the southern approach to the new Ōtaki River Bridge. The lowering of groundwater on the south bank of the Ōtaki River has already occurred with the construction of the approaches to the current road and rail bridges and any effects are anticipated to be localised.

1.2 Groundwater Management Plan Scope

The scope of this Groundwater Management Plan is to outline how groundwater will be managed to ensure that there are:

- No significant changes to wetland hydrological conditions, and
- No permanent changes to the ability of existing bore owners to abstract water from their existing water supply bores.

This plan will define the means of measuring these outcomes through monitoring and reporting in accordance with the relevant consent conditions.

There are two aspects covered in the consents relevant to groundwater: groundwater monitoring and groundwater take and use.

Construction activities that may potentially impact groundwater are:

- Damming of groundwater.
- Earthworks construction in the vicinity of the deeper cuts located south of Ōtaki River and north of Ōtaki resulting in permanent draw down of groundwater.
- Subsoil drain constructions.
- Temporary dewatering during construction including undercut beneath bridge and other structures and undercut of the peats and other unsuitable materials during progressive excavation and backfilling.
- Bridge pile construction at the Waitohu Stream and Ōtaki River.
- Geotechnical investigation boreholes.
- Geotechnical instrumentation installation.

At this stage, the consent relating to take and use (NSP13/01.0046) is not intended to be utilised. Water for construction will be sourced from stored surface water, commercial providers or alternative consented sources. However, consent conditions GT.4-7 do have some wider implications for the management of the Project.

1.3 Referenced Consent Conditions

Table 2 below lists all resource consents relevant to groundwater management.

Table 2: Referenced Consent Conditions

| Consent Number | Consent Type | Activity |
|-------------------|------------------------------|---|
| NSP13/01.005 | Land use consent – s9(2) | Bore construction. |
| NSP13/01.007 | Water permit – s14(2)(a) | Damming and diversion of groundwater. |
| NSP13/01.008 | Discharge permit -s15 (1)(a) | Discharge of sediment-laden (including chemically-treated) water to water. |
| NSP13/01.0010 | Land use consent –s9(2) | Construction of boreholes (bores for bridge piles where they intercept groundwater). |
| NSP13/01.019 | Land use consent –s9(2) | Construction of boreholes (bores for bridge piles where they intercept groundwater). |
| NSP13/01.0045 | Land use consent s9(2) | Construction of boreholes (including bores for the purpose of abstracting groundwater). |
| NSP13/01.0046 | Water permit -s14(2)(a) | The take and use of groundwater for bore testing, dust suppression and construction purposes. |
| NSP13/01.0047 | Land use consent –s9(2) | Bore construction. |
| NSP13/01.0050 | Land use consent –s14(2)(a) | To dam groundwater and surface water via a new wetland. |
| NSP13/01.0051 | Water permit -s14(2)(a) | To permanently divert groundwater and surface water. |

Table 3 below lists all consent conditions relevant to groundwater management.

Table 3: Resource Consent Conditions

| Condition No. | Activity | Condition | Where addressed |
|------------------|---------------------------|---|--------------------|
| GT.1 | Groundwater monitoring | In managing the construction of the Project and the potential for changes to the groundwater levels to occur, the consent holder shall prepare a GMP. The consent holder shall submit the GMP to the Manager for certification at least 20 Working Days prior to Commencement of Construction. The GMP shall be submitted as an appendix to the CEMP. The GMP shall achieve the following outcomes: | Noted |
| | | a) That there shall be no changes to the groundwater levels that result in a significant change to wetland hydrological conditions; and b) That there shall be no permanent changes to the ability of existing bore | Noted Noted |

| Condition No. | Activity | Condition | Where addressed |
|------------------|---------------------------|--|-------------------|
| | | owners to abstract water (including at maximum consented rates or volumes) from their existing water supply bores. The definition of and means by which these outcomes will be measured and achieved shall be confirmed in the GMP and through the monitoring and reporting required in accordance with Conditions GT.2 and GT.3. Construction shall not commence until the consent holder has received the Manager's written certification of the GMP. | |
| GT.2 | Groundwater monitoring | The consent holder shall implement measures identified in the GMP to manage the groundwater level as part of any relevant SSEMP. The purpose of the measures to manage groundwater level in the GMP is to set out the BPO for groundwater monitoring and management and procedures to minimise changes in groundwater levels. The groundwater level detail in the GMP shall include information regarding: | |
| | | a) the schedule of groundwater monitoring bores identifying piezometer | 7.4 Appendix E |
| | | depth, screen length and geological unit; b) the locations of groundwater monitoring bores shown on plans; | Appendix B |
| | | c) the locations of monitoring stations; | Appendix B |
| | | d) duration of monitoring pre and post construction; | 2.4.6 |
| | | e) monitoring frequency; | 2.4.6 |
| | | f) monitoring methods; | 2.4.7 |
| | | g) reporting requirements including identification of departure from 'natural' groundwater levels; | 2.4.10 |
| | | h) details of mitigation options including, if appropriate, triggers for implementation of mitigation measures; | 2.5 |
| | | i) response management, including minimum flow cut-offs or any other restrictions on the operation of the construction water supply bores; | NA |
| | | j) review procedures; | 2.7 |
| | | k) definition of terms (e.g. 'significant change' and 'proactively monitor'); and | 2.4.6 |

| Condition No. | Activity | Condition | Where addressed |
|---------------|------------------------|---|-----------------|
| | | I) Surveys and monitoring of existing groundwater users within 500m of any abstraction well, and within 500m of the Project footprint between stations at chainage 4100 and 4400, unless otherwise agreed to by the Manager. | 2.4.3 |
| GT.3 | Groundwater monitoring | At 6 monthly intervals during construction, and for 12 months following completion of construction within each Stage, the consent holder shall review and report the results of monitoring as compared with expected effects on groundwater levels assessed from groundwater modelling and the established range of groundwater levels determined from groundwater monitoring prior to the construction. This review will have regard to the final construction methodology and progress at the time of the review. In addition, an annual report will be prepared and submitted to the Manager by 1 May each year that describes: a) The groundwater monitoring that has been undertaken since the Commencement of Construction; b) The actual and potential effects arising | 2.4.10 |
| | | from the groundwater level changes; c) Any remedial or mitigation measures | 2.5 |
| | | that have been implemented; d) Any changes to proposed remedial and mitigation measures; and | |
| | | e) Any changes proposed for the future monitoring programme or to alert levels. | 2.7 |
| GT.4 | Extraction rate | The consent holder shall ensure, in relation to groundwater takes and use a) The location, design, implementation and operation of the groundwater takes shall be in general accordance with the consent application. b) The rate at which water is taken from the water supply bores, other than for well testing, including simulation of effects shall not exceed: i) 110,000 m3/year at a maximum of 300 m3/day (cumulatively, across all bores); and | NA |

| Condition No. | Activity | Condition | Where addressed |
|---------------|------------------------|--|-----------------|
| | | ii) A maximum pumping rate of 35 litres/sec from each bore. | |
| GT.4A | Monitoring | The consent holder shall, subject to landowner access being provided, survey the water supply wells within 500m of any abstraction well, and within 500m of the Project footprint between the stations at chainage 4100 and 4400, to obtain a list of existing water supply wells, their depths, and abstraction volumes and rates. This shall be used to develop proactive monitoring measures to assess the impact of the abstraction of water for construction. | 2.4.3 |
| GT.5 | Monitoring | The consent holder shall undertake the following: a) Install and maintain a water meter on each water supply bore prior to the commencement of the take and for the duration of the abstraction from the point of take. The water meter shall measure both cumulative water abstraction and the instantaneous rate of take, and be capable of providing a pulse counter output; and b) The water meter shall be calibrated to ensure that the error does not exceed +/-5%. The water meter shall be installed in accordance with manufacturer's specifications. Advice Note: Where any consumptive water take for site office use exceeds 5 litres/second, the Resource Management (Measuring and Reporting of Water Takes) Regulations 2010 will apply. | NA |
| GT.6 | Mitigation measures | a) The consent holder shall proactively monitor and ensure that existing groundwater users (consented and permitted users) who cannot use their own water supply due to construction of the Project receive a replacement water supply. | 2.5.1 |
| | | b) The consent holder shall avoid adversely affecting KCDC's public water supply bores and shall ensure access to those bores for maintenance and servicing is maintained throughout the Project. | 2.3.2 |

| Condition No. | Activity | Condition | Where addressed |
|---------------|--------------|---|--------------------|
| | | c) A replacement water supply shall be provided within 2 Working Days of monitoring results identifying a change in ability to use their own water supply. | 2.5.1 |
| | | d) The replacement water supply shall provide not less than the volume and quality of water which existed before any change as a result of Construction of the Project. | 2.5.1 |
| GT.7 | Notification | The consent holder shall notify the registered drinking-water supply operators concerned and KCDC, as soon as reasonably practicable, if an event occurs due to the Project that may have a significant adverse effect on the quantity or quality of the water at any registered drinking-water supply abstraction point. | 2.3.1 2.6 |

Table 4 below identifies the Principal's Requirements relating to Groundwater Monitoring.

Table 4: Principal's Requirements

| Requirement No. | Requirement Type | Description |
|--------------------|-----------------------------|---|
| A4.4.1 | Monitoring Reports | A4.4.1.1 During construction the Contractor shall provide monitoring reports to the Engineer every 3 months providing detailed monitoring data on ground movements, groundwater and pore water pressures and variations in the characteristics, including strength gain, of underlying materials. The reports shall provide comparison of monitored results against the baseline design predictions in both tabular and graphical formats. Where results are not following the predictions, the Contractor shall detail their proposed design and/or construction strategy to address these issues. These reports shall also meet the reporting requirements of the resource consents, in particular water well monitoring reports. |
| A4.12.1 | Groundwater and Water Wells | A.4.12.1.1 The consents for the Project require groundwater monitoring and monitoring of the wells in the area, particularly where: a The cutting for the expressway to the south of Ōtaki River has the potential to lower the groundwater level in the area. b Extraction of water from water abstraction wells has the potential to affect the existing water takes |

| | | from water bores (wells) in the area, including consented and unconsented water takes. |
|---------|----------------------|---|
| | | A.4.12.1.2 The Contractor shall fulfil the consent requirements to monitor the groundwater levels and water wells in the surrounding area, as identified in the consents, and if any existing water takes are affected, then provide them a temporary water supply, or replacement water bores as may be appropriate. |
| A4.12.2 | Contaminated Land | A.4.12.2.4 Management of contaminated groundwater shall be allowed for in the Groundwater Management Plan (GWMP). |

2 GROUNDWATER MANAGEMENT

2.1 Introduction

There are two groundwater zones identified along the express route by GWRC. The location and characteristics of these groundwater zones are set out in Table 5.

Table 5: Groundwater Zones in Project Area

Ōtaki Groundwater Zone

The Ōtaki groundwater zone extends across the coastal plain from the southern margin of the Ōtaki River valley to the northern boundary of the Wellington Region, combining the existing Ōtaki and Waitohu groundwater zones. To the east of SH1 the southern boundary of the Ōtaki groundwater zone follows the prominent river terrace that forms a hydraulic divide between the Q2 gravels of the Hautere Plain and the Q1 alluvium of the Ōtaki River floodplain. To the west of SH1 this boundary becomes less well defined, particularly near the coast where it is partially obscured by Holocene sand deposits. The eastern boundary follows the approximate contact between the Q5 and Q6 alluvial terraces (and associated alluvial fans) and the greywacke bedrock of the Tararua foothills.

Primary surface water features in the Ōtaki groundwater zone include the Ōtaki River and the Waitohu Stream. Both streams exhibit significant interaction with the adjacent unconfined aquifer, losing water downstream of their emergence from the Tararua foothills and gain appreciable base flow in their lower reaches. Some smaller spring-fed streams also occur on the Ōtaki River floodplain including Waimanu Stream east of SH1 and Rangiuru Stream which drains into the Ōtaki River near Ōtaki Beach. Although the groundwater resources in the Q1 gravels and underlying Q5/Q6 alluvium are laterally continuous across the zone (essentially forming a single groundwater resource), due to the varying nature of groundwater/surface water interaction in the Ōtaki River and Waitohu Stream catchments, the larger zone is sub-divided into two smaller units (the Ōtaki sub-zone and Waitohu subzone) for groundwater allocation purposes to reflect the differing nature of groundwater / surface water interaction in these catchments.

Te Horo Groundwater Zone

The Te Horo groundwater zone extends across the coastal plain from the Ōtaki River in the north to a southern boundary running parallel with Peka Peka Road combining the existing Hautere and Coastal groundwater management zones. The eastern boundary follows the contact between the coastal plain and the Tararua foothills.

Groundwater resources in this area are hosted in a thick succession of alluvial gravel materials accumulated on the alluvial fan formed by the Ōtaki River over the late Quaternary period. These materials host a succession of waterbearing intervals which become increasingly well confined at depth. West of the prominent marine terrace running parallel to SH1, the upper portion of the alluvial materials have been replaced by Holocene sand and gravel deposits up to 35 metres thick accumulated as a result of shoreline progradation over the past 6,500 years forming a shallow unconfined aquifer system. The primary surface water feature in the Te Horo groundwater zone is the Mangaone Stream which traverses the coastal plain to reach the coast at Te Horo Beach. The Mangaone stream loses water to the underlying unconfined aquifer as it crosses the Hautere Plain and gains flow from the unconfined sand aquifer over its lower reaches. Significant wetland areas occur in the Te Hapua complex located near Peka Peka

Source

Kapiti Coast Ground water Resource Investigation Proposed Framework for Conjunctive Water Management. pdf

Appendix A shows locations where groundwater may be affected by the construction works, including wetlands, deep excavations and bridge piles. Appendix B shows the locations of existing and proposed geotechnical standpipes, groundwater monitoring stand pipes and groundwater wells which will be monitored during construction.

A schedule of the existing groundwater monitoring bores identifying the piezometer depth, screen length and geological unit is presented in Appendix C.

2.2 Groundwater Conditions

Table 6 presents the expected groundwater conditions at the location of proposed deep cuts which may extend below groundwater level, and existing wetlands which may be affected by the proposed earthworks.

Table 6: Expected Ground Conditions

| Location | Description | | | | |
|---|--|--|--|--|--|
| Deep cuts north of Ōtaki (Chainage 01220 to 01440) | The cut for the expressway formation and associated subsurface drainage is not expected to extend below the observed groundwater level in this area. | | | | |
| Ōtaki North Wetlands (Chainage 01440 to 01640) | The existing wetlands will be removed as part of the expressway construction and will be replaced with lined wetlands. The pavement subgrade is expected to be at or very close to the seasonal high groundwater levels observed in the standpipes in the vicinity of the Ōtaki North wetlands. The typical groundwater level is 1 to 3m below the seasonal high. Subsoil drainage will be constructed which will result in the drawdown of groundwater up to 1m below the seasonal high water level but not below the seasonal fluctuations. | | | | |
| Deep cuts south of Ōtaki River (Chainage 03880 to 04800¹) | The cut for the expressway formation and associated subsurface drainage is expected to extend below the observed groundwater level in this area. This may result in permanent groundwater draw down of up to 2m below the water level. The draw down is expected to be greatest immediately adjacent to the excavation and diminish with distance from the excavation. The area groundwater regime immediately to the west has already been modified by the construction of the rail cutting on the south bank of the Ōtaki River. | | | | |
| Mary Crest Wetland (Chainage 10120 to 10220) | The local road and expressway formations are on embankments elevated above the wetlands and groundwater surface. There will be no connection or outlet to drainage that is likely to lower the groundwater levels in this area. | | | | |
| Deep cuts south of Mary Crest (Chainage 10260 to 10620) | The cut for the expressway formation and associated subsurface drainage is expected to extend below the observed groundwater level in this area. This will result in permanent groundwater draw down of up to 3m below the seasonal high water level. The draw down will be greatest immediately adjacent to the excavation and will diminish with distance from the excavation. | | | | |

Note: 1. Chainages are where the proposed expressway is 2.5m below existing ground levels

2.3 Existing Groundwater Users

A schedule of existing groundwater users potentially affected by the construction works is presented in Appendix D. These typically comprise bores that are a) operated under permitted activity or RMA conditions permitting takes without requirement for consent, b) consented groundwater takes, and c) registered drinking water users (which are also likely consented).

Although a significant number of historical groundwater users have been identified by reviewing historical data, information on current users is sparse. Many of the possible users' bores predate the Resource Management Act. Further work will be done to determine the current users and in particular the aquifers they draw from and the quality and quantity of takes.

2.3.1 Registered drinking water users

A register of Drinking Water Suppliers is maintained by the Ministry of Health. The register can be accessed at http://www.esr.cri.nz/assets/WATER-CONTENT/Images-and-PDFs/RegisterofSuppliers-2016a.pdf.

The registered drinking water suppliers in the wider Project area are;

• Gary Road Water Committee

- c/- 61 Gary Road, Peka Peka, Kāpiti .
- Community: GAR003 Gary Rd Water Supply
- Size: Neighbourhood
- Volume Capability: -
- Category: Networked Supply
- Source: G01100 Gary Rd Bore

The Gary Road supply bore is located approximately 600m beyond the southern boundary of the Project. The nearest construction works to the supply are filling activities. The water bores are closer to the already completed section of expressway between MacKays and Peka Peka. It is considered that there will be no impact on the water supply from the Project.

Kāpiti Coast District Council (A0409) Private Bag 60601, Paraparaumu

- Community: HAU003 Hautere
- Size: Minor
- Volume Capability: 1,200 m3/day
- Category: Networked Supply
- L Source: S00081 Bores next to Ōtaki River
- Community: OTA003 Ōtaki
- Size: Medium
- Volume Capability: 8,986 m3/day
- Category: Networked Supply
- Source: G00303 Rangiuru Road Bores

- Source: G00302 Tasman Road Bores

Kāpiti Coast District Council (KCDC) have advised that the 3 groundwater supply wells for the Ōtaki Water Scheme are more than 2.5km from the proposed expressway alignment. These wells are unlikely to be affected by the proposed construction.

• Te H2Oro Water Company Ltd

- PO Box 72, Te Horo

Community: TEH008 Te H2Oro Water Supply

Size: NeighbourhoodVolume Capability: -

Category: Networked SupplySource: G00993 Te H2Oro Bore

The company's bores are located to the east of the construction area and are unlikely to be affected.

2.4 Groundwater Monitoring

Groundwater monitoring has already been undertaken by Opus and AECOM. Refer to report titled Peka Peka to Ōtaki Expressway, Groundwater Monitoring Results, December 2014 to December 2016 (Opus 2016) and Notice to Contractor NTC18 in Appendix E. This information along with further monitoring prior to construction commencing will be used to establish current groundwater levels. A schedule of all groundwater monitoring bores in Appendix C presents details of the bore location and screen details.

The existing wells which can be located and are still in working condition will continue to be monitored. A number of monitoring bores are located within the construction footprint. These monitoring bores will be destroyed and it is not intended to replace them. This is with the exception of monitoring bores between chainage 3900 and 4500 (south bank Ōtaki River) where the ground level will be lowered below the current groundwater level. Table 7 shows that 2 monitoring bores will be destroyed and replaced by 6 new ones.

Table 7: Groundwater Monitoring Standpipes South Bank Ōtaki River

| Monitoring bore | Location | Comment |
|-----------------|----------|---|
| BH 810 | 4000 | To be installed and baseline established against BH 236 |
| BH 812 | 4000 | To be installed and baseline established against BH 236 |
| BH 236 | 4000 | Will be destroyed in construction process |
| BH 818 | 4100 | To be installed and baseline established against BH 236 |
| BH 819 | 4100 | To be installed and baseline established against BH 236 |
| BH 820 | 4400 | To be installed and baseline established against BH 214 |
| BH 821 | 4400 | To be installed and baseline established against BH 214 |
| BH 214 | 4500 | Will be destroyed in construction process |

2.4.1 Further information

The Opus report has identified a number of potential groundwater users but does not include information on the nature of the take, such as: well location, well depth, screening levels and take volumes and rates. Further work will be undertaken to gather this information to more accurately define potentially affected parties.

2.4.2 River levels

The shallow aquifer next to the Ōtaki River is expected to be strongly influenced by the flow level in the Ōtaki River. Information on flow levels will be collected and correlated against the levels in the stand pipes and existing bores. The purpose of this is to determine how strong the influence of the river on the groundwater levels is.

2.4.3 Groundwater wells to be monitored

As per Condition GT.2 we will undertake surveys and monitoring of existing groundwater users within 500m of any abstraction well, and within 500m of the Project footprint between stations at chainage 4100 and 4400. Refer to Appendix B showing the location of groundwater wells to be monitored during construction. The monitoring of existing bores is subject to being given permission by bore owners to undertake monitoring.

2.4.4 Groundwater monitoring standpipes

Groundwater monitoring standpipes (Table 7) will be installed between the proposed deep excavations and the groundwater wells which are being monitored. The purpose of these standpipes is to identify the impact of the earthworks at the designation boundary to provide early warning of potential impact to the groundwater wells. Locations of proposed groundwater monitoring standpipes in BH818 to BH821 are shown on Figure PP2Ō-FG-GT-0-0003 in Appendix B. The standpipes will be constructed with the response zone just below the lowest point of the expressway cut. The standpipe to the east of the road alignment will be used to provide a control.

2.4.5 Geotechnical standpipes

Groundwater levels in both existing and proposed geotechnical standpipes will also be monitored to assess the impact of construction works on groundwater. The locations of the existing and proposed geotechnical standpipes are shown on Figures PP2Ō-FG-GT-0-0001 to 0008 in Appendix B. Geotechnical standpipe piezometers which are located within the earthworks footprint will be decommissioned during construction and will not be replaced. The Monitoring Bore Schedule in Appendix C has identified those bores that are expected to be within the earthworks footprint.

2.4.6 Duration and frequency of pre and post construction monitoring

Monitoring of groundwater monitoring standpipes will be done on a monthly basis following installation and continue for 12 months following construction completion as per condition GT.3. Monitoring frequency for wells and standpipes within 100m of excavations which extend below the seasonal high groundwater level will be proactively monitored weekly during excavation and for 1 month following completion of the excavation. Monitoring of the geotechnical standpipes will be

monthly until either access is no longer available (due to construction of the expressway) or will continue for 12 months following construction completion. Additionally, data from wells (in 200 series) and the geotechnical standpipes shown in the Figures PP2Ō-FG-GT-0-0001 to 0008 in Appendix B is available from May 2012 and will assist in assessing construction impacts.

2.4.7 Monitoring methods

Groundwater levels in standpipes and groundwater wells will be monitored manually using a water level meter to measure the depth of groundwater below the ground surface.

A water level logger will be installed in the groundwater monitoring standpipes to provide continuous water level monitoring. The water level logger will record water levels at least every minute. Data from the level loggers will be downloaded monthly or weekly during the construction phase.

2.4.8 Assessment

An assessment of the monitoring data collected prior to commencement of bulk earthworks will be undertaken. The assessment is to determine what would be a significant change in groundwater levels to impact upon existing groundwater users. This will then be used to set trigger levels. The assessment will consider;

- Individual bore data against all bores,
- Seasonal trends,
- Climatic conditions, and
- River levels

2.4.9 Trigger levels

Monitoring of the groundwater at the existing and proposed piezometers, plus the private wells (where permission has been obtained) will be undertaken on a regular basis until the commencement of bulk earthworks, anticipated for February 2018. Prior to the commencement of bulk earthworks, a revised GMP will be submitted to GWRC for certification. The revised GMP will provide groundwater trigger levels as 'alert levels' which will require additional monitoring, or 'action trigger' which will require mitigation measures.

The alert trigger will be set to respond to a change in groundwater level between the control locations and those in the impact zone. The action trigger level is set to respond to a change in groundwater level that will adversely affect water yield and is not a seasonal effect. If trigger levels in the monitoring wells are exceed then additional assessment will be undertaken (refer to following sections 2.5 and 2.6) and mitigation measures implemented if required.

The interim trigger levels are as follows;

• Alert level: a 7.5% decrease groundwater level from maximum depth level (based upon the groundwater range),

 Action level: a 10% decrease groundwater level from maximum depth level (based upon the groundwater range)

Prior to the commencement of bulk earthworks a schedule of groundwater triggers will be submitted to GWRC for approval. The schedule will identify monitoring bores installed for the Project and private groundwater bores within the monitoring zone. The schedule will provide location information, monitoring results to date and the trigger levels. In addition a report will provide a record of the efforts made to gain access to private wells for monitoring the impacts on groundwater users.

2.4.10 Reporting Requirements

Groundwater monitoring reporting will take place at 6-monthly intervals prior to construction, 3-monthly during bulk earthworks and for 12 months following completion of construction within each Stage.

These reports will provide a brief summary of seasonal conditions (e.g. rainfall/groundwater levels +/- "normal" along with any anomalies/changes/trends).

In addition, an annual report will be prepared and submitted to the Manager by 1 May each year that describes:

- a) The groundwater monitoring that has been undertaken since the commencement of construction;
- b) The actual and potential effects arising from the groundwater level changes;
- c) Any remedial or mitigation measures that have been implemented;
- d) Any changes to proposed remedial and mitigation measures; and
- e) Any changes proposed for the future monitoring programme or to alert levels.

2.5 Mitigation Techniques

2.5.1 Groundwater within wetlands

If groundwater levels within the existing wetlands which are to remain, are affected by the earthworks or associated subsurface drainage then mitigation measures will be developed. These mitigation measures may include, but are not limited to:

- Redesign of the surface water system in vicinity of the wetland
- Reforming the wetland with a lining system

2.5.2 Quality

The potential to contaminate groundwater from drilling activities will be avoided by ensuring all drilling and piling is undertaken in accordance with "NZS 4411:2001 Environmental standard for drilling of soil and rock". All drilling contractors will be contractually obligated to meet this standard. The potential interconnecting aquifers will be identified by proof drilling. The proof drilling will provide information on groundwater so the design and construction of piles can ensure the aquifer is sealed off. This minimises the potential for contamination of the aquifer or leakage.

The contamination of groundwater from other activities is addressed in the CEMP, particularly Appendix E- the Hydro Carbon Spill Plan.

2.6 Response Management

Where an Alert trigger level is exceeded, the Construction Manager, Environmental Manager, Design Manager and Geotechnical Design Lead will be notified by the Environmental Management Team within 3 working days with details of actions to be undertaken. Actions are outlined below:

- Repeat monitoring of piezometer exceeded and closest piezometers in the vicinity;
- Increase frequency of groundwater level monitoring to daily for all piezometers and groundwater wells within 50 m radius of the affected monitoring piezometer.

Where an Action trigger level for a monitoring bore is exceeded, activities that have the potential to cause adverse effects will be ceased or mitigated. The following people will be notified:

- Construction Manager
- Environmental Manager
- Design Manager
- Geotechnical Design Lead
- Engineer to Contract

In addition GWRC will be notified of the exceedance and the proposed mitigation will be discussed with them.

Works may recommence without mitigation once groundwater levels return to sub-action levels. Alternatively, works may recommence if written notice is received from GWRC.

If groundwater contamination is encountered, GWRC will be notified as soon as practical. An assessment will be undertaken to determine if the contamination is project-related, and if so, the source of contamination will be isolated. If the contamination is project-related, FCC will report to GWRC on the cause of contamination, the impact, measures taken to stop the contamination and the clean-up undertaken. An interim report will be prepared within 10 days of the contamination being reported.

If a change in quantity or quality affecting a registered drinking water supply is identified the registered drinking water supplier (see section 2.3.1) will be contacted as soon as practicable.

2.6.1 Ground water levels- existing users

If construction activities result in significant change in monitored ground water levels and yield, the cause and impact of these changes will be assessed. If attributable to the Project the following mitigation measures will be instigated;

- Providing temporary storage tanks and delivery water using trucks. A replacement drinking water supply will be provided within 2 days of a water user being unable to use their supply.
- Lower pump levels in existing wells (if feasible).
- Increase depth of current users' bores.
- Provide new bores to current users if the existing bore is rendered inoperable.

2.7 Complaints

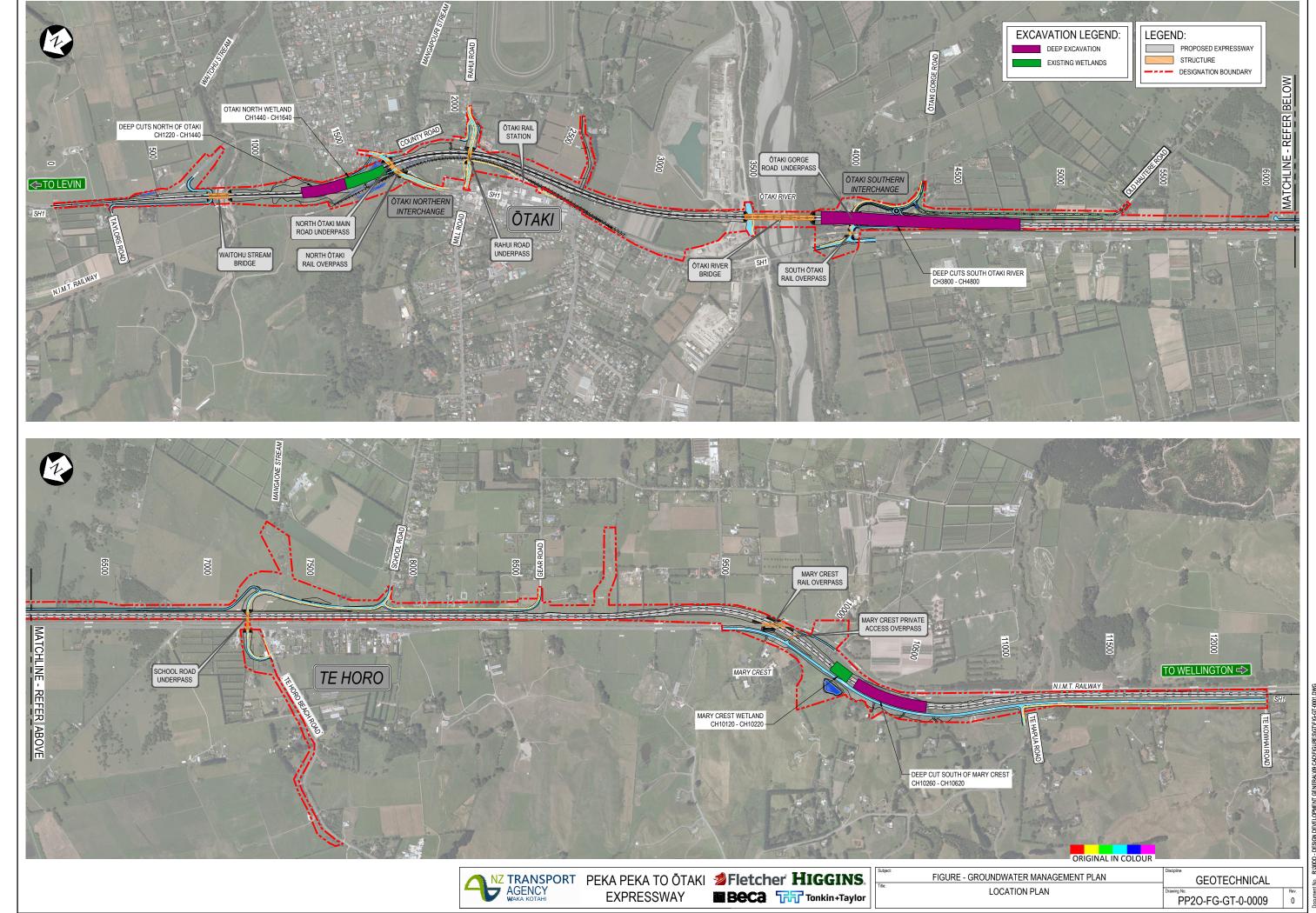
Complaints relating to groundwater will be treated as part of the CEMP complaints process. The process allows;

- A complaints register is maintained by the Stakeholder & Communications Manager and updated regularly as new complaints are received.
- A summary of complaints will be provided to KCDC/GWRC on a monthly basis. The complaints register will be made available at all times to KCDC/GWRC upon request.
- Complaints can be received by email to pp2o@nzta.govt.nz, by calls to the 0800 line or recorded messages if out of manned hours for the phone, in person to Project staff, or in writing.

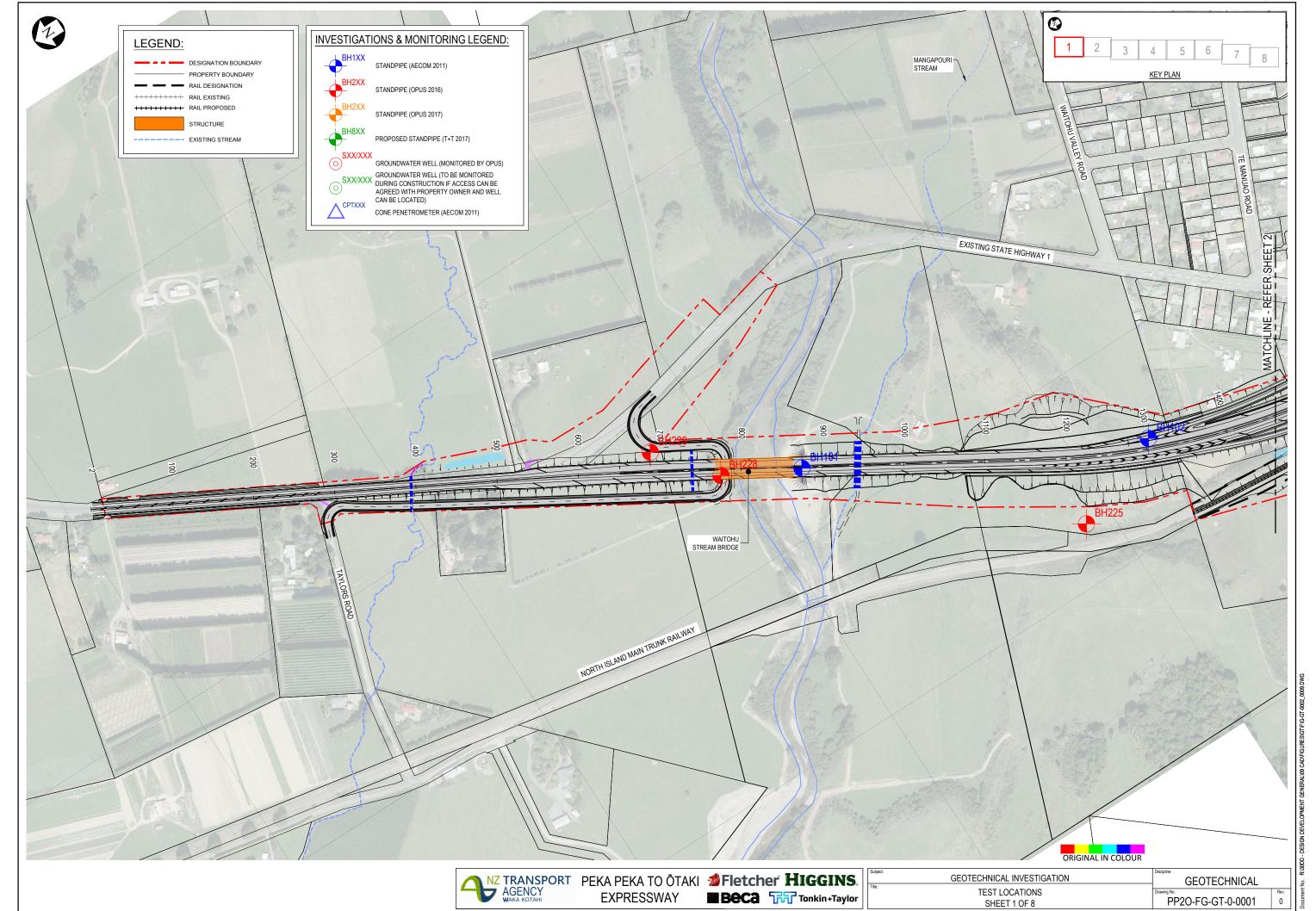
2.8 Review Procedures

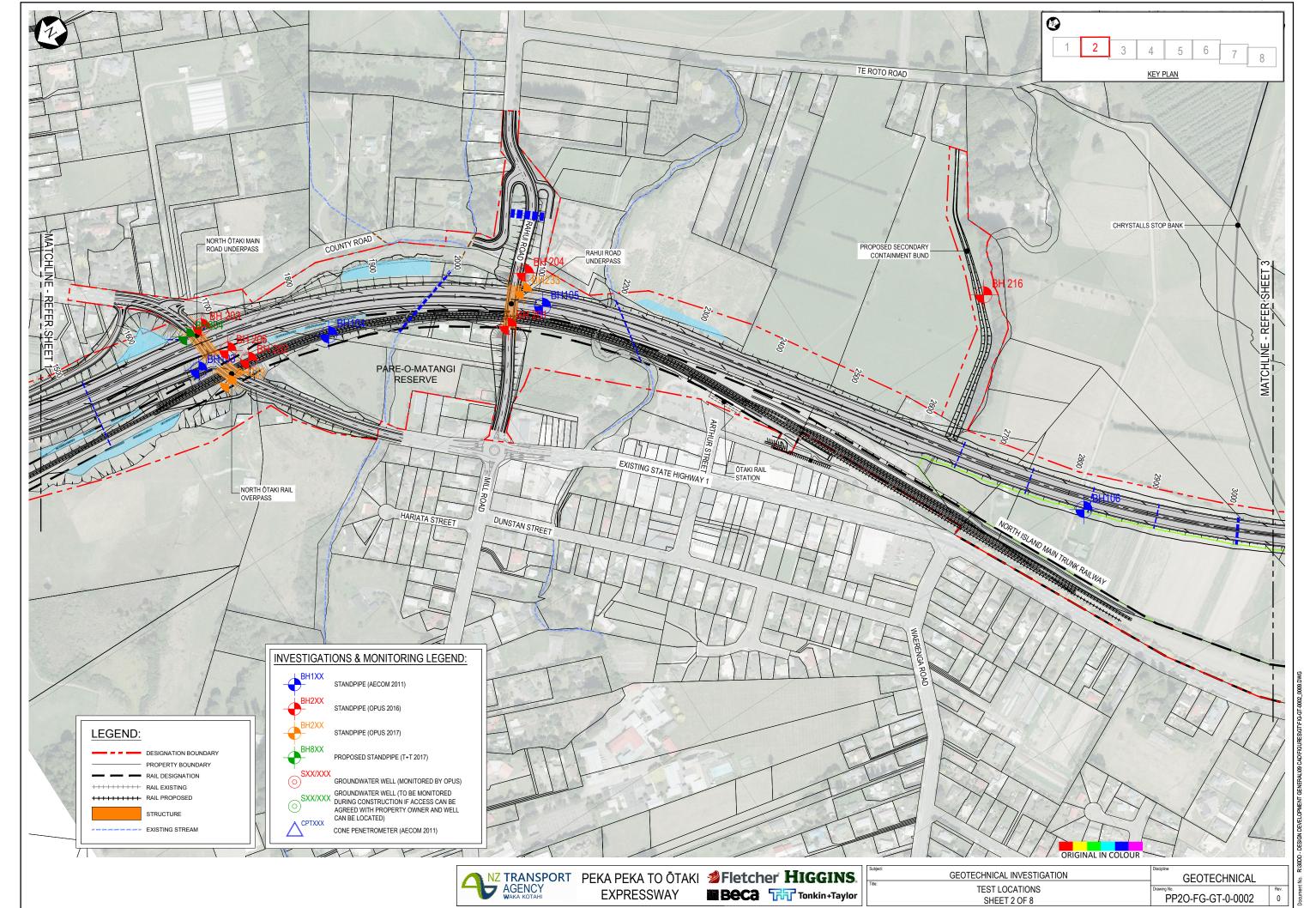
On an annual basis (May each year) a review of the GMP would be undertaken to ensure that the monitoring captures any potential adverse effects on groundwater and that mitigations methods are effective. If the review results in modification of the GMP, it will be submitted to GWRC for certification.

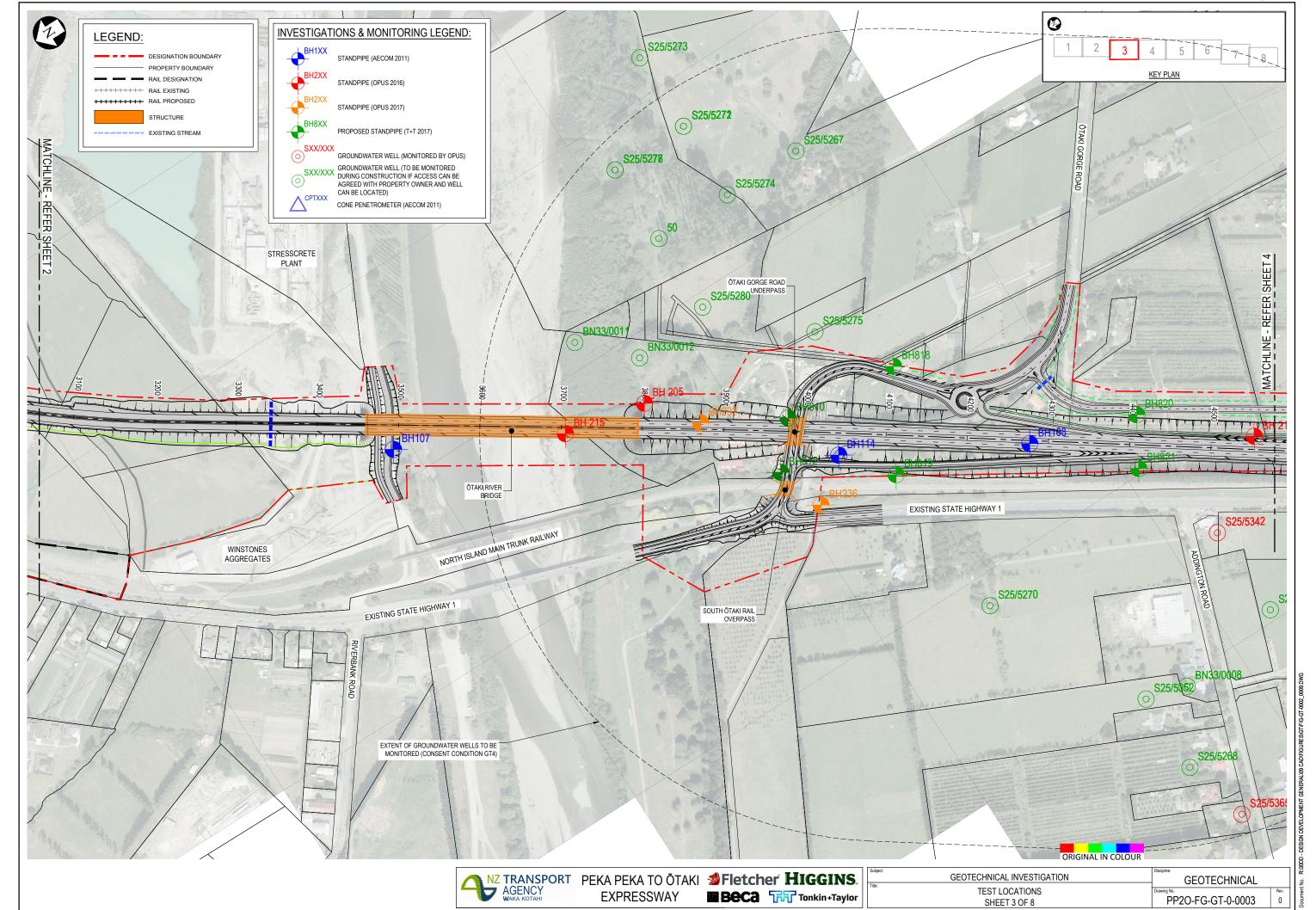
APPENDIX A - LOCATIONS WHERE GROUNDWATER MAY BE AFFECTED BY THE CONSTRUCTION WORKS

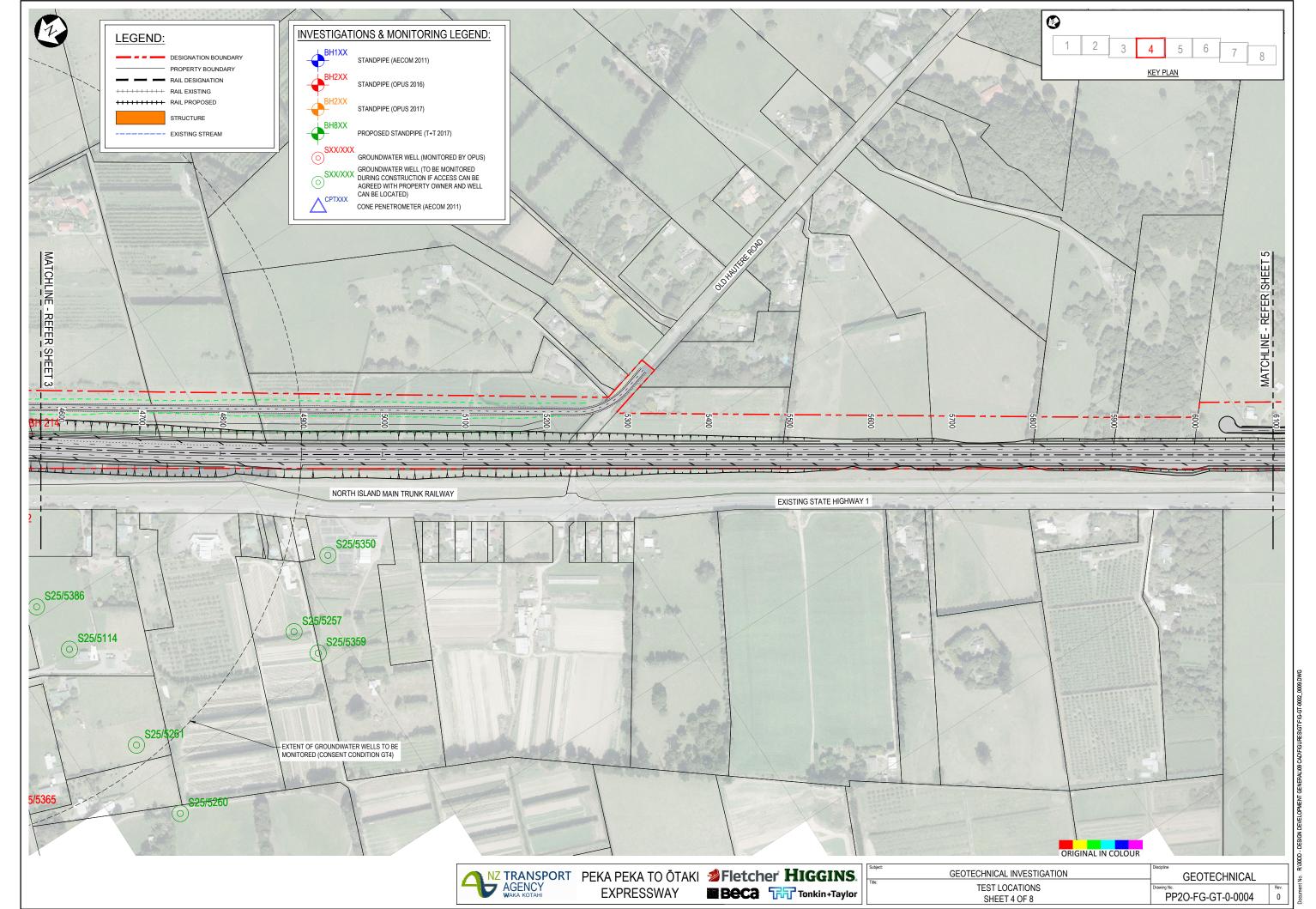


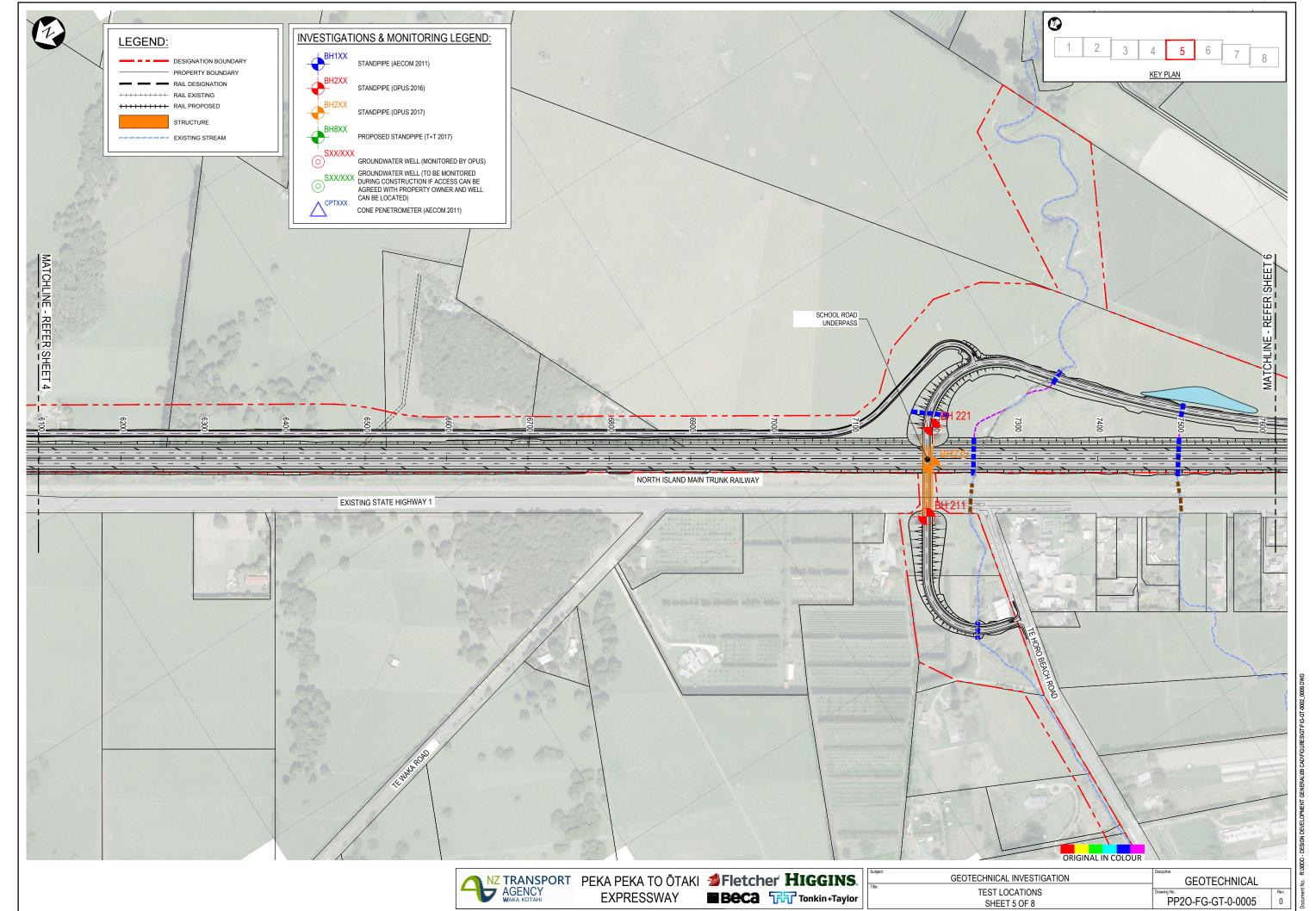
APPENDIX B - LOCATION OF STANDPIPES AND GROUNDWATER WELLS

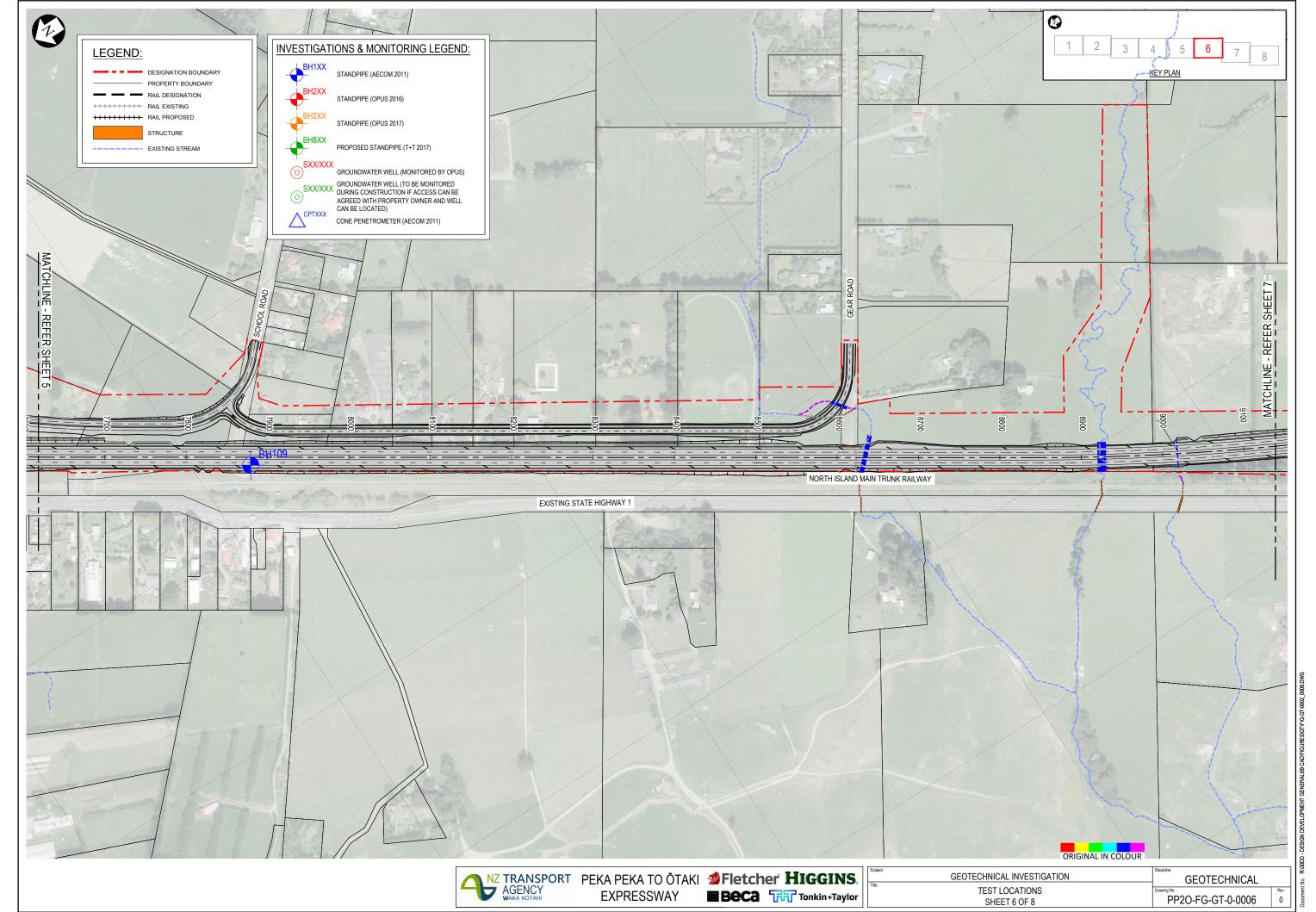


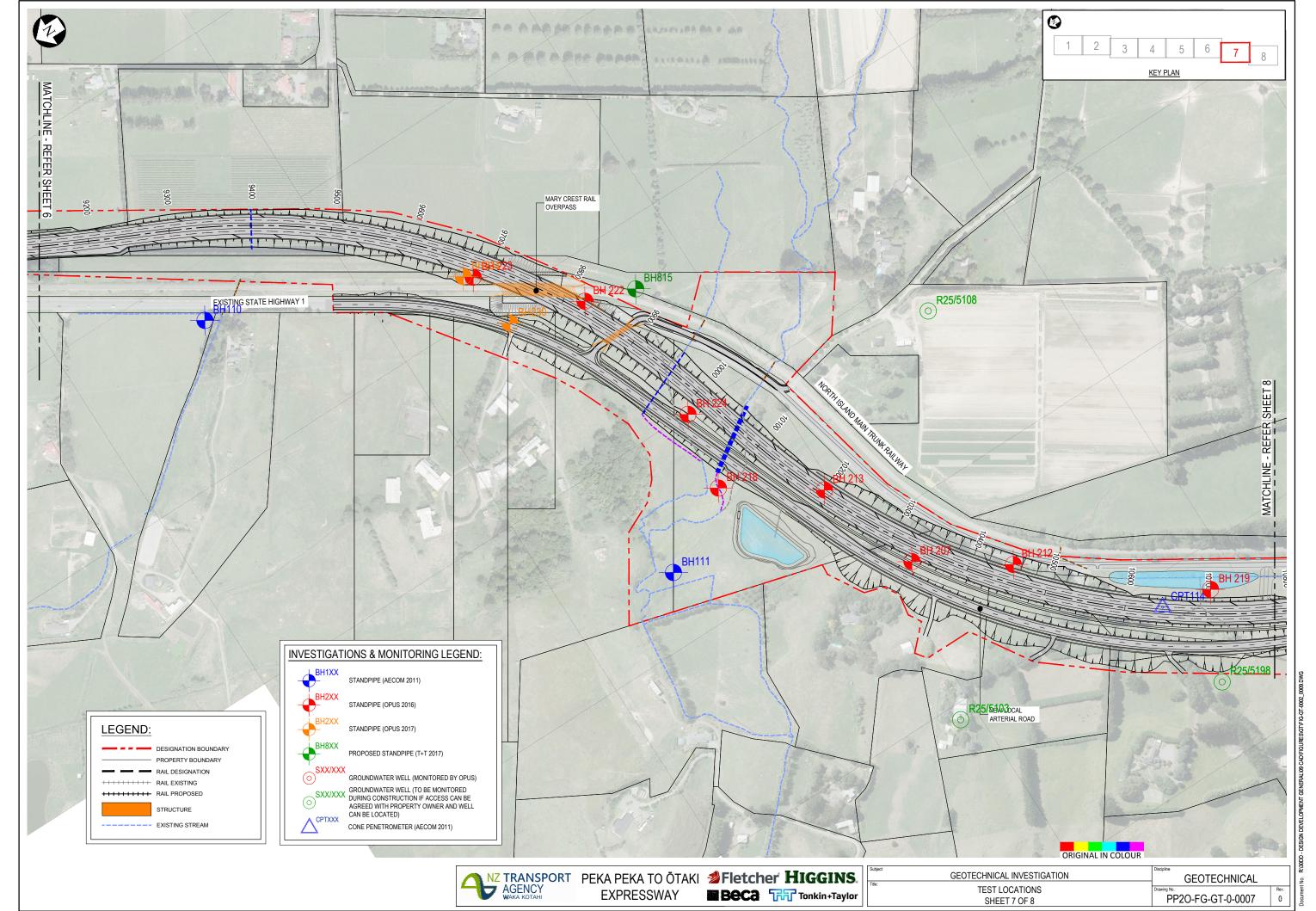


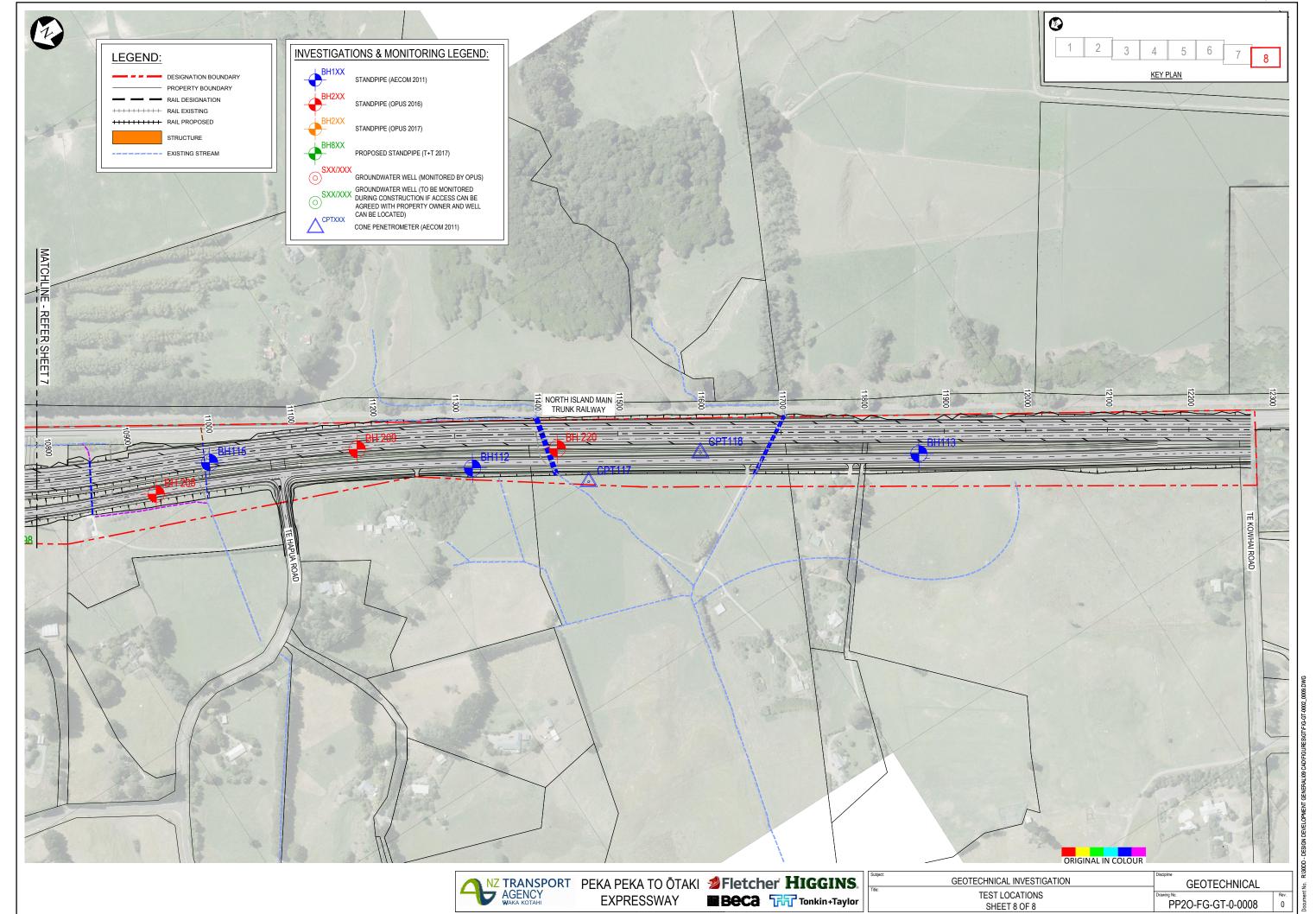












APPENDIX C - SCHEDULE OF GROUNDWATER MONITORING BORES

| Location | | | | | | | | Response Zone | 2 | | | | |
|--------------------|------------------------|------------------------------|---|---------------|------------------|---------------|------------------|-------------------|---|---|--|--|--|
| Borehole ID | Easting (NZTM) | sting (NZTM) Northing (NZTM) | | Top of Screen | Bottom of Screen | • | Bottom of Screen | Screen length (m) | Geological Unit | Comment | | | |
| BH101 | 1782715.3 | 5486569.7 | 24.6 | (mbgl) 0.5 | (mbgl) 15 | (mRL) 24.1 | (mRL) 9.6 | 14.5 | Recent alluvium (Q1a) and quaternary beach alluvium (Q5b) | + | | | |
| BH102_A | | | 25.4 | 0.5 | 2.4 | 24.9 | 23.0 | 1.9 | Quaternary beach alluvium (Q5b) | | | | |
| BH102_B | 1782516.8 | 5486191.3 | 25.4 | 8.0 | 18.5 | 17.4 | 6.9 | 10.5 | Quaternary beach alluvium (Q5b) | \Box | | | |
| BH103_A | 1782397.3 | 5485851.0 | 17.4 | 0.5 | 5.0 | 16.9 | 12.4 | 4.5 | Inter-dune deposit (Q1sd) and quaternary beach alluvium (Q5b) | 4 | | | |
| BH103_B | | | 17.4 | 6.0 | 17.0 | 11.4 | 0.4 | 11.0 | Quaternary beach alluvium (Q5b) | | | | |
| BH104 BH105 | 1782347.7 1782235.1 | 5485692.6 5485451.6 | 12.2 13.1 | 1.5 1.0 | 17.0 10.0 | 10.7 12.1 | -4.8 3.1 | 15.5 9.0 | Quaternary terrace alluvium (Q2a) and quaternary beach alluvium (Q5b) Quaternary terrace alluvium (Q2a) and quaternary beach alluvium (Q5b) | _ | | | |
| BH106 | 1781664.3 | | 5485023.8 14.2 2.0 12.5 12.2 1.7 10.5 Quaternary terrace alluvium (Q2a) and quaternary beach alluvium (Q5b) 5484500.7 14.8 7.0 10.0 7.8 4.8 3.0 Quaternary beach alluvium (Q5b) | | | | | | | | | | |
| BH107 | 1781240.4 | | | | | | | | | | | | |
| BH108 | 1780824.7 | 5483837.3 | 22.8 | 2.0 | 19.5 | 20.8 | 3.3 | | Quaternary terrace alluvium (Q2a) and quaternary beach alluvium (Q5b) | | | | |
| BH109 | 1778861.0 | 5480817.5 | 18.8 | 2.0 | 20.0 | 16.8 | -1.2 | | Quaternary terrace alluvium (Q2a) and quaternary beach alluvium (Q5b) | AECOM Standpipes. Monitored May 2012 to | | | |
| BH110_A BH110 B | 1778015.9 | 5479621.2 | 15.5 15.5 | 7.0 15.0 | 9.0 17.0 | 8.5 0.5 | 6.5 -1.5 | | Quaternary beach alluvium (Q5b) Quaternary beach alluvium (Q5b) | ——April 2013 | | | |
| BH110_B BH111 A | | | 15.2 | 5.5 | 11 | 9.7 | 4.2 | <u> </u> | Quaternary beach alluvium (Q5b) | | | | |
| BH111_B | 1777444.2 | 5479302.5 | 15.2 | 18.5 | 25 | -3.3 | -9.8 | | Quaternary beach alluvium (Q5b) | | | | |
| BH112_A | 1776731.0 | 5478246.9 | 13.0 | 1.5 | 3.5 | 11.5 | 9.5 | 2.0 | Inter-dune deposit (Q1sd) | | | | |
| BH112_B | | | 13.0 | 7.8 | 9.8 | 5.2 | 3.2 | 2.0 | Dune sand (Q1s) | | | | |
| BH113 | 1776451.0 | 5477776.4 | 13.3 | 1.5 | 14 | 11.8 | -0.7 | | Dune sand (Q1s) and quaternary beach alluvium (Q5b) | | | | |
| BH114 BH115 | 1780939.4 1776910.2 | 5484043.0 5478514.6 | 22.7 12.3 | <u>3</u> | 20 20 | 19.7 7.3 | 2.7 -7.7 | 17.0 15.0 | Quaternary terrace alluvium (Q2a) and quaternary beach alluvium (Q5b) Dune sand (Q1s) and quaternary beach alluvium (Q5b) | \dashv | | | |
| CPT114 (CN) | 1777084.0 | 5478818.4 | 12.3 | 0 | 3 | 12.3 | 9.3 | 3.0 | Inter-dune deposit (Q1sd) and dune sand (Q1s) | 7 | | | |
| CPT117 (CQ) | 1776640.1 | 5478135.8 | 12.4 | 0 | 3 | 12.4 | 9.4 | 3.0 | Inter-dune deposit (Q1sd) | | | | |
| CPT118 (CR) | 1776596.2 | 5478001.6 | 14.4 | 0 | 3 | 14.4 | 11.4 | 3.0 | Inter-dune deposit (Q1sd) | | | | |
| BH 201 | 1782237.3 | 5485500.4 | 13.7 | 4.5 | 12.5 | 9.2 | 1.2 | 8.0 | Quaternary beach alluvium (Q5b) | | | | |
| BH 202 | 1782374.2 | 5485792.9 | 22.6 | 8.0 | 13.0 | 14.6 | 9.6 | | Quaternary beach alluvium (Q5b) | | | | |
| BH 203 BH 204 | 1782440.3 1782281.1 | 5485820.0 5485447.6 | 25.1 14.6 | 8.0 4.0 | 13.0 10.0 | 17.1 10.6 | 12.1 4.6 | | Quaternary beach alluvium (Q5b) Quaternary beach alluvium (Q5b) | <u> </u> | | | |
| BH 205 | 1782281.1 | 5484210.1 | 11.5 | 3.0 | 9.5 | 8.5 | 2.0 | | Recent alluvium (Q1a) | | | | |
| BH 206 | 1782398.1 | 5485807.9 | 17.5 | 7.5 | 11.0 | 10.0 | 6.5 | | Quaternary beach alluvium (Q5b) | | | | |
| BH 207 | 1777297.6 | 5479048.1 | 29.7 | 8.0 | 12.0 | 21.7 | 17.7 | 4.0 | Dune sand (Q1s) and quaternary beach alluvium (Q5b) | | | | |
| BH 208 | 1776912.8 | 5478590.6 | 11.7 | 3.5 | 6.5 | 8.2 | 5.2 | | Dune sand (Q1s) | | | | |
| BH 209 | 1776826.5 | 5478353.6 | 17.8 | 3.0 | 6.0 | 14.8 | 11.8 | 3.0 | Dune sand (Q1s) | | | | |
| BH 211 BH 212 | 1779180.9 1777227.2 | 5481431.2 5478944.8 | 16.7 22.3 | 5.0 5.0 | 9.0 9.0 | 11.7 17.3 | 7.7 13.3 | 4.0 4.0 | Quaternary terrace alluvium (Q2a) and quaternary beach alluvium (Q5b) Inter-dune deposit (Q1sd) and dune sand (Q1s) | <u> </u> | | | |
| BH 213 | 1777429.6 | 5479091.3 | 22.1 | 2.0 | 6.0 | 20.1 | 16.1 | 4.0 | Quaternary beach alluvium (Q5b) | | | | |
| BH 214 | 1780683.5 | 5483599.8 | 23.1 | 5.0 | 10.0 | 18.1 | 13.1 | . | Quaternary terrace alluvium (Q2a) and quaternary beach alluvium (Q5b) | | | | |
| BH 215 | 1781142.8 | 5484312.1 | 10.3 | 1.0 | 5.0 | 9.3 | 5.3 | 4.0 | Recent alluvium (Q1a) | | | | |
| BH 216 | 1781953.8 | 5484985.4 | 14.6 | 4.0 | 8.0 | 10.6 | 6.6 | | Quaternary beach alluvium (Q5b) | | | | |
| BH 218 | 1777501.9 | 5479200.0 | 18.4 | 1.0 | 4.0 | 17.4 | 14.4 | | Dune sand (Q1s) and quaternary beach alluvium (Q5b) | OPUS Standpipes. Monitored December 2014 | | | |
| BH 219 BH 220 | 1777070.7 1776695.0 | 5478757.0 5478146.4 | 11.9 12.9 | 1.0 0.5 | 4.0 5.0 | 10.9 12.4 | 7.9 7.9 | 3.0 4.5 | Inter-dune deposit (Q1sd) and dune sand (Q1s) Inter-dune deposit (Q1sd) | to December 2016 | | | |
| BH 221 | 1779269.7 | 5481366.4 | 18.0 | 4.0 | 9.0 | 14.0 | 9.0 | | Quaternary terrace alluvium (Q2a) and quaternary beach alluvium (Q5b) | | | | |
| BH 222 | 1777783.9 | 5479214.5 | 23.4 | 1.0 | 6.0 | 22.4 | 17.4 | 5.0 | Dune sand (Q1s) and quaternary beach alluvium (Q5b) | | | | |
| BH 223_A | 1777882.8 | 5479314.5 | 23.1 | 2.0 | 4.0 | 21.1 | 19.1 | 2.0 | Dune sand (Q1s) and quaternary beach alluvium (Q5b) | | | | |
| BH 223_B | | | 23.1 | 9.0 | 11.0 | 14.1 | 12.1 | | Quaternary beach alluvium (Q5b) | | | | |
| BH 224 | 1777598.4 | 5479182.6 | 19.9 | 0.5 | 5.0 | 19.4 | 14.9 | | Quaternary beach alluvium (Q5b) | _ | | | |
| BH225 BH226 | 1782469.5 1782832.3 | 5486312.0 5486716.1 | 28.9 25.6 | 10.0 8.0 | 14.0 12.0 | 18.9 17.6 | 14.9 13.6 | | Quaternary beach alluvium (Q5b) Quaternary beach alluvium (Q5b) | \dashv | | | |
| BH228 | 1782761.4 | 5486658.4 | 25.2 | 9.0 | 13.0 | 16.2 | 12.2 | | Quaternary beach alluvium (Q5b) | \dashv | | | |
| BH229 | 1782363.5 | 5485829.1 | 26.9 | 9.5 | 14.5 | 17.4 | 12.4 | . | Dune sand (Q1s) and quaternary beach alluvium (Q5b) | | | | |
| BH230 | 1777810.7 | 5479306.9 | 22.3 | 9.5 | 15.5 | 12.8 | 6.8 | 6.0 | Quaternary beach alluvium (Q5b) | | | | |
| BH232 | 1781064.4 | 5484164.7 | 12.1 | 1.0 | 6.0 | 11.1 | 6.1 | | Recent alluvium (Q1a) | _ | | | |
| BH233 | 1782263.4 | 5485461.4 | 14.3 | 5.5 | 8.5 | 8.8 | 5.8 | | Quaternary beach alluvium (Q5b) | | | | |
| BH234 BH235 | 1777890.0 1779230.4 | 5479324.3 5481392.2 | 23.3 17.8 | 2.0 3.5 | 6.0 8.5 | 21.3 14.3 | 17.3 9.3 | 4.0 5.0 | Dune sand (Q1s) and quaternary beach alluvium (Q5b) Quaternary terrace alluvium (Q2a) and quaternary beach alluvium (Q5b) | \dashv | | | |
| BH236 | 1780899.5 | 5484094.4 | 22.2 | 5.0 | 9.0 | 17.2 | 13.2 | | Quaternary terrace alluvium (Q2a) and quaternary beach alluvium (Q5b) | | | | |
| BH810 | 1781011.5 | 5484070.8 | | | 1 | * | | TBD | , | | | | |
| BH812 | 1780960.0 | 5484114.4 | | TBD | | | | | | | | | |
| BH815 | 1777763.1 | 5479153.7 | | TBD | | | | | | | | | |
| BH818 | 1780995.04 | 5483927.338 | TBD | | | | | | | | | | |
| BH819 | 1780881.087 | 5483996.466 | TBD TBD | | | | | | | | | | |
| BH820 | 1780783.711 | 5483707.253 | | | | | | | | Proposed Groundwater Monitoring Standpipe | | | |
| BH821 | 1780783.711 | 5483740.733 | TBD | | | | | | \dashv | | | | |
| S25/5342 | 1780608.0 | 5483740.733 | | | | | | | Existing Ground Water Wells (Monitored by | | | | |
| S25/5365 | 1780300.0 | 5483863.0 | 22.6 Unknown | | | | | | Opus) | | | | |
| 323/3303 | 1/00300.0 | J403003.U | 0 22.0 UIKIIOWII Opus) | | | | | | | | | | |

| | | Location | | | | | | | |
|-------------|----------------|----------------------------------|-------------------------|-------------------------|------------------------|---------------------------|-------------------|-----------------|--|
| Borehole ID | Easting (NZTM) | Northing (NZTM) BH RL (NZVD2009) | Top of Screen (mbgl) | Bottom of Screen (mbgl) | Top of Screen (mRL) | Bottom of Screen (mRL) | Screen length (m) | Geological Unit | Comment |
| 50 | 1781283 | 5484086 | | | | | TBD | | |
| BN33/0008 | 1780469 | 5483835 | | | | | TBD | | |
| BN33/0011 | 1781231 | 5484242 | | | | | TBD | | |
| BN33/0012 | 1781172 | 5484185 | | | | | TBD | | |
| S25/5114 | 1780427 | 5483692 | | | | | TBD | | |
| S25/5252 | 1781218 | 5481785 | | | | | TBD | | |
| S25/5257 | 1780296 | 5483447 | | | | | TBD | | |
| S25/5260 | 1780183 | 5483686 | | | | | TBD | | |
| S25/5261 | 1780283 | 5483686 | | | | | TBD | | |
| S25/5265 | 1780183 | 5483886 | | | | | TBD | | |
| S25/5267 | 1781283 | 5483886 | | | | | TBD | | |
| S25/5268 | 1780383 | 5483886 | | | | | TBD | | |
| S25/5270 | 1780683 | 5483986 | | | | | TBD | | 1 |
| S25/5271 | 1781383 | 5483986 | | | | | TBD | | Existing Groundwater Wells (within 500m from |
| S25/5272 | 1781383 | 5483986 | | | | | TBD | | chainages 4100 to 4400). Location of wells and access to be confirmed prior to construction. |
| S25/5273 | 1781483 | 5483986 | | | | | TBD | | access to be committed prior to construction. |
| S25/5274 | 1781283 | 5483986 | | | | | TBD | | |
| S25/5275 | 1781083 | 5483986 | | | | | TBD | | |
| S25/5277 | 1781383 | 5484086 | | | | | TBD | | |
| S25/5278 | 1781383 | 5484086 | | | | | TBD | | |
| S25/5280 | 1781183 | 5484086 | | | | | TBD | | |
| S25/5350 | 1780353 | 5483361 | | | | | TBD | | |
| S25/5352 | 1780483 | 5483886 | | | | | TBD | | |
| S25/5359 | 1780258 | 5483436 | | | | | TBD | | |
| S25/5386 | 1780493 | 5483698 | | | | | TBD | | |
| R25/5103 | 1777101 | 5479102 | | | | | TBD | | |
| R25/5108 | 1777546 | 5478865 | | | | | TBD | | |
| R25/5198 | 1776967 | 5478806 | | | | | TBD | | |

APPENDIX D - EXISTING GROUNDWATER USERS

| ABEL | ID | APPELLATION | OWNER | Address | Phone Number | TITLE | GWRC Database Bores | Known Bores, Wells and Water Take Consents |
|------|---------|------------------------------------|--|--|----------------------------------|------------|----------------------------------|---|
| 1 | 3860923 | Lot 2,DP 18929,0.8534 | Philip James McPhail Fiona Michele McPhail | 39 Addington Road, Otaki | | WN880/12 | | No |
| 2 | 3782481 | Lot 4,DP 18929,1.6137 | Alan Keith Colman Mary Patricia Colman | 23 Addington Road, Otaki | 06 364 7676 | WN722/66 | S25/5261 | Yes |
| 3 | 3751937 | Lot 3,DP 18929,0.8093 | Public Trust & Elmswood Trustee Limited (Don Stuart Charlett & Myrna Kathleen Read) | 35 Addington Road, Otaki | 06 364 6467 | WN863/47 | | No |
| 4 | 3802702 | Lot 1,DP 17292,0.2023 | Jennifer Kaye Haugh | 40 Addington Road, Otaki | 06 364 6080 | WN609/296 | | No |
| 5 | 3814270 | Lot 1,DP 68010,0.3550 | Jason Robert John Kemp | 36 Addington Road, Otaki | | WN38A/922 | | No |
| 6 | 7241877 | Lot 2,DP 429805,2.2119 | Stephen William Forsyth Joanne Louise Forsyth | | | 516277 | S25/5114 S25/5386 | Yes |
| 7 | 3807401 | Lot 1,DP 20824,0.1012 | Rodney Robert Clifton Joy Ruth Clifton, Cullinane Steele Trustees 2003 Limited | 1189 State Highway 1, Otaki- Waikanae | 06 364 3105 | WN902/43 | | No |
| 8 | 3871250 | Lot 1,DP 22239,0.1115 | Penelope Vicky Bertelsen | | | WN914/27 | | No |
| 9 | 3796173 | Lot 1,DP 81665,0.0814 | Penelope Vicky Bertelsen | | | WN48B/143 | | No |
| 11 | 3941763 | Lot 2,DP 81665,0.0027 | F R P V Bertelsen Limited | | | WN48B/144 | | No |
| 12 | 3890445 | Pt Lot 2,DP 30709,0.1369 | Brent Victor Bertelsen Jane Lesley Bertelsen | | | WN48B/145 | | No |
| 31 | 3985175 | Pt Lot 1,DP 55964,6.2626 | F R P V Bertelsen Limited | | | WN48B/144 | S25/5359 S25/5257 | Yes |
| 34 | 3982198 | Lot 2,DP 65764,4.1697 | Jane Lesley Bertelsen Robin James Barrie Roger Gerard Downey | | | WN34B/900 | | No |
| 10 | 3803215 | Lot 1,DP 65764,0.5577 | Kelvin John Smith | 1209 State Highway 1, Otaki- Waikanae | 06 364 0999 | WN34B/899 | | No |
| 13 | 3833031 | Lot 1,DP 72050,0.3925 | Lesley Dawn Mann Albert Henry Mann | 26 Addington Road, Otaki | 06 364 8528 | WN40A/21 | | No |
| 14 | 7241876 | Lot 1,DP 429805,0.7093 | Colin Stanley Fairhurst Kevin Paul Forgeson | 1217 State Highway 1, Otaki- Waikanae | 06 364 8976 | 516276 | | No |
| 15 | 3830194 | Lot 5,DP 18929,0.8169 | David Nolan Randall and Jill Marie Frances Randall, and Bruce Curran | | | WN19A/1225 | | No |
| 16 | 3867069 | Lot 4,DP 77471,1.5030 | Kennott Trust Company Limited (D & J Pritchard, T Nowland) (Land Matters) | 20 Addington Road, Otaki | | WN43D/81 | BN33/0008 | Yes |
| 17 | 7241878 | Lot 3,DP 429805,0.1294 | Michael Scott Hyland Queenie Rawinia Rikihana Graham John Winterburn Lenore Jane Winterburn | -3 Addington Road. Otaki | 11 Rangiuru Rd, Otaki | 524962 | S25/5342 | Yes |
| 18 | 3821072 | Lot 7,DP 18929,0.1280 | Michael Scott Hyland Queenie Rawinia Rikihana Graham John Winterburn Lenore Jane Winterburn | | 06 364 8676 | 524962 | | No |
| 19 | 3948905 | Lot 3,DP 77471,1.3260 | Royden Stuart Mayfield Susan Claire Mayfield | Toad Hall, 4 Addington Road, Otaki | 06 364 8900 | WN43D/80 | | No |
| 20 | 3934410 | Section 84,Blk X Waitohu SD,0.2527 | Railway Purposes NZGZ 1929 p 2761 | | | | | No |
| 21 | 3894305 | Lot 1,DP 77010,0.5655 | Patricia Kathleen Chapman Kevin William Chapman | 40 Otaki Gorge Road, Akatarawa- Otaki | 06 364 5470 (Chapman KW & TK) | WN43D/359 | | No |
| 22 | 3829246 | Lot 1,DP 51817,0.2444 | Michael Douglas Nutting Barry Norris Nutting James Clifford Simpson | 32 Otaki Gorge Road, Akatarawa- Otaki | 06 364 7342 (ME Nutting) | WN21C/421 | | No |
| 23 | 6915862 | Lot 1,DP 368337,0.7968 | Her Majesty the Queen (Ex- Guthrie) | 19 Otaki Gorge Road | | 277591 | | No |
| 24 | | Lot 1,DP 75822,0.6685 | Her Majesty the Queen (Ex- Angus & Hall) | 15 Otaki Gorge Road Akatarawa | | WN43A/409 | | No |
| 25 | | Lot 1,DP 86069,0.0495 | Victor John Walker Vivienne Walker | 66 Otaki Gorge Road, Akatarawa- | 06 364 5786 | WN53D/378 | | No No |
| 26 | 3822259 | Lot 1,DP 77899,0.3860 | Victor John Walker Vivienne Walker | Otaki | | WN53D/378 | | No |
| 27 | | Lot 4,DP 75822,0.4735 | Graeme Bruce McGregor Gillian Mary Hay | 65 Otaki Gorge Road, Akatarawa- Otaki | 06 364 8208 | WN43A/412 | | No |
| 28 | | Lot 5,DP 75822,0.1123 | The Kapiti Coast District Council ,Local Purpose Reserve (Esplanade) | 1 | | WN43A/413 | | No |
| 29 | | Lot 6,DP 75822,0.1742 | Crown Land | | | | | No |
| 32 | | Lot 3,DP 454823,18.4230 | Stephen John Rowland, Sharleen Workman, Advisory Trustees 011 Limited | 62 Addington Road, Otaki | | 584410 | S25/5128 | Yes |
| 33 | 4021960 | Lot 2,DP 55964,5.6174 | Wendy Anne Devenport | 45 Addington Road, Otaki | | WN25D/986 | S25/5260 | Yes |
| 35 | 7399196 | Lot 2,DP 454823,4.4095 | Richard Hudson Caughley Sarah Elizabeth Playne Caughley Richard Hudson Caughley Sarah Elizbeth Playne Caughley | | | 584409 | S25/5289 S25/5288 S25/5290 | Yes |
| 36 | 3899979 | Lot 1,DP 34974,4.0468 | Neil Jackson Baldwin Betty Dorothy Baldwin | 42 Addington Road, Otaki | 06 364 5769 | WN11C/430 | 323,3230 | No |
| 37 | | | Balance Parcel NZGZ 2009 p 1935 (RAIL) | / | | 1 | + | No |

| ABEL | ID |) | APPELLATION | OWNER | Address | Phone Number | TITLE | GWRC Database Bores | Known Bores, Wells and Water Take Consents |
|------|----|---------|-----------------------------|---|--|--|-----------------|--|--|
| | | | | | | | | | |
| | 38 | 3758084 | Lot 1,DP 27257,4.8562 | Her Majesty the Queen (Ex- Bertelsen) | 9 Old Hautere Road | | WNE2/728 | 42 | Yes |
| | 39 | 3752756 | Lot 2,DP 72050,4.5410 | Remus Mihaila Lacramioara Daniela Mihaila | 30 Addington Road, Otaki | | WN40A/22 | \$25/5268 | Yes |
| | 40 | 3975083 | Pt Lot 2,DP 27257,5.1975 | Sarah Lee Brown Mark Gilbert Maclean | 11 Old Hautere Road, Te Horo | | WN40D/618 | S25/5256 | Yes |
| | 41 | 3898990 | Lot 3,DP 27257,4.2492 | Christopher Morris Wendy Mary Morris | 19-21 Old Hautere Road, Te Horo | | WNE2/1437 | | No |
| | 42 | 3918240 | Pt Lot 3,DP 52450,5.0616 | John Peter Boyle Susan Patricia Boyle | Old Hautere Road, Te Horo | 224B Parkes Line Road, Upper Hutt 04 526 7385 | WN53C/592 | | No |
| | 43 | 4031601 | Lot 1,DP 77471,5.2850 | Simon Dirk Berend van den Berg Jacqueline van den Berg | 24 Addington Road, Otaki | 221 Manly Street, Paraparaumu Beach 04 904 6143 | WN43D/78 | S25/5352 | Yes |
| | 44 | 3894307 | Pt Lot 3,DP 56271,5.0580 | Gregory Wayne Elliott Julie Edna Elliott | | | WN43D/360 | | No |
| | 45 | 3951014 | Lot 2,DP 77471,8.0660 | J C Morrison Farm Limited (John M & Christine A Morrison) | 4 Addington Road, Otaki | 4/38a Roxburgh Street, Mt. Victoria, Wellington 6011 04 385 2559 | WN43D/79 | S25/5270 | Yes |
| | 46 | 3914077 | Lot 2,DP 56271,5.5975 | Diane Nise Henderson Stephen Peter Henderson Murray Ivan Deans | | | WN27C/10 | | No |
| | 47 | 3909578 | Lot 1,DP 56271,5.5310 | Her Majesty the Queen (Ex- Kilbirnie Animal Health Properties Limited) | 34 Otaki Gorge Road | | WN27C/9 | | No |
| | 48 | 3770570 | Lot 2,DP 56978,1.4975 | Her Majesty the Queen (Ex- Matthews) | 12-29 Otaki Gorge Road | | WN27B/146 | | No |
| | 49 | 6915863 | Lot 2,DP 368337,5.0740 | Gees Thoroughbreds Limited | | | 277592 | S25/5275 S25/5280 | Yes |
| | 50 | 4063553 | Lot 2,DP 52450,6.7450 | Greenolive Limited (Brian J B & Sadie J Lange) | 68 Otaki Gorge Road, Akatarawa- Otaki | 06 364 0664 | WN26A/387 | | No |
| | 51 | 3902719 | Pt Lot 1,DP 52450,5.8005 | Donald McIntosh McLeod | 44 Otaki Gorge Road, Akatarawa- Otaki | 06 364 5212 | WN53D/379 | | No |
| | 52 | 3992989 | Lot 3,DP 56978,5.1170 | Her Majesty the Queen (Ex- Lange) | 3 Otaki Gorge Road | | WN27B/147 | BN33/0012 BN33/0011 | Yes |
| | 53 | 3749070 | Pt Lot 1,DP 16503,51.0220 | Fletcher Concrete and Infrastructure Limited | | | WN1200/35,84260 | S25/5345 | No |
| | | 7162767 | | NZGZ 2012 p 2294 (RAIL) | | | 117117 | | No |
| | 55 | | Section 2,SO 417765,14.1129 | Wellington Regional Council ,Set apart as Local Purposes Reserve (soil conservation and river control) \ NZGZ 2012 p 2294 [Classification and Vesting of a Reserve. Local Purpose (Soil Conservation and River Control) Reserve and Vests in Wellington R | | | 596260 | | No |
| | 56 | 3955322 | Lot 3,DP 75822,5.1935 | Phillip Andrew Guthrie Tracy Jane Guthrie | | | WN43A/411 | | No |
| | 57 | 3760869 | Pt Lot 5,DP 59965,5.7669 | Martin Arthur Broad Heather Dorothy Lear Linda Jane Broad | 70 Otaki Gorge Road, Akatarawa- Otaki | AM & MJ Broad 06 364 5815 | WN42C/40 | S25/5411 | Yes |
| | 58 | 3987108 | Lot 7,DP 59965,6.0774 | Yurdakul Bagci Naciye Bagci Robert Anthony Elms | 86 Otaki Gorge Road, Akatarawa- Otaki | | WN29C/222 | | No |
| | 59 | 3993194 | Lot 2,DP 27891,7.4386 | Darren Paul Nicholas Leanne Nicholas | 69 Otaki Gorge Road, Akatarawa- Otaki | 06 364 6842 | WNF1/1231 | \$25/5271 \$25/5272 \$25/5267 | Yes |
| | 60 | 3895967 | Lot 1,DP 27891,5.3216 | Max Foster Nevill Green Lee Anne Green Christopher Elliot Ritchie | 73 Otaki Gorge Road, Akatarawa- Otaki | 06 364 5094 | WNF1/1232 | | No |
| | 61 | 3753433 | Pt Lot 2,DP 3527,9.5910 | Soil Conservation and River Control NZGZ 1961 p 1862 | | | | 50 \$25/5278 \$25/5277 \$25/5273 \$25/5274 | Yes |
| | 65 | 3932262 | | GWRC | + | | + | 323/32/4 | No |

TPG (The Property Group) Contact: Rano Lealuga, 027 633 6111

APPENDIX E - EXISTING GROUNDWATER MONITORING DATA

Notice to Contractor No 18 – Groundwater Monitoring Data - BH101 to BH113 & CPT114, CPT117 & CPT118 between May 2012 and April 2013.

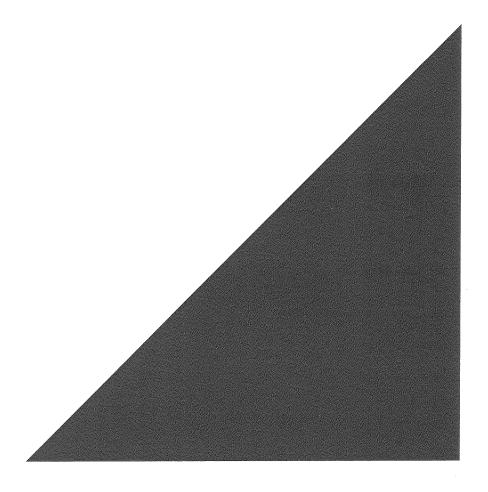
Peka Peka to Ōtaki Groundwater Monitoring Results December 2014 to December 2016, Opus International Consultants December 2016



Peka Peka to Otaki Expressway

Groundwater Monitoring Results

December 2014 to December 2016





Peka Peka to Otaki Expressway

Groundwater Monitoring Results

December 2014 to December 2016

Prepared By

Dolan Hewitt

Engineering Geologist

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New Zealand

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Date:

Reference:

December 2016

Report number: Status:

5C2771.00 GER 2016/29

Issue 1

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| | 6.2 Trial Pit Groundwater Observations | |
| | 6.3 Groundwater Fluctuations | 3 |
| | 6.4 Surface Water Movement | |

1 Introduction

The NZ Transport Agency (NZTA) commissioned Opus International Consultants (Opus) to prepare a specimen design, and manage construction, of the proposed Peka Peka to Otaki Expressway (PP2O) project. This included geotechnical investigations and monitoring of groundwater levels along the length of the project area. The project area extends 13 km from Taylors Road, north of the Otaki Township, to Te Kowhai Road in Peka Peka in the south. The site project area is illustrated on Figures 1 to 16.

Groundwater levels were monitored in piezometers which were installed within boreholes during the specimen design and tender stage geotechnical investigations. Groundwater levels were also monitored in pre-existing privately owned water wells.

This report presents the groundwater monitoring results for the period from December 2014 to December 2016.

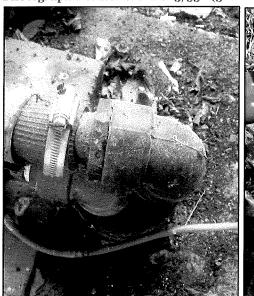
2 Piezometers

A total of 32 piezometers monitored groundwater conditions across the PP2O project area. The monitoring locations have been professionally surveyed. Their co-ordinates are presented in Table 1 and their positions are shown in Figures 1 to 16.

Piezometer construction consisted of 25 mm PVC pipe and filter fabric, surrounded by well graded sand and sealed with an upper and lower bentonite cap (Appendix A). The groundwater response zones are presented in Table 1.

3 Water Wells

Water wells were pre-existing private bores and their construction details and groundwater response zones are unknown. The area of influence of the construction works and the consented and recorded water wells in the area are shown in Figure 17. Only two of these water wells at the following locations were available for monitoring; S25/5342 (3 Addington Road) and S25/5365 (40 Addington Road). These water wells were monitored and their locations are shown in both Figures 6a and 17. Photographs of these bores are presented below.







4 Groundwater Monitoring

Groundwater levels were monitored during the specimen design stage and tender stage geotechnical investigations as investigation locations were completed, extending from December 2014 through to December 2016. Monitoring was undertaken using a flat tape water level dip meter with a probe tip sensor.

The monitoring during the investigations was carried out by Griffiths Drilling (NZ) Ltd. The frequency of monitoring has varied over time:

- Approximately trimonthly from February 2015 to July 2015;
- Approximately fortnightly from August 2015 to December 2015;
- Approximately monthly from January 2016 December 2016.

During the monitoring period, some of the piezometers were destroyed by livestock movements or obscured by flood deposits.

5 Groundwater Monitoring Results

The groundwater monitoring results are summarised in Table 1. The complete monitoring dataset is presented in Appendix B.

6 Groundwater Observations

6.1 Surface Ponding

Locations prone to experiencing periodic surface ponding were observed across the project area and were more common during the winter months. Areas where surface ponding was observed is presented in Table 2. Locations are presented north to south.

Piezometer levels and monitoring data identifies some of these surface ponding sites. However, as piezometer monitoring was not continuous (i.e. measurements fortnightly to trimonthly), and piezometers were not installed in all areas prone to surface ponding, some areas of potential surface ponding may not have been identified through piezometer measurements alone.

6.2 Trial Pit Groundwater Observations

Groundwater observations were recorded within trial pit excavations.

Groundwater flows into trial pits were noted to occur predominantly within the interdune deposits however, some trial pits excavated within terrace and recent alluvium, and also exhibited groundwater seepage. Trial pits excavations were open for approximately 2 hours.

Table 3 presents the locations where groundwater seepage or flow was observed within trial pit excavations. Locations are presented north to south.

6.3 Groundwater Fluctuations

Over the course of the groundwater monitoring period, fluctuations in groundwater levels were noted from ground observations and piezometer levels. As with surface ponding observations, fluctuations in groundwater levels were largely influenced by seasonal variations and rainfall events.

Table 1 presents the maximum and minimum recorded groundwater levels for each piezometer. Locations are presented north to south.

6.4 Surface Water Movement

During the site investigations, surface water movements were observed.

Surface water movement was most notable in the area of recent alluvial deposits situated around BH218, TP325 and TP345 (otherwise known as Marycrest property). The stream/river flowing adjacent to these site investigation locations would often flood over the banks flowing out across the adjacent river flats up to 30 m from the main channel. It was not uncommon for active alluvial deposits to have accumulated above the piezometer at BH218.

Such surface flooding events were common during winter months but were also noted to occur during severe rainfall events.

Tables

Opus International Consultants Ltd

Table 1 - Borehole locations and piezometer response zones

| | No. of Records | 38 | 39 | 39 | 39 | 38 | 39 | 39 | 38 | 38 | | 39 | 39 | 37 | 38 | 38 | 36 | | 36 | 36 | 27 | 29 | 28 | 27 | 25 | 28 |
|-------------------------------|---------------------|---------|---------|---------|---------|---------|---------|----------|---------|---------|-------------------------|---------|---------|---------|---------|---------|---------|-------------------------|---------|---------|---------|---------|---------|---------------------|-------|---------|
| | Z SZ | | | | | | | | | | | | | | | | | - | | | | | | | | |
| | Min. | 9:90 | 13.82 | 16.62 | 10.42 | 9.76 | 11.16 | 20.35 | 10.68 | 16.28 | talled | 10.40 | 19.24 | 17.55 | 15.15 | 9.47 | 9:68 | alled | 17.78 | 11.27 | 11.32 | 10.36 | 18.89 | 13.65 | 21.35 | 15.76 |
| Results | Max. (mRIL) | 11.94 | 14.78 | 18.62 | 12.44 | 11.10 | 15.19 | 27.99 | 12.18 | 17.56 | No piezometer installed | 11.71 | 21.35 | 20.34 | 18.36 | 10.42 | 11.71 | No piezometer installed | 18.88 | 12.37 | 12.87 | 12.48 | 22.96 | 22.44 | 22.44 | 20.36 |
| | Min. (mbgl) | 2.20 | 8.25 | 6.92 | 2.55 | 0.85 | 2.73 | 2.17 | 0.00 | 29.0 | No | 5.45 | 1.36 | 2.21 | 5.14 | 0.27 | 3.30 | No.] | 0.00 | 0.00 | 0.50 | 5.95 | 0.85 | 1.06 | 1.06 | 00.00 |
| | Max. (mbgl) | 4.24 | 9.21 | 8.92 | 4-57 | 2.19 | 6.76 | 9.81 | 1.50 | 1.95 | | 92.9 | 3.47 | 5.00 | 8.35 | 1.22 | 5.03 | | 1.10 | 1.10 | 1.10 | 8.07 | 4.92 | 9.85 | 2.15 | 4.60 |
| 9 | 76 | | | | | | | | | | | | | | | | | | | | | | | | | |
| Depth to Response Zone (mRL) | Bottom of Sereen | 1.64 | 10.03 | 12.54 | 4.99 | 2.454 | 6.92 | 18.16 | 5.68 | 12.23 | manager, p | 8.16 | 13.71 | 16.55 | 13.5 | 2.69 | 7.01 | | 14.88 | 8.365 | 8.37 | 9.425 | 17.81 | 19.50 | 12.50 | 15.36 |
| Depth to R | Top of Screen | 9.64 | 15.03 | 17.54 | 10.99 | 8.954 | 10.42 | 22.16 | 89.8 | 15.23 | er installed | 12.16 | 17.71 | 20.55 | 18.5 | 69.6 | 11.01 | er installed | 17.88 | 11.365 | 12.87 | 14.425 | 22.81 | 21.50 | 14.50 | 19.86 |
| Depth to Response Zone (mbgl) | Bottom of Sereen | 12.50 | 13.00 | 13.00 | 10.00 | 9.50 | 11.00 | 12.00 | 6.50 | 6.00 | No piezometer installed | 9.00 | 9.00 | 6.00 | 10.00 | 5.00 | 8.00 | No piezometer installed | 4.00 | 4.00 | 5.00 | 9.00 | 00.9 | 4.00 | 11.00 | 5.00 |
| Depth to Re (m | Top of Screen | 4.50 | 8.00 | 8.00 | 4.00 | 3.00 | 7.50 | 8.00 | 3.50 | 3.00 | | 5.00 | 5.00 | 2.00 | 5.00 | 1.00 | 4.00 | | 1.00 | 1.00 | 0.50 | 4.00 | 1.00 | 2.00 | 9.00 | 0.50 |
| Borchole | RL(m) | 14.14 | 23.03 | 25.54 | 14.99 | 11.95 | 17.92 | 30.16 | 12.18 | 18.23 | 22.46 | 17.16 | 22.71 | 22.55 | 23.50 | 10.69 | 15.01 | 10.52 | 18.88 | 12.37 | 13.37 | 18.43 | 23.81 | 23.50 | | 20.36 |
| | S | 5485496 | 5485791 | 5485818 | 5485446 | 5484217 | 5485808 | 5479036 | 5478594 | 5478349 | 5484080 | 5481429 | 5478931 | 5479090 | 5483607 | 5484309 | 5485013 | 5484409 | 5479182 | 5478788 | 5478146 | 5481366 | 5479214 | 5479314 | | 5479182 |
| Forestings | 9 | 1782236 | 1782375 | 1782438 | 1782282 | 1781115 | 1782405 | 17777309 | 1776918 | 1776822 | 1780967 | 1779181 | 1777216 | 1777411 | 1780678 | 1781144 | 1781914 | 1781201 | 1777506 | 1777078 | 1776695 | 1779269 | 1777783 | 1777882 | | 1777598 |
| Borchole | a | BH 201 | BH 202 | BH 203 | BH 204 | BH 205 | BH 206 | BH 207 | BH 208 | BH 209 | BH210 | BH 211 | BH 212 | BH 213 | BH 214 | BH 215 | BH 216 | BH 217 | BH 218 | BH 219 | BH 220 | BH 221 | BH 222 | BH 223 ² | | BH 224 |

| ia Ti | No. of Records | 7 | 6 | 14, | ∞ | 7 | 7 | | ည | 2 | 5 | 4 | 4 | S | 5 |
|------------------------|---------------------|---------|---------|---|---------|---------|---------|-------------------------|---------|---------|---------|---------|---------|----------------------------------|-----------------------------------|
| | Min. (mRL) | 15.19 | 20.85 | talled | 20.00 | 13.50 | 21.45 | talled | 10.66 | 10.84 | 20.84 | 11.52 | 13.98 | 18.12 | 16.31 |
| Results | Max. (mRL) | 16.09 | 22.02 | No piezometer installed | 20.71 | 14.12 | 21.89 | No piezometer installed | 11.24 | 11.27 | 21.61 | 12.54 | 15.27 | 17.61 | 15.92 |
| | Min. (mbgl) | 13.20 | 4.05 | No | 4.91 | 13.24 | 0.87 | No | 1.35 | 3.52 | 2.12 | 5.71 | 7.35 | 4.88 | 69.9 |
| | Max. (mbgl) | 14.10 | 5.22 | | 5.62 | 13.86 | 1.31 | | 1.93 | 3.95 | 2.89 | 6.73 | 8.64 | 5.39 | 7.08 |
| Depth to Response Zone | Bottom of Screen | 15.29 | 14.07 | | 12.62 | 12.86 | 7.26 | | 6:29 | 6.29 | 17.73 | 9.75 | 13.62 | | |
| Depth to R | Top of Screen | 19.29 | 18.07 | ter installed | 16.62 | 17.86 | 13.26 | ter installed | 11.59 | 9.29 | 21.73 | 14.75 | 17.62 | Unknown | Unknown |
| Depth to Response Zone | Bottom of Screen | 14.00 | 12.00 | No piezometer installed | 13.00 | 14.50 | 15.50 | No piezometer installed | 6.00 | 8.50 | 6.00 | 8.50 | 9.00 | Unka | Unk |
| Depth to R | Top of Screen | 10.00 | 8.00 | *************************************** | 9.00 | 9.50 | 9.50 | | 1.00 | 5.50 | 2.00 | 3.50 | 5.00 | | |
| 4 | RL (m) | 29.29 | 26.07 | 25.37 | 25.62 | 27.36 | 22.76 | 21.304 | 12.59 | 14.79 | 23.73 | 18.25 | 22.62 | 23.004 | 23.004 |
| | Northing3 | 5486311 | 5486716 | 5486741 | 5486658 | 5485830 | 5479307 | 5479112 | 5484165 | 5485461 | 5479324 | 5481392 | 5484094 | 54837024 | 54838634 |
| | Easting | 1782469 | 1782832 | 1782802 | 1782761 | 1782364 | 1777811 | 1777514 | 1781064 | 1782263 | 1777890 | 1779230 | 1780899 | 17806084 | 17803004 |
| Bowohoolo | D | BH225 | BH226 | BH227 | BH228 | BH229 | BH230 | BH231 | BH232 | BH233 | BH234 | BH235 | BH236 | 3 Addington Road, Otaki | 40 Addington Road, Otaki |

Some of the groundwater records may be influenced by drilling operations using drilling fluids or surface water during storm events.

¹ Water well locations at 3 and 40 Addington Road were pre-existing to the site investigations and response zones are unknown.

* BH223 has two piezometers installed.

*All coordinates presented are rounded to the nearest meter. Coordinates were professionally surveyed using NZ Map Grid projection and Local Datum Wellington 1953 and are accurate to approximately 100

4 Co-ordinates were measured using handheld GPS.

Table 2 – Surface ponding observations

| Location of surface ponding observation | Visually observed depth of surface ponding (magl) | Timing and duration of surface ponding |
|---|---|---|
| Area of interdune deposits around TP347 | ~1.0 m | Swampland – continuous ponding for most of the year. |
| BH206 | ~0.05 m | Boggy ground only over winter months; days of ponding after heavy rainfall. |
| Area of terrace alluvium around TP342 | ~0.05 m | Boggy ground only over winter months; days of ponding after heavy rainfall. |
| Area of sand dunes and terrace alluvium around TP330 and TP344 | ~0.05 m | Boggy ground only over winter months; days of ponding after heavy rainfall. |
| BH218 | ~0.10 m | Minimal boggy/ponding over winter months; days of ponding from adjacent stream overflow after heavy rainfall. |
| BH219 | ~0.30 m | Continuous ponding over winter months; days to a week after heavy rainfall. |
| BH208 | ~0.10 m | Boggy ground only over winter months; days of ponding after heavy rainfall. |
| BH220 | ~0.10 m | Boggy ground only over winter months; days of ponding after heavy rainfall. |
| C430 | ~0.05 m | Boggy ground only over winter months; days of ponding after heavy rainfall. |

Table 3 – Trial pit groundwater observations

| Location | Depth of excavation (m) | Depth below ground of groundwater seepage (m) | Date of trial pit excavation | Groundwater observations along length of excavation face |
|----------|-------------------------------|--|------------------------------------|--|
| TP329 | 4.10 | 3.90 | 16/02/2015 | Minor seepage/occasional dripping. |
| TP348 | 4.10 | 0.00 | 26/06/2015 | Swampland, exceptionally high flow. |
| TP305 | 4.50 | 0.70 | 22/01/2015 | Continuous flow. |
| TP347 | 3.00 | 0.00 | 23/06/2015 | Swampland, exceptionally high groundwater flow. |
| TP304 | 4.20 | 0.70 | 22/01/2015 | Continuous flow. |
| TP346 | 3.00 | 0.00 | 23/06/2015 | Swampland, exceptionally high flow. |
| TP340 | 4.70 | 4.50 | 11/05/2015 | Minor seepage/localised dripping. |
| TP342 | 3.50 | 3.00 | 11/05/2015 | Continuous flow. |
| TP303 | 4.50 | 4.00 | 21/01/2015 | Minor seepage/occasional dripping. |
| TP302 | 4.10 | 3.80 | 21/01/2015 | Minor seepage/occasional dripping. |
| TP301 | 4.50 | 4.40 | 21/01/2015 | Minor seepage/continuous dripping. |
| TP335 | 3.30 | 2.40 | 29/04/2015 | Minor seepage/localised dripping. |
| TP308 | 4.40 | 3.80 | 23/01/2015 | Minor seepage/localised dripping. |
| TP311 | 4.00 | 2.41 | 26/01/2015 | Minor seepage/occasional dripping. |
| TP322 | 5.40 | 5.10 | 12/02/2015 | Minor seepage/occasional dripping. |
| TP330 | 4.10 | 0.75 | 19/02/2015 | Continuous flow. |
| TP343 | 3.80 | 0.50 | 18/05/2015 | Continuous flow. |
| TP344 | 4.00 | 1.00 | 18/05/2015 | Continuous flow. |
| TP331 | 1.30 | 0.80 | 19/02/2015 | Continuous flow. |
| TP325 | 4.70 | 1.00 | 13/02/2015 | Continuous flow. |
| TP345 | 3.40 | 2.00 | 18/05/2015 | Continuous flow. |
| TP324 | 4.70 | 3.90 | 12/02/2015 | Minor seepage/localised dripping. |
| TP337 | 5.00 | 4.90 | 30/04/2015 | Minor seepage/localised dripping. |

Figures:

Site Project Area, Site Investigation Locations & Water Well Locations



2011 Site investigations

- Borehole Location
- ▼ Cone Penetration Test Location Trail Pit Location
- Borehole Location
 Cone Penetration Test Location
- Dynamic Cone Penetrometer Location
 Trial Pit Location
- Dynamic Cone Penetrometer Location Cone Penetration Test Location Borehole Location
- Pavement Pit Location Hand Auger Location
- Trial Pit Location

Investigation To be carried out on award of preferred tenderer

Proposed Borehole Location

Index:

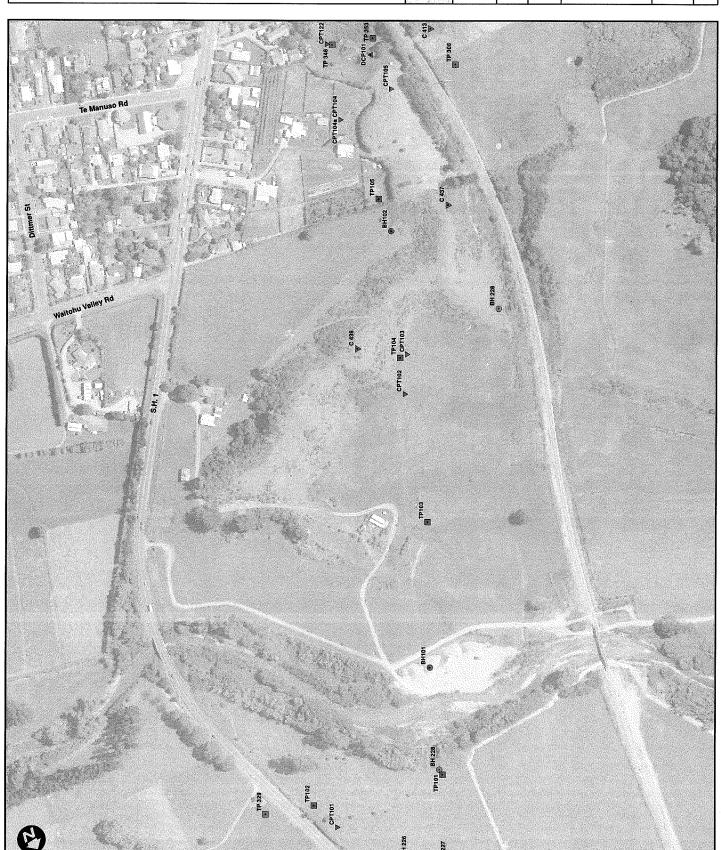
Engineering Geology and Locations of Geotechnical Investigations

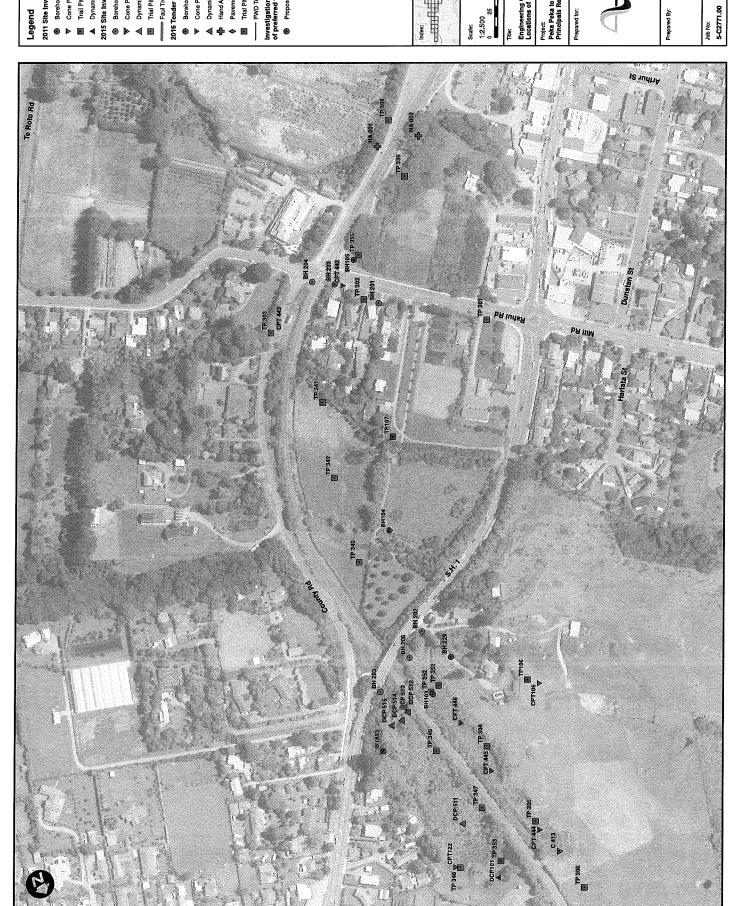
Project: Peka Peka to Claki Expressway Principals Requirements & Specimen Design Phase

NZ TRANSPORT AGENCY
WAKA KOTAHI

Prepared By:

OPUS







2011 Site Investigations

- Borehole Location
- Trail Pit Location
- 2015 Site Investigations
- Cone Penetration Test Locatio Borehole LocationCone Penetration T
- 2016 Tender Site investigation ****** Faul Trench Location
- Cone Penetration Test Location Borehole Location
- Dynamic Cone Penetrometer Location Hand Auger Location
- ** FWD Test Location Trial Pit Location

Investigation To be carried out on award of preferred tenderer

Proposed Borehole Location

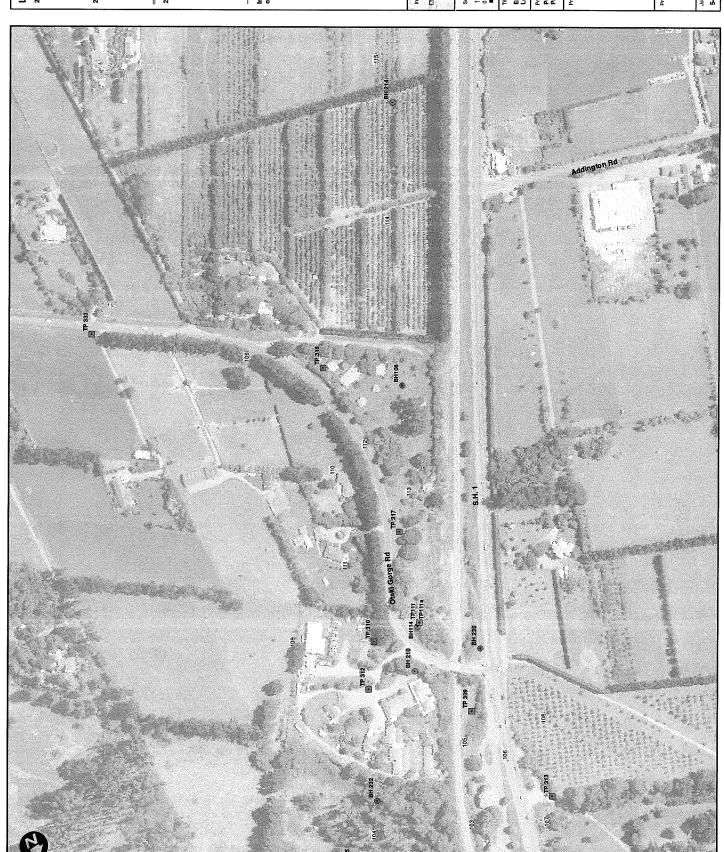


2011 Site Investigations

- Dynamic Cone Penetrometer Location Dynamic Cone PerTrial Pit Location
 - mm Faul Trench Location
- Dynamic Cone Penetr ▲ Dynamic Cone Penet
 ■ Hand Auger Location



OPUS

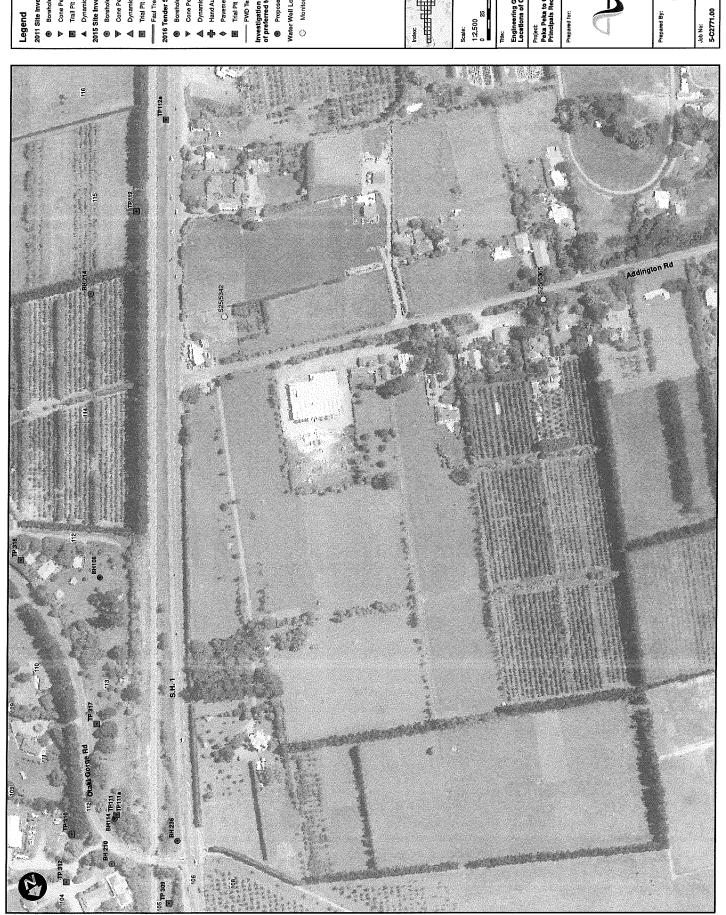


- Borehole Location
- Borehole Location▼ Cone Penetration T

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|------|-------|----|---|
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| ale: | 2,500 | | |

Engineering Geology and Locations of Geotechnical Investigations





- ▼ Cone Penetration Test Location

A Dynamic Cone Penetration Locatio

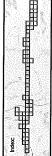
2015 Site Investigations

- Borehole Location
 Cone Penetration Test Location

 - Dynamic Cone PenetrometerTrial Pit Location

2016 Tender Site investigations management Faul Trench Location

- Cone Penetration Test Locatio Dynamic Cone Penetrometer Borehole Location
 Cone Penetration 1
- Hand Auger Location
 - Pavement Pit Location Pavement Pit Locati
 Trial Pit Location



Project Peka Peka to Otaki Expressway Principals Requirements & Specimen Design Phase

NZ TRANSPORT AGENCY
WAKA KOTAHI

OPUS

Date: 13/01/2017

6a



- Borehole Location
- ♥ Cone Penetration Test Location Trail Pit Location

Dynamic Cone Penetration

- Borehole Location
- Cone Penetration Test Location

Dynamic Cone Penetromete

Trial Pit Location

- 2016 Tender Site Investigation: **Meeting Faul Trench Location**
- Dynamic Cone Penetrometer Location ▼ Cone Penetration Test Location
 - Pavement Pit Location Hand Auger Location

Proposed Borehole Location

| | | 100 | Meters |
|------|-------|-----|--------|
| | | 20 | |
| ale: | 2,500 | 52 | |

Project: Peka Peka to Claki Expressway Principals Requirements & Specimen Design Phase Engineering Geology and Locations of Geotechnical Investigations



OPUS

Prepared By:



- Trail Pit Location
- 2015 Site Investigations Borehole Location
- ▲ Dynamic Cone Penetrometer
 ▼ Trial Pit Location
 - mme Faul Trench Location
- 2016 Tender Site Investigatio

Cone Penetration Test Location ❸ Borehole Location▼ Cone Penetration 1

- Dynamic Cone Penetro

- Proposed Borehole Location

Project: Peka Peka to Otaki Expressway Principals Requirements & Specimen Design Phase

NZ TRANSPORT AGENCY
WAKA KOTAHI

OPUS



- 2011 Site Investigations Borehole Location
- ▼ Cone Penetration Test Location
- 2015 Site Investigations Borehole Location
- Dynamic Cone Penetrometer Location
 Trial Pit Location
- 2016 Tender Site Investigations Borehole Location
- Cone Penetration Test Location
- Dynamic Cone Penetrometer Location
 Hand Auger Location
- Pavement Pit Location
- Investigation To be carried out on award of preferred tenderer --- FWD Test Location

Proposed Borehole Location

Project Peka Peka to Claki Expressway Principals Requirements & Specimen Design Phase

VZ TRANSPORT AGENCY

OPUS





2011 Site Investigations

- ♥ Cone Penetration Test Location Borehole Location
- Dynamic Cone Penetration Location Trail Pit Location
 - 2015 Site Investigations Borehole Location
- Dynamic Cone Penetrometer Location
 Trial Pit Location Cone Penetration Test Location

2016 Tender Site Investigation

- ▼ Cone Penetration Test Location Borehole Location
- ▲ Dynamic Cone Penetrometer Location
 Hand Auger Location
 - Pavement Pit Location

Proposed Borehole Location

| | 100 Meters | |
|--------|-----------------|--|
| | 20 | |
| Scale: | 1:2,500 0 25 | |

Engineering Geology and Locations of Geotechnical Investigations



V NZ TRANSPORT AGENCY WAKA KOTAHI

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| |

Prepared By:

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|---------|------------|
| Figure: | |
| Date: | 29/07/2016 |



- Borehole Location
 Cone Penetration Test Location
 - Trail Pit Location
- Dynamic Cone Penetration Location 2015 Site Investigations
 - Borehole Location
 Cone Penetration 7
- Dynamic Cone Penetrometer Location Cone Penetration Test Location
 - Dynamic Cone PenTrial Pit Location

2016 Tender Site Investigations mmmm Faul Trench Location

- Cone Penetration Test Location Borehole Location

 Cone Penetration 7
 - Dynamic Cone Penetrometer Hand Auger Location
 - Pavement Pit Locat
 Trial Pit Location

Pavement Pit Location

Proposed Borehole Location



Project Peka Peka to Otaki Expressway Principals Requirements & Specimen Design Phase

NZ TRANSPORT AGENCY
WAKA KOTAH! Prepared By:

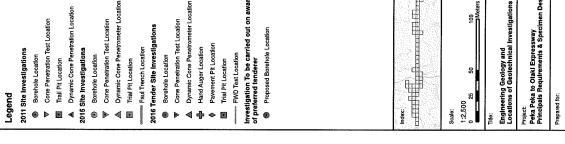
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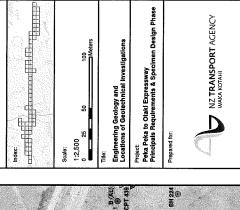
Date: 29/07/2016

42

CPT 447 BH 234 TP 322

CPT107









❸ Borehole Location
 ▼ Cone Penetration Test Location

Trail Pit Location

Dynamic Cone Penetration Location

❸ Borehole Location▼ Cone Penetration Test Location

 Dynamic Cone Penetrometer Location
 Trial Pit Location Faul Trench Location

 Borehole Location
 Cone Penetration Test Location Dynamic Cone Penetrometer
 Hand Auger Location

Pavement Pit Location
Trial Pit Location

Investigation To be carried out on award of preferred tenderer

Proposed Borehole Location

Peka Peka to Otaki Expressway Principals Requirements & Specimen Design Phase

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OPUS



2011 Site Investigations

- Borehole Location
- ▼ Cone Penetration Test Location Trail Pit Location
 - A Dynamic Cone Penetration 2015 Site Investigations Borehole Location
- Cone Penetration Test Location Dynamic Cone Penetro
- 2016 Tender Site Investigation manumen Faul Trench Location

Trial Pit Location

- Cone Penetration Test Location Borehole Location
- Dynamic Cone Penetrometer Location Hand Auger Location
- Pavement Pit Location Trial Pit Location

Investigation To be carried out on award of preferred tenderer

1:2,500

Prepared By:

NZ TRANSPORT AGENCY
WAKA KOTAHI

OPUS

Figure: Date: 29/07/2016



- ❸ Borehole Location▼ Cone Penetration Test Location
- Dynamic Cone Penetration Location 2015 Site investigations Trail Pit Location
 - Borehole LocationCone Penetration Test Location
- Dynamic Cone Penetrometer Location
 Trial Pit Location mms Faul Trench Location
 - 2016 Tender Site Investigation ❸ Borehole Location▼ Cone Penetration T
- Dynamic Cone Penetrometer
 Hand Auger Location

Cone Penetration Test Location

- Pavement Pit Locatio Pavement Pit Locati

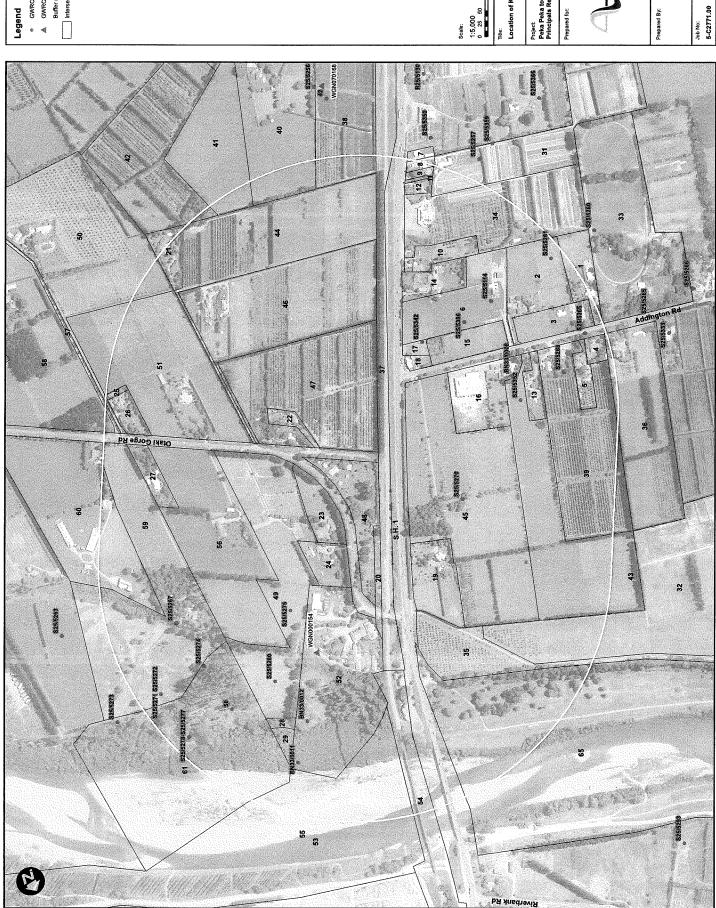
Peka Peka to Otaki Expressway Principals Requirements & Specimen Design Phase

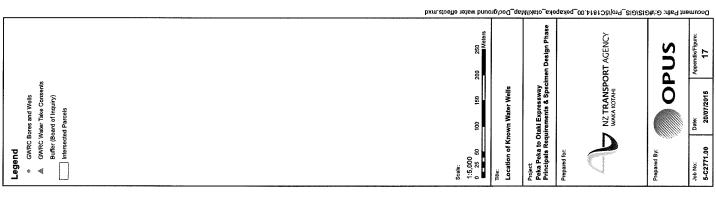


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Date: 29/07/2016

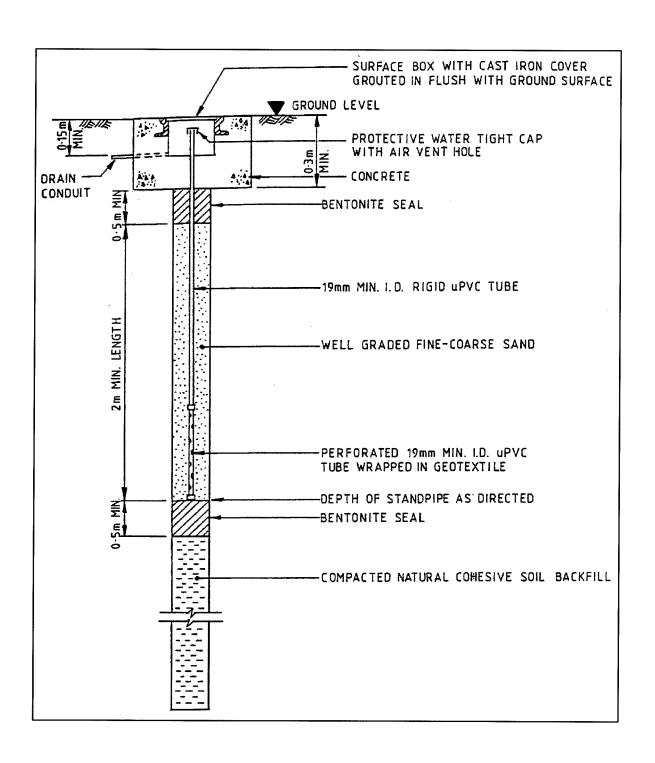
16





Appendix A:

Piezometer Construction Specification



Appendix B:

Piezometer Monitoring Records

Appendix B –Piezometer monitoring records (mbgl)

| | Water level in terms of mbgl | | | | | | | | | | |
|-------------|------------------------------|--------------|---|--------------|--------------|--------------|--------------|--------------|--------------|--|--|
| Borehole ID | 16/02/2015 | 27/02/2015 | 6/03/2015 | 13/03/2015 | 23/03/2015 | 3/04/2015 | 10/04/2015 | 22/04/2015 | 8/05/2015 | | |
| BH201 | 4.24 | 4.10 | 4.10 | 4.07 | 4.08 | 4.05 | 0.55 | 4.00 | 0.80 | | |
| BH 202 | 9.08 | 4.10 9.16 | 4.13 9.21 | 4.07 9.05 | 4.08 9.10 | 4.05 9.10 | 3.55 8.96 | 4.00 9.10 | 3.89 9.00 | | |
| BH 203 | 8.24 | 8.60 | 8.66 | 8.41 | 8.35 | 8.30 | 8.05 | 8.32 | 8.25 | | |
| BH 204 | 4.41 | 4.48 | 4.54 | 4.41 | 4.40 | 4.40 | 4.00 | 4.32 | 4.05 | | |
| BH 205 | 2.05 | 2.12 | 2.19 | 2.00 | 2.05 | 2.05 | 1.92 | 2.00 | 1.97 | | |
| BH 206 | 5.17 | 5.21 | 5.16 | 5.05 | 5.00 | 4.95 | 4.72 | 4.97 | 4.90 | | |
| BH 207 | 9.09 | 9.00 | 9.16 | 9.21 | 9.20 | 9.21 | 9.10 | 9.15 | 9.05 | | |
| BH 208 | 0.48 | 0.52 | 0.60 | 0.55 | 1.50 | 1.49 | 1.45 | 1.47 | 1.41 | | |
| BH 209 | 1.83 | 1.89 | 1.92 | 1.83 | 1.77 | 1.70 | 1.70 | 1.76 | 0.67 | | |
| BH210 | No piezometer | | | | | | | | | | |
| BH 211 | 6.65 | 6.71 | 6.69 | 6.72 | 6.76 | 6.70 | 5.60 | 6.75 | 6.69 | | |
| BH 212 | 3.26 | 3.24 | 3.24 | 3.30 | 3.43 | 3.42 | 3.35 | 3.39 | 3.28 | | |
| BH 213 | | | 5.00 | 3.10 | 3.26 | 3.25 | 3.20 | 3.25 | 3.21 | | |
| BH 214 | | 5.28 | 8.35 | 8.29 | 8.20 | 8.17 | 7.90 | 8.16 | 8.05 | | |
| BH 215 | | 1.15 | 1,22 | 1.07 | 1.05 | 1.00 | 1.00 | 1.05 | 1.05 | | |
| BH 216 | | | 4.65 | 5.03 | 5.00 | 4.95 | 4.60 | 4.97 | 4.81 | | |
| BH 217 | No piezometer | | 4000 | J. J | 5,00 | 7.20 | 4.00 | 7.2/ | 4.01 | | |
| BH 218 | | | | 0.68 | 0.60 | 0.58 | 0.52 | 0.60 | 0.58 | | |
| BH 219 | | | | 1.05 | 1.05 | 1.00 | 1.00 | 1.05 | 1.03 | | |
| BH 220 | | | | *** | - | | | | | | |
| BH 221 | | | | | | | | | 7.62 | | |
| BH222 | | | | | | | | | | | |
| BH2231 | | | | | | | | | | | |
| BH223 | | | *************************************** | | | | *** | | | | |
| BH224 | | | · | | | | | | | | |
| BH225 | | | | | | | | | | | |
| BH226 | | | | | | | | | | | |
| BH227 | No piezometer | | | | | | | | | | |
| BH228 | | | | | | | | | | | |
| BH229 | | | | | | | | | | | |
| BH230 | | | | | | | | | | | |
| BH231 | | | | | | | | | | | |
| BH232 | No piezometer | | | | | | | | | | |
| BH233 | • | | | | | | | | | | |
| BH234 | | | | | | | | | | | |
| BH235 | | | | | | | | | | | |
| BH236 | | | | | | | | | | | |
| Note: | i | | | | | | | | | | |

Blank cells indicate investigation location not completed at monitoring date.

Some early records for each piezometer may be influenced by drilling operations using fluids.

Shallower response zone of BH223.

Appendix B –Piezometer monitoring records (mbgl)

| Borchole | Water level in terms of mbgl | | | | | | | | |
|------------------|--|--------------|--------------|--------------|--------------|--------------|--------------|--------------|--------------|
| TD . | 11/05/2015 | 18/05/2015 | 3/06/2015 | 8/06/2015 | 16/06/2015 | 6/07/2015 | 15/07/2015 | 30/07/2015 | 20/08/2015 |
| BH201 | 3.65 | 2.20 | 2.75 | 3.00 | 3.14 | 3.15 | 3.20 | 3.22 | 3.23 |
| BH 202 | 8.80 | 8.25 | 8.43 | 8.45 | 8.80 | 8.79 | 8.80 | 8.80 | 8.87 |
| BH 203 BH 204 | 8.17 | 7.95 2.55 | 7.88 2.95 | 7.92 3.02 | 6.92 3.66 | 7.25 3.65 | 7.52 3.60 | 7.97 3.72 | 7.91 3.74 |
| BH 205 | 3.72 1.95 | 1.45 | 1.52 | 1.65 | 1.15 | 0.97 | 0.94 | 0.87 | 0.89 |
| BH 206 | 4.75 | 4.10 | 4.10 | 4.15 | 6.40 | 6.39 | 6.35 | 6.45 | 6.40 |
| BH 207 | 8.91 | 8.10 | 9.10 | 9.15 | 9.81 | 8.95 | 8.70 | 8.10 | 8.10 |
| BH 208 | 1.39 | 1,29 | 1.42 | 1.45 | 0.97 | 1.00 | 0.95 | 0.86 | 0.85 |
| BH 209 | 1.72 | 1.61 | 1.72 | 1.75 | 1.73 | 1.80 | 1.75 | 1.68 | 1.70 |
| BH210 | No piezometer | | | , , | | | • | | • |
| BH 211 | 6.05 | 5.95 | 6.00 | 6.05 | 5.84 | 5.62 | 5.57 | 5.45 | 5.50 |
| BH 212 | 2.95 | 2.05 | 3.40 | 3.47 | 2.12 | 1.85 | 1.65 | 1.41 | 1.45 |
| BH 213 | 3.00 | 2.21 | 3.32 | 3.37 | 3.07 | 3.15 | 3.18 | 3.10 | 3.15 |
| BH 214 | 7.55 | 6.25 | 6.88 | 6.93 | 7.25 | 6.90 | 6.88 | 6.85 | 6.80 |
| BH 215 | 1.00 | 0.90 | 1.15 | 1.18 | 0.81 | 0.52 | 0.47 | 0.40 | 0.41 |
| BH 216 | 4.40 | 3.30 | 3.72 | 3.77 | 4.32 | 4.05 | 4.10 | 3.97 | 3.95 |
| BH 217 | No piezometer | | | | | | | | |
| BH 218 | 0.60 | 0.55 | 1.00 | 1.10 | 0.65 | 0.45 | 0.40 | 0.30 | 0.35 |
| BH 219 | 1.00 | 0.93 | 1.05 | 1.07 | 0.77 | 0.67 | 0.60 | 0.51 | 0.54 |
| BH 220 | 1.05 | 0.91 | 1.02 | 1.09 | 1.05 | 1.10 | 1.00 | 0.85 | 0.81 |
| BH 221 | | 7.58 | 7.77 | 7.79 | 8.03 | 7.85 | 7.82 | 7.79 | 7.75 |
| BH222 | | | 4.47 | 4.53 | 4.92 | 1.90 | 1.70 | 0.91 | 0.89 |
| BH2231 | | | 9.81 | 9.85 | 1.34 | 1.15 | 1.15 | 1.07 | 1.08 |
| BH223 | | | | | 1.38 | 1,21 | 1.20 | 1.10 | 1.10 |
| BH224 | | | 4.52 | 4.60 | 0.30 | 0.42 | 0.35 | 0.00 | 0.02 |
| BH225 | | | | | | | | | |
| BH226 | | | | | | | | | |
| BH227 | | | | | | | | | |
| BH228 | 70-70-7-10-20-20-7-10-7-10-7-10-7-10-7-1 | | | | | | | | |
| BH229 | | | | | | | | | |
| BH230 | | | | | | | | | |
| BH231 | | | | | | | | | |
| BH232 | | | | | | - | | | |
| BH233 | | | | | | | | | |
| BH234 | | | | | | | | | |
| BH235 | | | | 2 | | | | | |
| BH236 | | | | | | | | | |
| Moto: | | | | | | | | | |

Note:
Blank cells indicate investigation location not completed at monitoring date.

Shallower response zone of BH223.

Appendix B –Piezometer monitoring records (mbgl)

| Borehole | Water level in terms of mbgl | | | | | | | | |
|------------------|------------------------------|--------------|--------------|---------------------------------------|--------------|--------------|--------------|--------------|--------------|
| ID | 1/09/2015 | 14/09/2015 | 29/09/2015 | 14/10/2015 | 23/10/2015 | 11/11/2015 | 25/11/2015 | 11/12/2015 | 17/12/2015 |
| BH201 | 3.20 | 3.33 | 3.35 | 3.46 | 3.58 | 3.73 | 3.70 | 3.71 | 3.70 |
| BH 202 | 8.86 | 8.91 | 8.88 | 8.90 | 8.91 | 8.94 | 8.91 | 8.93 | 8.90 |
| BH 203 BH 204 | 7.98 3.65 | 7.99 3.72 | 7.96 3.69 | 7.82 3.76 | 8.00 3.81 | 8.04 4.00 | 7.00 3.97 | 7.01 3.97 | 6.99 3.96 |
| BH 205 | 0.85 | 0.87 | 0.90 | 0.91 | 0.92 | 0.97 | 0.96 | 0.98 | 0.98 |
| BH 206 | 6.36 | 6.41 | 6.40 | 6.36 | 6.54 | 6.76 | 6.69 | 6.67 | 6.68 |
| BH 207 | 8.15 | 2.17 | 8.14 | 8.22 | 8.27 | 8.32 | 8.30 | 8.31 | 8.30 |
| BH 208 | 0.82 | 0.80 | 0.81 | 0.88 | 0.40 | 0.95 | 0.94 | 0.95 | 0.97 |
| BH 209 | 0.71 | 1.72 | 1.72 | 1.76 | 1.75 | 1.78 | 1.77 | 1.78 | 0.78 |
| BH210 | No piezometer | | | | | | | • | |
| BH 211 | 5.61 | 5.60 | 5.58 | 5.66 | 5.83 | 5.99 | 5.96 | 5.98 | 5.97 |
| BH 212 | 1.49 | 1.53 | 1.55 | 1.71 | 2.02 | 2.24 | 2.21 | 2.22 | 2.24 |
| BH 213 | 3.30 | 3.33 | 3.35 | 3.47 | 3.55 | 3.62 | 3.60 | 3.60 | 3.62 |
| BH 214 | 6.79 | 6.82 | 6.81 | 6.84 | 6.87 | 6.95 | 6.93 | 6.94 | 6.92 |
| BH 215 | 0.38 | 0.39 | 0.40 | 0.44 | 0.45 | 0.49 | 0.48 | 0.50 | 0.49 |
| BH 216 | 3.96 | 4.00 | 3.98 | 4.03 | 4.02 | 4.05 | 4.01 | 4.00 | 4.01 |
| BH 217 | No piezometer | | | | - | | | | |
| BH 218 | 0.30 | 0.27 | 0.42 | 0.53 | 0.88 | 0.92 | 0.91 | 0.94 | 0.94 |
| BH 219 | 0.49 | 0.52 | 0.50 | 0.59 | 0.67 | 0.78 | 0.76 | 0.77 | 0.76 |
| BH 220 | 0.73 | 0.71 | 0.89 | 0.88 | 0.86 | 0.92 | 0.91 | 0.93 | 0.95 |
| BH 221 | 7.73 | 7.72 | 7.70 | 7.78 | 7.87 | 8.05 | 8.05 | 8.05 | 8.04 |
| BH222 | 0.87 | 0.88 | 0.85 | 0.90 | 0.92 | 0.97 | 0.97 | 0.98 | 0.98 |
| BH2231 | 1.06 | 1.09 | 1.09 | 1.09 | 1.10 | 1.11 | 1.09 | 1.09 | 1.10 |
| BH223 | 1.07 | 1.10 | 1.09 | 1.12 | 1,12 | 1.15 | 1.13 | 1.14 | 1.13 |
| BH224 | 0.10 | 0.50 | 0.10 | 0.05 | 0.01 | 0.00 | 0.02 | 0.01 | 0.02 |
| BH225 | | | | | | | | | |
| BH226 | | | | | | | | | |
| BH227 | | | | | | | | | |
| BH228 | | | | | | | | | |
| BH229 | | | | , , , , , , , , , , , , , , , , , , , | | | | ****** | |
| BH230 | | | | | | | | | |
| BH231 | | | | | | | | | |
| BH232 | | | | | | | | | |
| BH233 | | | - | | | | | | |
| BH234 | | | | | | | | | |
| BH235 | | | | | | | | | |
| BH236 | | | | | | | | | |
| Nate: | | | | | | | | | |

Blank cells indicate investigation location not completed at monitoring date.

Shallower response zone of BH223.

2016/29 | December 2016 Opus International Consultants Ltd

Appendix B –Piezometer monitoring records (mbgl)

| Borehole | Water level in terms of mbgl | | | | | | | | | | |
|----------------------------------|------------------------------|--------------|--------------|--|--------------|--------------|--------------|--------------|--------------|--|--|
| ID | 21/01/2016 | 17/02/2016 | 7/03/2016 | 19/04/2016 | 23/05/2016 | 20/06/2016 | 29/06/2016 | 12/07/2016 | 22/07/2010 | | |
| BH201 | 4.15 | 4.12 | 4.10 | 3.93 | 3.52 | 3.10 | 3.17 | 3.30 | 3.37 | | |
| BH 202 | 9.16 | 9.15 | 9.17 | 9.08 | 9.05 | 8.80 | 8.85 | 8.80 | 8.92 7.82 | | |
| BH 203 BH 204 | 8.67 4.53 | 8.65 4.57 | 8.57 4.48 | 8.27 4.21 | 7.97 3.87 | 7.40 3.60 | 7.55 3.67 | 8.50 3.77 | 3.86 | | |
| BH 205 | 2.13 | 2.10 | 2.10 | 1.99 | 1.89 | 1.48 | 1.47 | 1.63 | 1.56 | | |
| BH 206 | 5.23 | 5.20 | 5.18 | 4.48 | 3.98 | 3.16 | 3.05 | 3.16 | 3.20 | | |
| BH 207 | 9.04 | 9.02 | 8.97 | 9.20 | 9.00 | 8.60 | 8.56 | 8.55 | 8.62 | | |
| BH 208 | 0.55 | 0.50 | 0.53 | 0.48 | 0.37 | 0.00 | 0.00 | 0.00 | 0.00 | | |
| BH 209 | 1.95 | 1.93 | 1.90 | 1.88 | 1.95 | 1.78 | 1.74 | 1.78 | 1.72 | | |
| BH210 | No piezometer | | | | | | | | | | |
| BH 211 | 6.74 | 6.70 | 6.68 | 6.42 | 6.01 | 5.59 | 5.64 | 5.55 | 6.02 | | |
| BH 212 | 3.30 | 2.27 | 2.24 | 2.21 | 2.36 | 2.17 | 1.92 | 1.77 | 1.72 | | |
| BH 213 | 3.13 | 3.15 | 3.10 | 3.44 | 3.64 | 3.83 | 3.77 | 3.75 | 3.77 | | |
| BH 214 | 5.30 | 5.25 | 5.20 | 5.41 | 5.57 | 5.71 | 5.67 | 5.53 | 5.60 | | |
| BH 215 | 1.17 | 1.15 | 1.12 | 0.89 | 0.78 | 0,40 | 0.27 | 0.64 | 0.63 | | |
| BH 216 | 4.12 | 4.05 | 4.05 | 4.29 | 4.50 | 4.50 | 4.47 | 4.54 | 4.69 | | |
| BH 217 | No piezometer | | | de la constante de la constant | | | 1991 | | LUMANUTUSTE | | |
| BH 218 | 0.70 | 0.71 | 0.69 | 0.42 | 0.31 | 0.00 | 0.00 | 0.00 | 0.00 | | |
| BH 219 | 1.10 | 1.05 | 1.02 | 0.97 | 0.48 | 0.10 | 0.05 | 0.03 | 0.31 | | |
| BH 220 | 0.97 | 0.90 | 0.87 | 0.82 | 0.87 | 0.69 | 0.65 | 0.50 | 0.54 | | |
| BH 221 | 8.07 | 8.05 | 8.00 | 7.41 | 7.22 | 6.50 | 6.12 | 6.14 | 6.67 | | |
| BH222 | 1.00 | 0.97 | 0.95 | 0.99 | 1.01 | 1.01 | 1.04 | 1.03 | 1.04 | | |
| BH2231 | 1.14 | 1.11 | 1.11 | 1.12 | 1.11 | 1.13 | 2.15 | 2.21 | 1.98 | | |
| BH223 | 1.15 | 1.14 | 1.14 | 1.07 | 1.10 | 1,06 | 2.12 | 2.15 | 2.00 | | |
| BH224 | 0.10 | 0.09 | 0.11 | 0.02 | 0.04 | 0.00 | 0.00 | 0.00 | 0.00 | | |
| BH225 | | | | 13.89 | 13.56 | 13.38 | 13.20 | 14.10 | 13.76 | | |
| BH226 | | | | 5.22 | 4.75 | 4.05 | 4.17 | 4.65 | 4.51 | | |
| BH227 | No piezometer | | | | | | | | | | |
| BH228 | | | | 5.62 | 5.21 | 4.95 | 4.91 | 5.13 | 5.13 | | |
| BH229 | | | | | | 13.24 | 13.46 | 13.44 | 13.45 | | |
| BH230 | | | | | | 1,21 | 1.13 | 1,31 | 1.26 | | |
| BH231 | No piezometer | | | | | | | | | | |
| BH232 | | | | | | | | 1.93 | 1.35 | | |
| BH233 | | | | | | | | 3.68 | 3.95 | | |
| BH234 | | | | | | | | 2.89 | 2.65 | | |
| BH235 | | | | | | | | | 6.73 | | |
| BH236 | | | | | | | | | 8.64 | | |
| 3 Addington Road 40 | | | | | | 5.39 | 5.30 | 5.31 | 5.15 | | |
| 40 Addington Road Vote: | | | | | | 7.08 | 7.01 | 7.02 | 6.89 | | |

Note:
Blank cells indicate investigation location not completed at monitoring date.
Shallower response zone of BH223.

Appendix B -Piezometer monitoring records (mbgl)

| | Water | level in terms | of mbgl | |
|-------------------------------|------------------|----------------|---------------------|--|
| Borchole ID | | · | 1 | |
| | 7/09/2016 | 14/10/2016 | 22/12/2016 | |
| BH201 | | | Could not | |
| BH 202 | 3.39 | 3.16 8.84 | locate | |
| BH 202 | 9.05 8.07 | 7.89 | 8.97 8.92 | |
| BH 204 | 3.88 | 3.61 | 3.92 | |
| BH 205 | 1.52 | 1.65 | Could not locate | |
| BH 206 | 3.30 | 2.73 | 2.96 | |
| BH 207 | 8.51 | 8.22 | 8.26 | |
| BH 208 | 0.00 | 0.00 | Could not locate | |
| BH 209 | 1.91 | 1.95 | Could not locate | |
| BH210 | No piezometer | | | |
| BH 211 | 6.07 | 5.46 | 6.33 | |
| BH 212 | 1.61 | 1.36 | 1.70 | |
| BH 213 | 3.78 | 3.91 | 3.79 | |
| BH 214 | 5.42 | 5.14 | 7.67 | |
| BH 215 | 0.61 | 0.74 | 0.56 | |
| BH 216 | 4.78 | 4.53 | Could not locate | |
| BH 217 | No piezometer | | | |
| BH 218 | 0.52 | 0.00 | 0.00 | |
| BH 219 | 0.48 | 0.38 | 0.00 | |
| BH 220 | Destroyed | Destroyed | Destroyed | |
| BH 221 | 6.50 | 5.95 | Could not locate | |
| BH222 | 1.23 | 1.20 | 1.60 | |
| BH2231 | 2.13 | 1.55 | Could not locate | |
| BH223 | 2.10 | 1.57 | Could not locate | |
| BH224 | 0.22 | 0.00 | 0.00 | |
| BH225 | 13.75 | Gate locked | Gate locked | |
| BH226 | 4.49 | 4.42 | 4.49 | |
| BH227 | No piezometer | | | |
| BH228 | 5.19 | 5.08 | Destroyed | |
| BH229 | 13.39 | 13.27 | 13.86 | |
| BH230 | 0.87 | 1.19 | 1.23 | |
| BH231 | No piezometer | | | |
| BH232 | 1.62 | 1.74 | 1.53 | |
| BH233 | 3.92 | 3.57 | 3.52 | |
| BH234 | 2.66 | 2.12 | 2.85 | |
| BH235 | 6.53 | 5.71 | 5.92 | |
| BH236 | <i>7</i> ⋅45 | 7.35 | 7.36 | |
| 3 Addington Road | 5.05 | 4.88 | Could not locate | |
| 40 Addington Road Vote: | 6.81 | 6.69 | Could not locate | |

Blank cells indicate investigation location not completed at monitoring date. 'Shallower response zone of BH223.



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